APPENDIX E

NOISE AND GROUNDBORNE VIBRATION IMPACT ASSESSMENT







NOISE AND GROUNDBORNE VIBRATION IMPACT ASSESSMENT

Pacific Rock Quarry Conditional Use Permit Modification Application LU10-0003

November 2020

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Noise and Groundborne Vibration Impact Assessment

Pacific Rock Quarry Conditional Use Permit Modification Application LU10-0003

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EXECUTIVE SUMMARY

This Noise and Groundborne Vibration Impact Assessment (NVIA) presents regulatory review, ambient noise measurements, noise impact predictions, and vibration impact predictions for the Pacific Rock Quarry located in unincorporated Ventura County near the City of Camarillo, California (Figure 1). This NVIA is intended for use as a technical document in support of the California Environmental Quality Act (CEQA) assessment for the Project.

The Project site consists of an existing aggregate mine and processing plant. Pacific Rock has prepared and submitted to the County a Conditional Use Permit (CUP) Major Modification application (LU10-0003) to entitle the following proposed modifications to the existing Project:

- Extend the life of the existing permitted operations.
- Expand the mining area boundary to the east, north, and south (Figure 2).
- Extend the operating schedule from six (6) to seven (7) days per week (to include material load out on Sundays).
- Allow additional material load out hours and a limited number of extended 24-hour operation days (60 days maximum per year).
- Operate a portable crushing and screening plant onsite to recycle concrete debris (Recycle Plant).
- Install a structure for a 24-hour onsite security guard.

The following existing Project features would remain unchanged:

- Daily maximum aggregate production rate.
- Number of daily truck trips and truck haul routes.
- Number of employees.
- Aggregate excavation and processing equipment and methods.
- Basting event hours, frequency, and methods.

This NVIA makes the following determinations regarding significance of noise and groundborne vibration impacts resulting from the Project:

- Noise impacts from onsite sources ("Non-Transportation") are less than significant after mitigation.
- Noise impacts from traffic sources ("Transportation") are less than significant.
- Groundborne vibration impacts are less than significant.
- The Project would result in a Class II impact, significant but mitigable to less than significant levels.



NOISE AND GROUNDBORNE VIBRATION IMPACT ASSESSMENT

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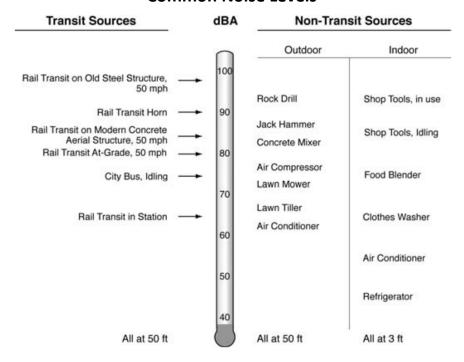
1.0 INTRODUCTION

This Noise and Groundborne Vibration Impact Assessment (NVIA) presents regulatory review, ambient noise measurements, noise impact predictions, and groundborne vibration impact predictions for the Pacific Rock Quarry located in unincorporated Ventura County near the City of Camarillo, California (Figure 1). Pacific Rock is proposing to extend the life of the Condition Use Permit (CUP), expand the mining area boundary, extend the operating schedule to seven (7) days per week, allow for additional material load out hours, operate a portable crushing and screening plant onsite to recycle concrete debris (Recycle Plant), and install a structure for a 24-hour onsite security guard (Project).

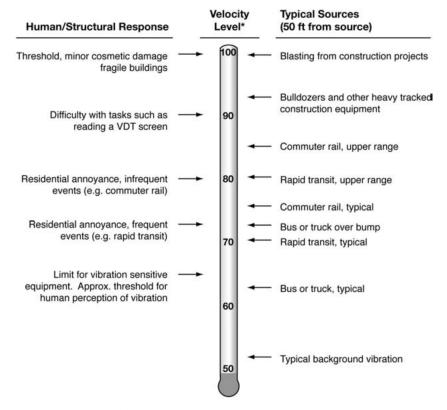
This NVIA is intended for use in the environmental review for the Project under the California Environmental Quality Act (CEQA). Methodologies and criteria outlined in the Ventura County 2040 General Plan Hazards and Safety Element (Ventura County, 2020), the Ventura County Initial Study Assessment Guidelines (Ventura County, 2011), the Ventura County Construction Noise Threshold Criteria and Control Plan (Ventura County, 2010), and applicable state and federal transportation agency (e.g., Caltrans, Federal Transit Administration, etc.) noise and vibration guidelines are utilized to determine the significance of Project impacts. The Project's onsite non-transportation industrial noise and vibration sources (e.g., equipment operating onsite, blasting, etc.) and transportation noise sources (i.e., haul trucks on public roads) have been quantified and compared to applicable significance thresholds in this NVIA.

Illustrations on the next page, which are from the Federal Transit Administration's (FTA's) *Transit Noise and Vibration Impact Assessment Manual* (Federal Transit Administration, 2018), present the intensity level of common noise and vibration generating activities.

Common Noise Levels



Common Vibration Levels



* RMS Vibration Velocity Level in VdB relative to 10-6 inches/second

Source: (Federal Transit Administration, 2018)

2.0 EXISTING SETTING

The Project site is located approximately 1.5 miles east of Lewis Road and approximately 2.0 miles south of U.S. Highway 101 off a private road (Howard Road), in unincorporated Ventura County near the City of Camarillo and the City of Thousand Oaks (Figure 1). The site is located on the west facing side of the Santa Monica Mountain Range. The Camarillo Airport is located approximately 4.5 miles away to the northwest and the Point Mugu Naval Air Station is located approximately 7.2 miles away to the southwest.

This section discusses the Project's existing environmental and regulatory settings.

2.1 Noise & Groundborne Vibration Fundamentals

2.1.1 Definitions

The following terms are employed in this NVIA:

- A-Weighted Sound Level (dBA): Sound pressure level measured using the A-weighting network, a filter
 which discriminates against low and very high frequencies in a manner similar to the human hearing
 mechanism at moderate sound levels.
- Ambient Noise Level: The noise that results from the combination of all sources, near and far.
- Community Noise Equivalent Level [CNEL dB(a)]: The long-term time average sound level, weighted as
 follows:
 - Frequency response is filtered using the A-weighting network.
 - Sounds occurring between 7:00 p.m. and 10:00 p.m. are weighted by +5 dB.
 - O Sounds occurring between 10:00 p.m. and 7:00 a.m. are weighted by +10 dB.
- **Decibel (dB):** A unit division, on a logarithmic scale, whose base is the tenth root of ten, used to represent ratios of quantities proportional to power. In simple terms, if the power is multiplied by a factor of ten, then ten is added to the representation of the power on the decibel scale. If 0 dB represents 1 unit of power, 60 dB represents one million units, etc.
- Equivalent Continuous Noise Level (L_{eq}): The level, in decibels, of the mean sound pressure averaged over time period, generally one hour. This is often referred to as "equivalent sound level" and hence the "eq" subscript. The "equivalence" is to a sound of constant level that has the same total acoustic energy content.
- **Peak Particle Velocity (PPV)**: The peak signal value of an oscillating vibration velocity waveform. PPV is usually expressed in inches per second (in/sec) in the United States.
- Root Mean Square (rms): The square root of the mean-square value of an oscillating waveform, where
 the mean-square value is obtained by squaring the value of amplitudes at each instant of time and then
 averaging these values over the sample time.
- Sound Pressure Level (SPL): The logarithmic measure of the power of a sound relative to a reference value, measured in decibels (dB). The sound pressure level is always associated with a specific location or distance from a sound source.
- **Sound Power Level (SWL):** The acoustical energy emitted by the sound source. The SWL is an absolute value that is not affected by the environment, unlike SPL.

2.1.2 Characteristics of Noise

Noise is often described as unwanted sound. Sound is defined as any pressure variation in air that the human ear can detect. If the pressure variations occur frequently enough (at least 20 times per second) they can be heard and are called sound. The number of pressure variations per second is called the frequency of sound, and is expressed as cycles per second, called Hertz (Hz).

Measuring sound directly in terms of pressure would require a very large and awkward range of numbers. To avoid this, the decibel scale was devised. The decibel scale uses the hearing threshold (20 micropascals of pressure), as a point of reference, defined as 0 decibels (dB). Other sound pressures are then compared to the reference pressure, and the logarithm is taken to keep the numbers in a practical range. The decibel scale allows a million-fold increase in pressure to be expressed as 120 dB. Another useful aspect of the decibel scale is that changes in decibel levels correspond closely to human perception of relative loudness as presented in Table 1.

The perceived loudness of sound is dependent upon many factors, including sound pressure level and frequency content. However, within the usual range of environmental noise levels, perception of loudness is relatively predictable, and can be approximated by weighing the sound level pressures between 1,000 and 5,000 Hz, which represent the most sensitive frequencies perceived by a healthy human ear and coincidentally the natural frequency range of human speech. This weighting network is referred to as the A-scale. There is a strong correlation between A-weighted sound levels (expressed as dBA) and community response to noise. For this reason, the A-weighted sound level has become the standard tool of environmental noise assessment. All noise levels reported in this NVIA are A-weighted. Table 1 provides sound pressure levels of typical noise sources in units of dBA and micropascals (μ Pa) of pressure.

Table 1 Typical A-Weighted Sound Levels of Common Noise Sources

Loudness Ratio	Micropascals (μPa)	dBA	Description
128	63,245,553	130	Threshold of Pain
64	20,000,000	120	Jet aircraft Take-Off at 100 feet
32	6,324,555	110	Riveting Machine at Operator's Position
16	2,000,000	100	Shotgun at 200 feet
8	632,456	90	Bulldozer at 50 feet
4	200,000	80	Diesel Locomotive at 300 feet
2	63,246	70	Commercial Jet Aircraft Interior During Flight
1	20,000	60	Normal Conversation Speech at 5-10 feet
0.5	6,325	50	Open Office Background Level
0.25	2,000	40	Background Level Within a Residence
0.125	632	30	Soft Whisper at 2 feet
0.0625	200	20	Interior of Recording Studio

Sources: (US EPA, 1971) and (Federal Interagency Committee on Noise, 1992).

Community noise is commonly described in terms of the ambient noise level, which is defined as the all-encompassing noise level associated with a given noise environment. A common statistical tool to measure the ambient noise level is the average, or equivalent, sound level (L_{eq}) over a given time period (usually one hour or less). The L_{eq} is also the foundation of the Community Noise Equivalent Level (CNEL) noise descriptor described below, which has a strong correlation with community response to noise. The maximum sound level (L_{max}) represents the highest instantaneous noise level recorded over a given time period (usually one hour or less), and can also be utilized to assess community noise impacts.

Community Noise Equivalent Level (CNEL) is based upon the average noise level over a 24-hour day, with a +5 decibel weighing applied to noise occurring during evening (7:00 p.m. to 10:00 p.m.) hours and a +10 decibel weighing applied to noise occurring during nighttime (10:00 p.m. to 7:00 a.m.) hours. These additions are made to account for the noise sensitive time periods during the evening and nighttime hours, when people are generally at home and more sensitive to sound. Because CNEL represents a 24-hour average, it tends to smooth out short-term variations in the noise environment. CNEL based noise standards are commonly used to assess noise impacts associated with variable noise sources, such as traffic, railroad and aircraft noise.

The maximum sound level (L_{max}) presents the highest instantaneous noise level recorded over a given time period (usually one hour or less). This value is useful as it can reveal short-term, intermittent noise sources (e.g., industrial equipment, etc.) within a noise environment, which would be lost with CNEL noise descriptor.

2.1.3 Characteristics of Groundborne Vibration

Vibration is like noise in that it involves a source, a transmission path, and a receiver. While vibration is related to noise, it differs in that noise is generally considered to be pressure waves transmitted through air, while vibration is usually associated with transmission through a structure. As with noise, vibration consists of an amplitude and frequency. A person's response to vibration depends on their individual sensitivity as well as the amplitude and frequency of the source.

Vibration can be described in terms of acceleration, velocity, or displacement. A common practice is to monitor vibration measures in terms of peak particle velocities (inches/second). Standards pertaining to perception as well as damage to structures have been developed for vibration in terms of peak particle velocity. At high enough amplitudes, ground vibration has the potential to damage structures and/or cause cosmetic damage (e.g., crack plaster). Ground vibration can also be a source of annoyance to individuals who live or work close to vibration-generating activities. Traffic, including heavy trucks traveling on a highway, rarely generates vibration amplitudes high enough to cause structural or cosmetic damage.

As vibrations travel outward from the source, they excite the particles of rock and soil through which they pass and cause them to oscillate by a few ten-thousandths to a few thousandths of an inch. Differences in subsurface geologic conditions and distance from the source of vibration would result in different vibration levels characterized by different frequencies and intensities. In all cases, vibration amplitudes would decrease with increasing distance. The maximum rate or velocity of particle movement is the commonly accepted descriptor of the vibration "strength." This is referred to as the peak particle velocity (PPV) and is typically measured in inches per second.

Human response to vibration is difficult to quantify. Vibration can be felt or heard well below the levels that produce any damage to structures. The duration of the event has an effect on human response, as does frequency. Generally, as the duration and vibration frequency increase, the potential for adverse human response increases.

Human and structural response to different vibration levels is influenced by a number of factors, including ground type, distance between source and receptor, duration, and the number of perceived vibration events. Table 2 displays the results of a 1974 study which relates human response to transient vibration sources (i.e., mining equipment) in terms of particle velocity (PPV) vibration levels.

Table 2 General Human Responses to Vibration Levels

Human Response to Vibration	Peak Vibration Threshold (in./sec. PPV)	
Severe	2.0	
Strongly perceptible	0.9	
Distinctly perceptible	0.24	
Barely perceptible	0.035	

Source: Transportation and Construction Vibration Guidance Manual (California Department of Transportation, 2013).

2.2 Physical Setting

This section describes noise sources in the areas around the Project site, the receptors of concern near the Project site and haul routes, and the existing ambient noise levels. Existing plant and excavation operations as well as haul truck activity are included in the baseline noise sources. The incremental increase in noise levels due to the Project is analyzed in this NVIA.

2.2.1 Project Site

The Project site is located in a rural area of unincorporated Ventura County. It is surrounded mainly by agricultural and open space land uses. The Conejo Mountain Memorial Cemetery and Funeral Home is located to the west of the Project site. The General Plan does not identify any other significant noise generating land uses in the immediate vicinity. The site is located on the southern flank of Conejo Mountain overlooking the Oxnard Plain, and separated from the Conejo Valley by the mountain crest. While the Camarillo Airport is a large source of noise in the south Camarillo area, it is approximately 4.5 miles to the northwest and has no appreciable influence on noise levels near the Project site.

As discussed above, the Pacific Rock Quarry is an existing aggregate mine and processing plant. The following operations are currently permitted under existing CUP 3817-3, and would not change as a result of the proposed Project:

- Daily maximum aggregate production rate.
- Number of daily truck trips and truck routes.
- Number of employees.
- Aggregate excavation and processing equipment (i.e., Aggregate Plant) and methods.
- Basting event operation hours, frequency, and methods.

These existing operations (e.g., aggregate excavation and processing, daytime haul truck activity, etc.) are considered baseline for this NVIA. Under CUP 3817-3, existing aggregate processing plant, excavation, and haul truck activities occur from 7:00 a.m. to 4:00 p.m., Monday through Sunday. Additionally, Condition #38 limits haul truck activity to sixty (60) truck loads per day (120 one-way trips) during normal operating hours. Both the existing truck trip limit and haul routes would not change as a result of the Project. Currently trucks leave the Project site

and travel down Howard Road/Pancho Road to Pleasant Valley Road, where they then either head north to U.S. State Highway 101 or south to State Route 1 (Pacific Coast Highway) for delivery to various locations (Figure 3).

The Ventura County 2040 General Plan identifies noise-sensitive land uses as including "residences; schools; historic sites; cemeteries; parks, recreation, and open space areas; hospitals and care facilities; sensitive wildlife habitats, including the habitat of rare, threatened, or endangered species; hotels and other short-term lodging (e.g., bed and breakfasts, and motels); places of worship; and libraries". As they would have the potential to experience different types of noise from the Project operations, noise-sensitive "receptors" considered in this NVIA have been separated into two categories based on whether they would have the potential to be impacted by 1) Project-related onsite industrial noise sources (i.e., "Non-Transportation" sources) or 2) Project-related offsite haul truck operations (i.e., "Transportation" sources). The receptors considered in this NVIA are described below. Note that, when possible, receptors are grouped and the noise impact at the worst-case portion of the group is determined. Figure 2 and Figure 3 (Appendix A) display the location of the receptors.

2.2.2 Vicinity Setting & Non-Transportation Receptors

As described above, the Project site is located in a semi-rural area of unincorporated Ventura County. Existing noise sources near the Project site receptors include equipment noise from Pacific Rock operations, noise from nearby agricultural operations, traffic noise from nearby roadways, and natural sounds (wind, plants rustling, birds/insects, etc.). Receptor 1 (R1), Receptor 2 (R2) and Receptor 3 (R3) within the vicinity of the Project site are described below.

- Receptor 1 (R1) is the Conejo Mountain Funeral Home to the west of the Project site. On the west side of the funeral home property, away from the Project site, is the funeral home building and on the east side, between the building and the Project site, is a grave yard. Noise sources near R1 include grounds keeping activities at the funeral home, cars on Howard Road, and nearby agricultural activities. Noise from the existing aggregate plant operations are faintly audible at R1 as background white noise.
 - Per the 2040 General Plan, the funeral home is considered a noise sensitive receptor.
- Receptor 2 (R2) collectively represents the group of residences to the east-southeast of the Project site, on the other side of the crest of the Santa Monica Mountain Range within the City of Thousand Oaks.
 Existing noise sources near R2 include cars on roads to the east, hikers passing by, and plants rustling in the wind.

Existing Project noise sources (i.e., excavation equipment, aggregate processing plant) were generally not audible in the area of R2. The ridge of the mountain blocks line-of-site between the Project site and residential receptors in this area. The mountain's large mass and height, in addition to the large distance between source and receptor, was noted to attenuate industrial noise to the point that it was not audible during visits to the site on December 20th and 21st, 2018. Though R2 does not have line-of-sight to existing operations, there is a potential for this receptor to have line-of-sight to new excavation areas, specifically mining activities expanding to the north. Please see Section 5.1 and Figures 4A, 4B, and 4C for more details regarding line-of-sight between the Project site and R2.

The R2 residences nearest to the expanded mining boundary were assessed, respectively located at the ends of Via Sandra and Via Pisa in the Dos Vientos Ranch community (Figure 2). Noise and vibration impacts are analyzed at the three (3) closest residences in this area, shown as R2-A, R2-B, and R2-C, and are meant to represent worst-case impacts for the entire receptor area. Due to the intervening mountains, noise impacts at R2 are less of a concern than vibration impacts, which travel more readily through solids.

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Receptor 3 (R3) collectively represents the various hiking trails located in open space areas to the southeast, east and northeast of the Project site. As compared to the other receptors considered in this evaluation (e.g., residences and funeral home), the open space area and trails are less frequently occupied and typically by fewer individuals. Nonetheless, R3 is analyzed as a representative "recreation/open space" sensitive receptor per the County's 2040 General Plan. Existing noise sources near R3 primarily include residential noise sources, periodic and variable buzzing of overhead transmission lines, and natural sounds (e.g., birds/insects, plants rustling in the wind, etc.). As with R2, existing Project noise sources were generally not audible during visits to the site on December 20th and 21st, 2018, as the ridge of the mountain blocks line-of-site between the existing operations and trails in this area. Please see Section 5.1 and Figure 4C, for more details regarding line-of-sight between the Project site and R3.

To determine worst-case noise and vibration impacts experienced by trail users, the portion of trails located closest to the Project site, specifically a location on the Powerline Trail, was analyzed (see Figure 2). As with R2, vibration impacts are evaluated at R3 as vibration from blasting could travel more readily through solids.

Ambient noise measurements were collected on December 20th and 21st, 2018 at Monitoring Locations 1 and 2 shown on Figure 2. Monitoring Location 1 is considered representative of noise levels at Receptors R2 and R3, and Monitoring Location 2 is considered representative of noise levels at Receptor R1. Noise generated by Pacific Rock's existing permitted activities (i.e., processing operations, mining operations, daytime haul truck activity, etc.) was captured within the ambient noise measurements as the site was operational during these days. The ambient noise measurements were collected by two (2) Quest DL SoundPro, Type 2 sound level meters equipped with random-incidence type microphones, windscreens and placed on tripods approximately 5-feet above ground level. Microphones were calibrated using Quest QC-10 calibrators before and after each measurement. Both long-duration (24-hour) and short-duration (15-minute) measurements were collected using A-weighted energy equivalent sound levels on a slow response time at 1-minute intervals for the long-duration measurements and 10-second intervals for the short-duration measurements.

To estimate evening and nighttime noise levels for certain receptors, measurements collected at the long-duration (24-hour) reference locations were compared to measurements at the short-duration (15-minute) monitoring locations during the same time of day to determine the dBA difference between the two points. For example, Monitoring Location 2 measurements (15-minute) collected between 3:37 p.m. and 3:52 p.m. when compared to noise levels collected at the Monitoring Location 1 24-hour reference point during the same time period show a noise level difference of -3.2 L_{eq} dBA. This difference between the measured values can be used as a correction factor, which is utilized to estimate the evening and nighttime L_{eq} 1H noise levels at short-duration monitoring locations. This same concept was also utilized to estimate daytime and nighttime noise levels at haul route Receptor 4 (R4). Please see Appendix C for additional details regarding these calculations.

The monitoring locations for R1 and R2/R3 are illustrated on Figure 2 (Appendix A). The results of ambient measurements collected at Monitoring Locations 1 and 2 as representative of the non-transportation receptors during the daytime, evening, and nighttime periods are summarized in Table 3. Complete noise measurement logs are included in Appendix C.

Space/Trails

Time Date(s) **Daytime Nighttime Evening** Receptor **Receptor Type** Measured Period(s) Leg 1H A, B Leq 1H A, B L_{eq}1H A, B Conejo Mountain R1 12/20/2018 Daytime 41.6 dBA 32.9 dBA 32.7 dBA Funeral Home Residence(s) & 12/20/2018 R2 & R3 Open 24-Hours 44.8 dBA 36.2 dBA 36.0 dBA 12/21/2018

Table 3 Ambient Monitoring Results @ Non-Transportation Receptors

Notes:

2.2.3 Regional Setting & Transportation Noise Receptors

The existing ambient noise environment near Project haul route (i.e., transportation) receptors is consistent with that of typical semi-urban/commercial areas. Existing noise sources include traffic on nearby roadways, agricultural operations, and commercial/industrial noise from facilities located on Pancho Road. Receptor 4 (R4) and Receptor 5 (R5) located within the vicinity of the Project haul routes are described below.

When considering a straight road segment, the noise levels are symmetrical on each side of the road and the same at any specified distance along the road (except near the ends of the road segment). For this reason, the nearest receptor to the road can be selected to conservatively represent noise impacts for a group of receptors (e.g. housing tract). In this NVIA, receptors were selected for each group of residences located near unique portions of the haul road geometry. These receptors (i.e., Receptors 4, 5A, 5B, and 5C) represent the worst-case impact for all receptors in that grouping. Figure 3 (Appendix A) shows the locations of the haul route receptors analyzed.

Receptor 4 (R4) is the residence located in unincorporated Ventura County, just south of the intersection
of Howard Road and Pancho Road. Noise sources near R4 primarily include nearby agricultural activities,
as it is surrounded by active agricultural operations on all sides.

Traffic noise generate by roadways to the north (e.g., Pleasant Valley Road, U.S. Highway 101, etc.) are faintly audible. Haul truck activity associated with the Project and surrounding agricultural operations are an infrequent but significant existing source of noise. This receptor generally has an unobstructed view of the Project haul route and passing trucks on Howard Road/Pancho Road. Due to the large distance between R4 and the Project site, existing aggregate plant and mining operations during the daytime are generally not audible from this location.

• Receptor 5 (R5) collectively represents the group of residences near the intersection of Pleasant Valley Road and Pancho Road within the City of Camarillo. Noise sources near R5 include cars on roads to the south and east (Pleasant Valley Road, U.S. Highway 101), as well as nearby agricultural and commercial operations. Pleasant Valley Road is a heavily trafficked roadway adjacent to R5, as it connects the U.S. Highway 101 to the north and the Pacific Coast Highway (State Route 1) to the south. Due to the large distance and intervening structures between R5 and the Project site, existing aggregate plant and mining operations are not audible from this location.

The residences nearest to the intersection, as well as one to the north and west, were assessed. Noise impacts are analyzed at the three (3) representative residences in this area, shown as R5-A, R5-B, and R5-C, and are meant to represent worst-case impacts for the entire receptor area. There is an existing 6-foot

A - Daytime = 6:00 a.m. - 7:00 p.m., Evening = 7:00 p.m. - 10:00 p.m., Nighttime = 10:00 p.m. - 6:00 a.m. (Ventura County, 2020).

B – Noise levels shown above were measured on 12/20/2018 and 12/21/2018. See Figure 2 (Appendix A) which shows the monitoring locations.

sound wall that runs adjacent to these receptors along the entire length of Pleasant Valley Road (see Figures 6 and 7).

Ambient noise measurements were collected at Receptors R4 and R5 on January 23rd and 24th 2019. Both long-duration (24-hour) and short-duration (15-minute) measurements were collected using A-weighted energy equivalent sound levels on a slow response time at 1-minute intervals for the long-duration measurements and 10-second intervals for the short-duration measurements. Additional detail regarding the monitoring results and calculations are included in Appendix C. Table 4 presents the existing ambient noise levels at representative receptors along the Project's haul routes.

Table 4 Ambient Monitoring Results @ Transportation Receptors

Pacantar	Receptor	Date(s)	s) Average Hour (L _{eq} 1H) ^{A, C}			CNEL
Receptor	Туре	Measured	Daytime	Evening	Nighttime	Outdoor
R4	Residence	1/23/2019 1/24/2019	59.8 dBA	50.7 dBA	47.9 dBA	58.9 dBA
R5	Residence(s)	1/23/2019 1/24/2019	77.4 dBA	66.3 dBA	65.4 dBA	62.2 dBA

Notes:

A – Daytime = 6:00 a.m. – 7:00 p.m., Evening = 7:00 p.m. – 10:00 p.m., Nighttime = 10:00 p.m. – 6:00 a.m. (Ventura County, 2020). These values are shown for informational purposes only.

B – CNEL = Sound levels measured during the evening hours (7:00 p.m. - 10:00 p.m.) are weighted by +5 dBA and sound levels measured during the nighttime hours (10:00 p.m. - 7:00 a.m.) are weighted by +10 dBA.

C – Noise levels shown above were measured on 1/23/2019 and 1/214/2019. See Figure 3 (Appendix A) which shows the monitoring locations.

Background noise levels at haul route receptors (R4 and R5) were also quantified using a computer model. Specifically, ambient noise levels were determined at R4 and R5 using a computer noise propagation model called SoundPLAN Essential 4.0. SoundPLAN Essential utilizes the same methods and algorithms as the Federal Highway Administration's *Traffic Noise Model* (TNM) to calculate noise impacts from traffic. In the TNM, a transportation noise source (e.g., Howard Road, Pleasant Valley Road, etc.) is input along with receptor locations to predict the noise levels associated with a specific vehicle trip count. Baseline traffic data collected by VRPA Technologies, Inc. (VRPA) on November 27th, 2018 and existing haul truck activity provided by Pacific Rock were input into the SoundPLAN Essential model to estimate background noise levels at haul route receptors. Table 5 presents the modeled background noise levels at haul route receptors. See Appendix E for additional information regarding this approach. Figure 6 in Appendix A displays the results of the baseline traffic noise model.

Table 5 Baseline Noise Modeling Results @ Transportation Receptors

Posentor	Posantar Tyna	Av	CNEL ^{B, C}		
Receptor	Receptor Type	Daytime	Evening	Nighttime	Outdoor
R4	Residence	53.2 dBA	34.6 dBA	25.6 dBA	50.3 dBA
R5-A	Residence(s)	59.9 dBA	53.8 dBA	49.8 dBA	59.7 dBA
R5-B	Residence(s)	60.2 dBA	54.7 dBA	50.6 dBA	60.3 dBA

Pacantar	Pocontor Type	Average Hour (L _{eq} 1H) ^{A, C}			CNEL ^{B, C}
Receptor	Receptor Type	Daytime	Evening	Nighttime	Outdoor
R5-C	Residence(s)	60.8 dBA	55.4 dBA	52.1 dBA	61.3 dBA

Notes: See Figure 6 (Appendix A) which shows the baseline noise levels modeled in SoundPLAN Essential.

- A Average L_{eq}1H: Daytime = 7:00 a.m. 7:00 p.m., Evening = 7:00 p.m. 10:00 p.m., Nighttime = 10:00 p.m. 7:00 a.m.
- B CNEL = Sound levels measured during the evening hours (7:00 p.m. 10:00 p.m.) are weighted by +5 dBA and sound levels measured during the nighttime hours (10:00 p.m. 7:00 a.m.) are weighted by +10 dBA.
- C Baseline noise levels shown were modeled in SoundPLAN Essential 4.0, using actual traffic data collected by VRPA on 11/27/2018 and haul truck activity provided by Pacific Rock.

When comparing the measured ambient noise levels in Table 4 to the modeled ambient noise levels in Table 5, the baseline noise levels modeled in SoundPLAN are lower than the ambient noise levels measured on January 23rd and 24th 2019. This is primarily because the SoundPLAN model only considers noise generated by vehicles on affected roadways, and excludes any other ambient noise sources (e.g., agricultural activities, nearby commercial centers, etc.) that exist in the areas around R4 and R5. Because the focus of this traffic noise analysis is to determine the impacts of new haul truck activity during the evening and nighttime hours, using the modeled baseline values is more appropriate. This is also a more conservative approach, as the lower numbers determined within the model present a lower baseline by which Project impacts are compared to (i.e., lower baseline means a greater chance for Project impacts).

2.3 Regulatory Setting

The regulatory setting consist of the Ventura County 2040 General Plan – Hazards and Safety Element, Noise (Ventura County, 2020), Ventura County Initial Study Assessment Guidelines (Ventura County, 2011), Ventura County Construction Noise Threshold Criteria and Control Plan (Ventura County, 2010), as well as applicable California Department of Transportation (Caltrans) and Federal Transit Administrations (FTA) guidance documents.

2.3.1 Ventura County General Plan Noise Element

The Ventura County 2040 General Plan – Hazards and Safety Element, Noise (Chapter 7.9) (Ventura County, 2020) presents standards for development of new noise-generating uses based on the noise sensitivity of a project's surroundings. The General Plan includes hourly (L_{eq}1H) significance thresholds for the daytime (6:00 a.m. to 7:00 p.m.), evening (7:00 p.m. to 10:00 p.m.), and nighttime (10:00 p.m. to 6:00 a.m.) hours. These hourly thresholds apply to "noise generators proposed to be located near any noise sensitive use". Noise sensitive uses include "residences; schools; historic sites; cemeteries; parks, recreation, and open space areas; hospitals and care facilities; sensitive wildlife habitats, including the habitat of rare, threatened, or endangered species; hotels and other short-term lodging (e.g., bed and breakfasts, and motels); places of worship; and libraries" as defined within the 2040 General Plan. A copy of the relevant 2040 General Plan text is included in Appendix B.

The 2040 General Plan also includes significance thresholds for sensitive receptors located near relatively continuous noise sources, such as roads, that use the Community Noise Equivalent Level (CNEL) noise metric. As defined in Section 2.1.1, CNEL describes noise impacts over a 24-hour period with penalties for noise generated during the evening (7:00 p.m. - 10:00 p.m.) and nighttime (10:00 p.m. - 7:00 a.m.) hours. The CNEL nighttime and daytime timeframes differ from the timeframes considered in the General Plan/CEQA Guidelines standards by one hour (CNEL daytime begins at 7:00 a.m. vs. 6:00 a.m. under the General Plan L_{eq} 1H standard). The CNEL

standard applies to transportation sources that vary over time and, per the General Plan Hazards and Safety Element, is the metric applied to Projects that cause traffic impacts to existing receptors.

In addition to the Ventura County 2040 General Plan criteria described above, the General Plan also presents a comprehensive land-use compatibility guideline graphic chart developed by the former California Office of Noise Control (CONC). This chart presents planning noise standards based on a sliding scale of impacts, ranging from "normally acceptable" to "clearly unacceptable" depending on the specific type of land use (e.g., residential, commercial, industrial, etc.) potentially impacted. While the land-use compatibility is not utilized to determine the significance of Project noise impacts, it is presented in this NVIA for information purposes. Please see Appendix B which presents the state land use compatibility chart taken from the Ventura County 2040 General Plan – Hazards and Safety Background Report (Chapter 11 – Hazards and Safety, Section 11.6 – Noise and Vibration, Table 11-10 – State Land Use Compatibility Standards for Community Noise Environment).

2.3.2 Ventura County Initial Study Assessment Guidelines

The Ventura County CEQA Guidelines (Ventura County, 2011) present methodologies for measuring noise levels and determining if their associated impacts are significant. Significance thresholds depend on ambient noise levels in the area of the project during each applicable time periods. If ambient levels are less than the thresholds, then the "fixed" thresholds are used. If ambient levels are greater than the fixed thresholds, then the "ambient noise +3 decibels (dB)" is used as the significance threshold. The CEQA Guidelines standards were used in the County General Plan described above (Appendix B).

The vibration thresholds referenced in the CEQA Guidelines are from the *Transit Noise and Vibration Impact Assessment Manual* (Federal Transit Administration, 2018), and apply to frequent vibration events from transportation sources (i.e., highways, rail lines, etc.), not blasting events. Therefore, the Caltrans vibration thresholds described below are utilized to determine the significance of infrequent vibration impacts resulting from blasting events.

2.3.3 Californian Department of Transportation

The *Transportation and Construction Vibration Guidance Manual* (California Department of Transportation, 2013) includes a chapter (Chapter 11) about blasting impacts assessment. In the absence of an established, local blasting vibration significance threshold guidance, criteria in the Caltrans manual are used to determine the significance of groundborne vibration in this NVIA.

2.3.4 Neighboring City Requirements

The Project site is located in unincorporated Ventura County, but has the potential to generate impacts at receptors located within the nearby cities of Camarillo and Thousand Oaks. Specifically, residential receptors (i.e., R1 and R2) and recreation/open space area receptor(s) to the east (i.e., R3) are located within the City of Thousand Oaks, and those located to the west (i.e., R5) are within the City of Camarillo.

The Thousand Oaks General Plan Noise Element (City of Thousand Oaks, 2000) includes land use planning standards for noise which are based on a sliding scale of impacts, where for low-density residential, 55 dBA CNEL is "clearly acceptable", 60 dBA CNEL is "normally acceptable", 65 dBA CNEL is "conditionally acceptable", and 75 dBA CNEL is "normally unacceptable". The Camarillo General Plan Noise Element (City of Camarillo, 2015) also includes similar sliding scale noise criteria. Specifically, for low-density residential, 60 dBA CNEL is "normally acceptable", 70 dBA CNEL is "conditionally acceptable", and 75 dBA CNEL is "normally unacceptable".

The Camarillo Municipal Code also contains specific noise regulations (Chapter 10.34). The Municipal Code includes significance thresholds for daytime (7:00 a.m. to 9:00 p.m.) and nighttime (9:00 p.m. to 7:00 a.m.) that are identical to the Ventura County General Plan thresholds for these same time periods. The only differences are the Municipal Code lacks a separate evening (7:00 p.m. – 10:00 p.m.) standard, and the daytime period begins one hour later (7:00 a.m.) while the nighttime period begins one hour earlier (9:00 p.m.) compared to the County's General Plan standards.

3.0 PROJECT DESCRIPTION

Pacific Rock has prepared a CUP Modification (LU10-003) application to modify their existing permitted operations under CUP 3817-3. Features of the Project that may affect the noise and vibration environment are as follows.

- Extend the life of the existing permitted operations.
- Expand the mining area boundary to the east, north, and south (Figure 2).
- Extend the operating schedule from six (6) to seven (7) days per week (to include material load out on Sundays).
- Allow additional material load out hours and a limited number of extended 24-hour operation days (60 days maximum per year).
- Operate a portable crushing and screening plant to recycle concrete debris (Recycle Plant).
- Install a structure for a 24-hour onsite security guard.

Table 6 summarizes and compares the existing and proposed operational parameters for the Project. Section 3.1 describes the onsite non-transportation noise and vibration sources (e.g., mobile excavation and stationary processing equipment, blasting, etc.) associated with the Project. Section 3.2 describes the offsite transportation sources of noise (i.e., haul truck activity on roadways) associated with the Project.

Table 6 Project Operational Parameters (Existing vs. Proposed)

A attiviture	Operational	Change		
Activity	Current Operations	Proposed Operations	Change?	
	CUP Boundary =	CUP Boundary =	+89 acres	
Excavation & Processing (e.g.,	115.5 acres	204.5 acres	(approx.)	
	7:00 a.m. – 4:00 p.m.	7:00 a.m. – 4:00 p.m.	No Chango	
aggregate excavation and	Monday – Saturday	Monday – Saturday	No Change	
processing, and use of	Daily Production =	Daily Production =		
explosives)	2,400 tons/day	2,400 tons/day	No Change	
	(mining/processing)	(mining/processing)		
Recycling Operations (crushing	None	7:00 a.m. – 4:00 p.m.	Now Operation	
and screening of concrete debris)	None	Monday – Saturday	New Operation	
Equipment Fueling and	5:30 a.m. – 10:00 p.m.	5:30 a.m. – 10:00 p.m.	No Chango	
Maintenance	Sunday – Saturday	Sunday – Saturday	No Change	
	Daily Limit =	Daily Limit =	No Change	
	120 trips/day	120 trips/day	No Change	
Truck Activity (use of water truck,	Haul Route = Howard	Haul Route = Howard		
material loading, entrance and	Road, Pancho Road,	Road, Pancho Road,	No Change	
exit)	Pleasant Valley Road	Pleasant Valley Road		
	7:00 a.m. – 4:00 p.m.	5:30 a.m. – 10:00 p.m.	Additional	
	Monday – Saturday	Sunday – Saturday	Hours & Days	
Limited 24-hour operations*	None	24 Hours	Now Operation	
(60 Days Maximum Per Year)	None	Sunday – Saturday	New Operation	

^{*} Extended processing and trucking is permitted for 60 days per year to satisfy Public Works, Caltrans, and other special/emergency projects that require nighttime deliveries. Daily truck trip limit (i.e., 120 trips/day) would remain unchanged during 24-hour emergency haul truck operations.

3.1 Non-Transportation Noise Sources

From a noise and vibration perspective, the primary onsite (i.e., non-transportation) modification proposed by this Project is the extension of the existing excavation operations to the east, north, and south (approximately 89 additional acres). This expansion would correct the existing slope conditions at the northerly and northeasterly side of the quarry, as well as expand onto recently acquired adjacent land. Aggregate excavation in these areas would be conducted in the same manner as currently occurs onsite, specifically by blasting and then pushing the loosened material over a steep slope. The following noise generating mobile equipment would continue to be used during aggregate excavation in the expanded mining areas. See Appendix D for additional information regarding the Project's onsite (i.e., non-transportation) mobile equipment noise sources:

Front-End LoaderExcavatorRock Drill

– Dozer – Water Truck

In addition to the expanded excavation boundary, Pacific Rock is proposing to operate a portable crushing and screening plant (Recycle Plant). The Recycle Plant would be used to recycle concrete debris into reusable materials. The portable Recycle Plant would operate in various locations within the center of the Project site, near the existing aggregate processing plant (see Figure 2). The Recycle Plant would operate during the same daytime time periods as the aggregate excavation and processing operations (Monday – Saturday, 7:00 a.m. – 4:00 p.m.). Please see Appendix D for additional information related to the Recycle Plant.

The Project also involves installation of a small structure to house a 24-hour onsite security guard. However, this Project component does not involve significance noise generating activities and would occur away from nearby receptors (Figure 2). Therefore, impacts from this proposed activity are not analyzed within this NVIA.

Although operations associated with the existing aggregate processing plant are considered part of the permitted baseline, and noise generated by this existing operation was captured in the ambient measurements (see Section 2.2), this source has also been included as a new Project source in the onsite noise analysis. Using this method is conservative, as generally existing sources operating while ambient measurements are collected are not also included within the Project impact calculations as this produces artificially high results. However, due to community concerns related to cumulative noise impacts, conservatively noise generated by the existing Aggregate Plant is also analyzed along with the other proposed onsite sources (i.e., mobile equipment in expanded mining areas, Recycle Plant). Please see Section 5.1 for more detail.

Since blasting activities are occasional and very short in duration (about 1-second), they do not have any substantial effect on the noise environment in the area. Blasting would continue to occur during daytime operating hours only (7:00 a.m. – 4:00 p.m.), and in the same manner as currently occurs onsite. Therefore, noise impacts from blasting operations are not assessed within this NVIA. Although noise impacts from blasting events are not analyzed, noise impacts from the rock drill, which is a noisy component of the blasting operations, is included in the onsite excavation noise assessment described above (Appendix D).

While blasting is not a concern from a noise perspective, it may have vibration impacts on the surrounding areas. This is especially true for Receptor 2 (R2) and Receptor 3 (R3) (see Section 2.2.2) since the Project proposes to extend excavation farther east towards this group of receptors. While the intervening mountains may help to reduce noise impacts at R2 and R3, vibration travels more readily through solids. Blasting activities would continue in the same manner as previously permitted. The following details describe the blasting process:

- 3-inch diameter holes to a depth of approximately 40-feet (rock drill utilized).
- 110 pounds of ammonium nitrate fuel oil (ANFO) explosives per hole.

- Larger blasts occur approximately twice a year and include 40 holes per blast.
- Smaller blasts occur a couple of times per week and include 10 holes per blast.
- There is a 5-millisecond delay between blasting in each hole.

3.2 Transportation Noise Sources

As part of the Project, Pacific Rock is requesting that Condition #38 be approved with this permit modification to continue allowing a maximum of 60 trucks (120 one-way trips) per day, and allow the Planning Director to authorize an increase in the maximum number of vehicles during emergencies. While the number of daily truck trips would not change, as shown in Table 6 the proposed haul truck operational hours have been expanded. Currently, haul truck activity occurs during daytime hours only (7:00 a.m. – 4:00 p.m.). Under the proposed Project, haul truck activity could occur during the additional nighttime hours of 5:30 a.m. to 7:00 a.m., daytime hours of 4:00 p.m. to 7:00 p.m., and the evening hours of 7:00 p.m. to 10:00 p.m. Additionally, the proposed 24-hour material load out and haul truck activities during emergency Public Works, Caltrans, or special projects also has the potential to produce noise impacts. Due to these extended haul truck hours, residential receptors along the Project haul route(s) may experience new noise impacts. Therefore, offsite haul truck noise impacts during the evening and nighttime hours are analyzed within this NVIA.

4.0 SIGNIFICANCE THRESHOLDS

According to the Appendix G Checklist in the CEQA Guidelines, a Project would have a significant environmental noise effect if it would result in the following:

- a) Generation of a substantial temporary or permanent increase in ambient noise levels in the vicinity of the project in excess of standards established in the local general plan or noise ordinance, or applicable standards of other agencies?
- b) Generation of excessive groundborne vibration or groundborne noise levels?
- c) For a project located within the vicinity of a private airstrip or an airport land use plan or, where such a plan has not been adopted, within two miles of a public airport or public use airport, would the project expose people residing or working in the project area to excessive noise levels?

4.1 Ventura County Noise Regulations & Significance Thresholds

As discussed in Section 2.3, Ventura County has adopted various guidelines, requirements and policies related to noise. Applicable Ventura County noise criteria are utilized to address CEQA Checklist item *a*). Specifically, the Ventura County 2040 General Plan – Hazards and Safety Element and CEQA Guidelines include significance thresholds for noise impacts at sensitive receptors, which state the following:

- (1) Noise sensitive uses proposed to be located near highways, truck routes, heavy industrial activities and other relatively continuous noise sources shall incorporate noise control measures so that:
 - a. Indoor noise levels in habitable rooms do not exceed CNEL 45; and
 - b. Outdoor noise levels do not exceed CNEL 60 or $L_{eq}1H$ of 65 dB(A) during any hour.
- (4) New noise generators, proposed to be located near any noise sensitive use, shall incorporate noise control measures so that ongoing outdoor noise levels received by the noise sensitive receptor, measured at the exterior wall of the building, does not exceed any of the following standards:
 - a. $L_{eq}1H$ of 55dB(A) or ambient noise level plus 3dB(A), whichever is greater, during any hour from 6:00 a.m. to 7:00 p.m.
 - b. $L_{eq}1H$ of 50dB(A) or ambient noise level plus 3dB(A), whichever is greater, during any hour from 7:00 p.m. to 10:00 p.m.
 - c. $L_{eq}1H$ of 45dB(A) or ambient noise level plus 3dB(A), whichever is greater, during any hour from 10:00 p.m. to 6:00 a.m.

Part (1) of this standard is primarily intended to be applied to new sensitive receptors (e.g., schools, residences, etc.) located next to existing noise sources (i.e., roads, railroads, highways). However, as discussed in Section 2.3.1, this standard may also appropriately be applied to projects that cause new traffic noise impacts to existing sensitive receptors. When assessing haul truck noise impacts over the course of a full operating day (e.g., 10.5-hours, 24-hours, etc.), the CNEL standard in Part (1) is more appropriate than the $L_{eq}(1H)$ standard, which only assesses noise impacts within a 1-hour timeframe. The CNEL standard also applies penalties to noise generated during the evening and nighttime hours, when receptors would be most sensitive to noise generated by new haul truck operations. Therefore, the standard presented in Part (1) from the County General Plan is utilized to determine the significance of noise impacts resulting from Project haul truck activity (i.e., transportation sources). Conversely, the $L_{eq}(1H)$ standards in Part (4) are more appropriate for the inconsistent noises generated by industrial equipment sources (i.e., mining equipment, Aggregate and Recycle Plant). For these reasons, the CNEL criteria presented in Part (1) is applied to transportation receptors located near the Project haul route (R4 and

R5), and the daytime, evening, and nighttime $L_{eq}(1H)$ criteria presented in Part (4) are applied to non-transportation receptors located near the Project site (R1, R2 and R3).

As discussed in Section 2.3.1, the Noise Element criteria are meant to apply "sensitive receptors", which are defined as "residences; schools; historic sites; cemeteries; parks, recreation, and open space areas; hospitals and care facilities; sensitive wildlife habitats, including the habitat of rare, threatened, or endangered species; hotels and short-term lodging (e.g., bed and breakfast, and motels); places of worship; and libraries". All of the receptors analyzed within this NVIA are considered "noise sensitive uses" per the 2040 General Plan, specifically "residences" (R2, R4 and R5), "parks, recreation, and open space" (R3) and "cemeteries" (R1). Therefore, the standards presented in Part (1) and Part (4) specific to these "noise sensitive uses" are applied to determine Project noise impacts.

In general, noise level changes of less than 3 dBA are not perceptible, and therefore 3+ dBA increase is commonly considered a "substantial increase" for the purposes of environmental noise assessment. This concept is used in Part (4) of the County standard to account for receptors where the existing background noise already exceeds the specified "fixed" criteria. Similarly, ambient plus 3+ dBA is also considered the significance criteria for Part (1) when the background CNEL noise levels exceed the specified standard. The applicable General Plan significance criteria are summarized in Table 7.

Table 7 Ventura County Noise Criteria

Inc	dustrial Source (Non-Tran	Traffic Source (Trans	portation) Criteria	
Time Period	Hours	Threshold L _{eq} (1H)	Outdoor	Indoor
Daytime	6:00 a.m. – 7:00 p.m.	55 dBA or ambient +3 dBA	CNEL = 60 dBA or	
Evening	7:00 p.m. – 10:00 p.m.	50 dBA or ambient +3 dBA	ambient +3 dBA L _{eq} (1H) = 65 dBA or	CNEL = 45 dBA or ambient +3 dBA
Nighttime	10:00 p.m. – 6:00 a.m.	45 dBA or ambient +3 dBA	ambient +3 dBA	

Source: Ventura County 2040 General Plan, Hazards and Safety Element, Noise (Chapter 7.9), September 2020.

Referring to monitoring results presented in Table 3, the ambient noise levels measured at the non-transportation receptors (R1 and R2/R3) are less than the applicable 1-hour ($L_{eq}1H$) "fixed thresholds" for the daytime, evening, and nighttime periods. Therefore, the "fixed thresholds" are utilized to determine the significance of Project noise impacts at Receptors R1, R2 and R3.

Table 8 Non-Transportation Significance Criteria

Receptor	Receptor Type	Daytime (L _{eq} 1H)	Evening (L _{eq} 1H)	Nighttime (L _{eq} 1H)
Receptor 1	Cemetery	55.0 dBA	50.0 dBA	45.0 dBA
Receptor 2	Residence(s)	55.0 dBA	50.0 dBA	45.0 dBA
Receptor 3	Recreation/Open Space	55.0 dBA	50.0 dBA	45.0 dBA

See Appendix C for more detail.

For the Project haul route receptors (R4 and R5), modeled ambient noise levels shown in Table 5 exceed the outdoor "fixed threshold" of 60 dBA CNEL at Receptor 5 (R5). Therefore, per Ventura County guidance, the ambient noise levels "+3 dBA" would be utilized to determine the significance of the Project's outdoor noise impacts at haul route Receptor(s) R5. These adjusted significance criteria for R5 (i.e., R5-A, R5-B, R5-C) are summarized in Table 9 below. Please note, the modeled ambient outdoor noise level at Receptor 4 (R4) was below the applicable "fixed" CNEL thresholds. Therefore, the "fixed thresholds" of 60 dBA CNEL is utilized to determine the significance of traffic impacts at R4.

Table 9 Transportation Significance Criteria

Receptor	Receptor Type	Outdoor (CNEL)
Receptor 4	Residence	60.0 dBA
Receptor 5-A	Residence(s)	62.7 dBA
Receptor 5-B	Residence(s)	63.3 dBA
Receptor 5-C	Residence(s)	64.3 dBA

Note: Per Standard (1) within the Ventura County Hazards and Safety Element, Noise Chapter, the "fixed" CNEL significance criteria is 60 dBA for outdoor noise levels experienced at sensitive receptors. See Appendix E for more detail.

4.1.1 Neighboring City Criteria Discussion

As discussed in Section 2.3.4, although the Project site is located in unincorporated Ventura County, some of the affected receptors are located within the nearby Cities of Camarillo and Thousand Oaks. Specifically, residential receptors (i.e., R1 and R2) and recreation/open space receptors (i.e., R3) to the east are located within Thousand Oaks, and those located to the west (i.e., R5) are within Camarillo.

The Thousand Oaks General Plan Noise Element (City of Thousand Oaks, 2000) and Camarillo General Plan Noise Element (City of Camarillo, 2015) include land use planning standards for noise based on a sliding scale of impacts. These standards are identical to the sliding scale found in the Ventura County General Plan land-use compatibility chart (Appendix B). Since the Ventura County Noise Element noise criteria are identical to Thousand Oaks and Camarillo General Plan criteria, the Ventura County standards are used to determine significance of noise impacts at Receptors R1, R2, R3 and R4 in this NVIA. Additionally, the County 1-hour (Leq1H) criteria during the daytime, evening, and nighttime periods is more stringent than the 24-hour CNEL standards, and is therefore conservative in comparison (i.e., impacts that do not exceed the Ventura County Leq1H thresholds would not exceed the Thousand Oaks/Camarillo Noise Element CNEL thresholds).

The Camarillo Municipal Code also contains specific noise regulations (Chapter 10.34). As some of the Project haul route receptors (i.e., R5) are located within the City limits, these standards may apply. The Municipal Code includes noise level limits for daytime (7:00 a.m. to 9:00 p.m.) and nighttime (9:00 p.m. to 7:00 a.m.) that are identical to the Ventura County General Plan thresholds for these same time periods (55 dBA and 45 dBA respectively). The only differences are the Municipal Code lacks a separate evening (7:00 p.m. – 10:00 p.m.) standard, and the daytime period begins one hour later (7:00 a.m.) while the nighttime period begins one hour earlier (9:00 p.m.) compared to the periods in the Ventura County General Plan standards. Due to the inclusion of a separate evening standard/penalty, on balance the Ventura County thresholds are more stringent than the Camarillo Municipal Code. Furthermore, because the residential receptor(s) located within the City of Camarillo are haul route receptors (i.e., R5), and exposed to relatively continuous noise sources, the 24-hour CNEL Noise

Element significance threshold is more appropriately applied. For this reason, the Ventura County 2040 General Plan – Hazards and Safety Element (Chapter 7.9 – Noise) CNEL thresholds shown in Table 9 are utilized to determine Project impacts at haul route receptors.

4.2 Vibration Significance Thresholds

While the CEQA Guidelines refer to thresholds in the FTA's *Transit Noise* and *Vibration Impact Assessment* (Federal Transit Administration, 2018), they are not appropriate to apply to blasting vibration. The *Transit Noise* and *Vibration Impact Assessment* thresholds are meant to be applied to transit sources that occur frequently throughout the day, which have a higher likelihood of causing damage and annoyance than infrequent, short duration (about 1-second) blasting events. For this reason, as discussed in Section 2.3.3, blasting specific thresholds from the Caltrans *Transportation and Construction Vibration Guidance Manual* (California Department of Transportation, 2013) are used to determine the significance of Project blasting vibration in this NVIA. This threshold criterion is utilized to address CEQA Checklist item *b*).

There are two (2) types of vibration significance thresholds, damage and annoyance. The damage thresholds are intended to prevent damage to structures while annoyance thresholds are intended to prevent annoyance to nearby residents. Table 22 in the Caltrans *Transportation and Construction Vibration Guidance Manual* includes a list of vibration levels and their effects on structures from a variety of sources. Table 10 below includes a number of these vibration levels. Note that a peak particle velocity (PVV) of 2.0 inches per second (in/sec) is utilized as the damage threshold in this NVIA.

Table 10 Vibration Structure Damage

Category	PPV (in/sec)
Equivalent to jumping on the floor	0.3
Equivalent to door slam	0.5
Equivalent to nail driving	0.9
No damage to a residential structure	<2.0
Probable damage to a residential structure	>4.0

Source: Table 22 within the Caltrans *Transportation and Construction Vibration Guidance Manual* (California Department of Transportation, 2013).

Table 11 presents the human response to blasting as described in the *Transportation and Construction Vibration Guidance Manual*. As there is a difference between perceptibility and annoyance, it is not appropriate to adopt a threshold of perceptibly to determine the significance of infrequent blasting events. The *Transportation and Construction Vibration Guidance Manual* indicates that "while a blaster can quite easily design his blasts to stay well below any vibration or air overpressure levels that could cause damage, it is virtually impossible to design blasts that are not perceptible by people in the vicinity." This NVIA conservatively uses the strongly perceptible PPV level of 0.50 in/sec to determine significance of blasting events from an annoyance standpoint.

Table 11 Human Response to Blasting Vibration

Average Human Response	PPV (in/sec)		
Barely to distinctly perceptible	0.02 – 0.10		
Distinctly to strongly perceptible	0.10 - 0.50		
Strongly perceptible to mildly unpleasant	0.50 – 1.00		
Mildly to distinctly unpleasant	1.00 – 2.00		
Distinctly unpleasant to intolerable	2.00 – 10.00		

Source: Table 21 within the Caltrans *Transportation and Construction Vibration Guidance Manual* (California Department of Transportation, 2013).

5.0 METHODOLOGIES

5.1 Assessment Methodologies – Non-Transportation Sources

As discussed in Section 3.1, the primary modification proposed by the Project from an onsite (i.e., "non-transportation") noise perspective is the expansion of the excavation operations. To determine impacts at nearby receptors, excavation equipment noise levels are quantified in this NVIA based on the expanded mining boundary. Figure 2 (Appendix A) shows the location of the expanded mine boundary and the nearest receptors of concern (R1, R2 and R3). To quantify the noise generated by the Project non-transportation noise sources (i.e., mobile mining and processing equipment), reference data from the *Construction Noise Threshold Criteria and Control Plan* (Ventura County, 2010) and the FHWA's *Roadway Construction Noise Model User Guide* (Federal Highway Administration, 2006) was utilized. Appendix D contains more detail related to the mobile equipment reference noise levels.

In addition to the expanded mining boundary, the Project would allow the operation of a portable Recycle Plant in various locations near the center of the Project site (Figure 2). The Recycle Plant would operate during the same daytime time periods as aggregate excavation and processing operations. As discussed in Section 3.1, although the aggregate processing plant (Aggregate Plant) is an existing permitted operation, conservatively noise generated by the plant has also been included in the non-transportation noise analysis. Figure 2 (Appendix A) displays the general area where the proposed portable Recycle Plant would operate onsite, as well as the existing location of the aggregate processing plant. As shown on Figure 2, the area closest to each receptor where the Recycle Plant could potentially operate was assessed to determine worst-case daytime noise impacts.

As discussed in Section 2.2.2, existing onsite operations are generally not audible at R2 and R3, and only faintly audible at R1. The ridge of the mountain and existing pit walls generally block line-of-site between the equipment noise sources and nearby receptors (R1, R2 and R3). A detailed analysis of the topography in and around the Project site was conducted for the three (3) residences that comprise Receptor 2 (i.e., R2-A, R2-B, and R2-C) and for the portion of the nearby hiking trail(s) represented by Receptor 3 (R3). Line-of-sight assessments were conservatively modeled from a second story vantage point (i.e., 15-feet above the ground surface) using current topographic data. As shown in Figure 4A and Figure 4C (Appendix A), Receptors R2-A, R2-C and R3 do not have line-of-sight to new Project excavation areas due to intervening mountains and topography. However, as shown on Figure 4B, Receptor 2-B (R2-B) is expected to have line-of-sight to three (3) new excavation areas located to the north. Figure 5 depicts these three (3) potential line-of-site (LoS) areas, shown as LoS-A, LoS-B, and LoS-C, in relation to Receptor 2-B. Receptor 2-B will potentially have direct line-of-sight to mobile equipment operating in these areas, and therefore no noise attenuation would result in this location due to the lack of intervening topography. Therefore, noise levels generated by mobile equipment operating within area LoS-A (Figures 4B and 5) were utilized to represent worst-case daytime noise impacts at Receptor 2-B (R2-B). Appendix D contains additional detail regarding the line-of-sight analysis.

Figure 2 (Appendix A) displays where the portable Recycle Plant would operate, specifically within the bottom of the existing excavation pit near the center of the Project site. The existing Aggregate Plant is also located in this area. Due to the intervening mountain range (Figures 4A, 4B, and 4C), residences at Receptor 2 and hikers at Receptor 3 are not expected to have line-of-sight to the existing Aggregate Plant or proposed Recycle Plant locations. Therefore, noise attenuation is assumed for these receptor-source combinations. Although it is anticipated that the existing excavation pit walls may shield views of the existing Aggregate Plant and proposed Recycle Plant from Receptor 1 (R1), this receptor may have line-of-sight to the top portions of the plant structures. Therefore, conservatively it is assumed that Aggregate Plant and Recycle Plant noise would not be attenuated at Receptor R1.

The non-transportation (i.e., onsite industrial noise sources) impact calculations (Appendix D) are based on the following conservative assumptions:

- As shown in Table 6, excavation and aggregate processing operations would continue to occur during the
 daytime hours only (7:00 a.m. 4:00 p.m.). The proposed Recycle Plant would also operate during
 daytime hours only. As such, only daytime noise impacts from onsite sources would occur at Receptors 1
 (R1), 2 (R2) and 3 (R3).
- The excavation equipment identified in Section 3.1 is conservatively assumed to operate simultaneously in the mining area or applicable line-of-sight (i.e., LoS) area closest to each receptor during the peak hour. This includes a loader, a dozer, an excavator, a rock drill, and a water truck. This is conservative because in reality not all mobile equipment would operate simultaneously in a single physical location closest to each receptor. For example, the rock drill is only used prior to blasting events, and would therefore operate separately from the other mobile equipment.
- Noise levels associated with the existing Aggregate Plant and proposed portable Recycle Plant are based on field measurements collected by Sespe Consulting, Inc. in 2020 from comparable rock crushing/recycling operation in Otay Mesa, California. This data was utilized in a previous Sespe study with a similar crushing/recycle plant (Sespe Consulting, Inc., 2020). In addition to the plants, when these measurements were collected at the Otay Mesa facility other ancillary equipment, specifically haul trucks and two (2) loaders, were also operating nearby simultaneously. Therefore, the noise levels measured at this facility represent a conservative overestimation of the noise generated by Pacific Rock's existing Aggregate Plant and proposed Recycle Plant. Appendix B contains relevant source noise references for the existing Aggregate Plant and proposed Recycle Plant.
- Noise impacts at receptors are calculated using standard logarithmic propagation equations from the guidance documents. These equations are also found in the applicable guidance documents published by the Federal Transit Administration (FTA) and Caltrans. This equation uses a logarithmic scale, with an approximate noise propagation factor of 6 decibels (dB) per doubling of distance.
 - Six (6) decibels per doubling of distance is accepted as the appropriate propagation factor for environmental noise impact assessments. As explained in the Caltrans *Technical Noise Supplement* (California Department of Transportation, 2013), sound from a small localized source radiates uniformly outward as it travels away from the source in a spherical pattern. The sound level attenuates or drops off at a rate of 6 dBA for each doubling of the distance. This decrease, resulting from the geometric spreading of the energy over an ever-increasing area, is referred to as the inverse square law. Appendix B presents an excerpt from the Caltrans guidance document.
- Peak 1-hour (L_{eq}1H) excavation noise levels are conservatively calculated when excavation equipment is
 operating as close to the affected receptors as possible (Figure 2) or within closest area with direct lineof-sight to the affected receptor (Figure 5).
- When looking at onsite noise impacts from mobile equipment (i.e., excavation operations), the crest of intervening mountain ranges between the equipment sources and R2 (i.e. R2-A, R2-B, R2-C) and R3 is estimated to provide -10 dBA of attenuation. Please see the barrier insertion loss calculations in Appendix D, which quantify the amount on noise attenuation expected due to the mountain ridge blocking line-of-sight between source and receptor. This is true of Receptors R2-A (Figure 4A) and R2-C and R3 (Figure 4C) as the intervening topography blocks their line-of-sight to the mine expansion areas. However, there is a potential for line-of-sight to exist between portions of the extended excavation areas and R2-B in three (3) locations to the north (Figure 4B). The closest excavation area with line-of-sight to R2-B is approximately 1,652 feet away (Figure 5). Mining in this area (i.e., LoS-A) would result in worst-case noise

impacts to receptor R2-B and is therefore utilized to determine significance (see calculations in Appendix D). As the excavation proceeds, the mass and height of intervening mountains would increase between the source and receptor resulting in increased noise attenuation and lower impacts at R2-B.

- For the existing Aggregate Plant and proposed Recycle Plant, the crest of intervening terrain between the plant locations and R2 and R3 also completely blocks line-of-sight between the noise sources and these receptors. Figures 4A, 4B, and 4C (Appendix A) show the line-of-sight geometry, and Appendix D which contains barrier insertion loss calculations. Therefore, an attenuation of -10 dBA is assumed at Receptor 2 (R2) and 3 (R3) when analyzing noise impacts from the Aggregate Plant and Recycle Plant operations. However, as described above, there is a potential for line-of-sight to exist between the Recycle Plant and R1. Therefore, no attenuation is assumed for the plant noise levels at R1.
- Total vibration impacts from blasting activities are determined in this NVIA based on the International Society of Explosives Engineers Blasters' Handbook, 17th Edition (International Society of Explosives Engineers, 1998), the blasting parameters described in Section 3.1, and the closest distance between the blasts and the receptors. The vibration equation presented in the Blasters' Handbook is identical to the equation outlined in applicable Caltrans guidance documents (California Department of Transportation, 2013).

Limited evening and nighttime activities are proposed as part of the Project to satisfy potential Public Works, Caltrans, and other special or emergency projects. However, only load out to haul trucks would be conducted during any extended evening and nighttime operations. Excavation and aggregate processing activities would continue to occur during daytime hours only (see Section 3.0). Nighttime load out activities would potentially impact R1 but, because the funeral home is only occupied during the daytime hours, noise during limited evening and nighttime operations would not adversely affect this land use. Additionally, these limited evening and nighttime operations would also not impact R2 and R3 due to the large distance between the truck loading area and receptors (minimum 2,295 feet), as well as the minimum -10 dBA attenuation provided by the intervening terrain. As discussed in Section 2.2.2, the existing aggregate processing plant and truck loading noise was not audible at R2 and R3 during previous site visits.

5.2 Assessment Methodologies – Transportation Sources

Project traffic/transportation noise impacts at receptors located along haul routes would result from aggregate delivery haul trucks on public roads. Project transportation noise was assessed using the SoundPLAN Essential 4.0 model software. As discussed in Section 2.2.3, SoundPLAN Essential uses the FHWA's *Traffic Noise Model* (TNM) algorithm to predict traffic noise impacts. Baseline traffic data on affected roadways was collected by VRPA Technologies, Inc. (VRPA) by measuring actual vehicle counts measured over a 24-hour period on November 27th, 2018. As discussed in Section 3.2, the daily haul truck trips associated with the Project would not change from existing levels (i.e., 60 loads/day, 120 one-way trips/day).

Total traffic count was modeled with SoundPLAN by combining the actual traffic counted by VRPA with estimated average hourly haul truck activity from the Pacific Rock Quarry. Specifically, SoundPLAN estimates that the existing daily truck trips (120 truck trips/day) would be spread evenly throughout the current operating day hours (i.e., average of 13 truck trips/daytime hour). Per the existing CUP, haul truck activity is limited to occur between 7:00 a.m. and 4:00 p.m. only. Using the methodology described above and average hourly baseline traffic data during the daytime, evening, and nighttime time periods, the SoundPLAN Essential model was used to calculate the baseline CNEL noise levels at Receptors R4 and R5 located along the haul route. Figure 6 (Appendix A) displays the results of the baseline noise model.

While the daily number of haul truck trips would not change from existing permitted levels (i.e., 60 loads/day, 120 one-way trips/day), the time period truck trips may occur would change. Specifically, allowing proposed 24-hour haul truck activity to satisfy Public Works, Caltrans, and other special/emergency projects would result in redistribution of daytime haul truck trips to periods during the evening and nighttime hours. The Project was modeled in SoundPLAN Essential assuming that all truck trips occur evenly throughout the evening (7:00 p.m. – 10:00 p.m.) and nighttime (10:00 p.m. – 7:00 a.m.) hours (i.e., average of 5 truck trips per evening/nighttime hour). This is conservative, as the CNEL noise metric adds the greatest penalty/weight to noise generated during these time periods (+5 dBA for evening noise, +10 dBA for nighttime noise).

Project noise impacts at haul route receptors (R4 and R5) were modeled over a 24-hour period (CNEL) for both the existing and proposed Project trip scenarios. The cumulative incremental noise impacts at each receptor are then compared to the appropriate criteria to determine significance.

6.0 PROJECT-LEVEL IMPACTS & MITIGATION MEASURES

6.1 Generation of Noise Levels in Excess of Applicable Standards

Impact Statement

Impact NO-1: Generation of a substantial temporary or permanent increase in ambient noise levels in the vicinity of the project in excess of standards established in the local general plan or noise ordinance, or applicable standards of other agencies? **(Appendix G Threshold Criteria (a))**

6.1.1 Non-Transportation Impact Analysis

To address the CEQA Criteria *a*) for non-transportation source noise impacts, the applicable Ventura County 1-hour (L_{eq}1H) noise criteria presented in Table 8 are utilized. Non-transportation (i.e., onsite) Project noise impacts are presented and compared to the applicable significance thresholds in Table 12 below. For the reasons discussed in Section 5.1, these impacts are conservative as the vast majority of Project operation days would result in lesser impacts. Where line-of-site between receptor and noise source is blocked, a -10 dBA attenuation factor was applied. Specifically, a -10 dBA attenuation is applied at R2-A, R2-C and R3 due to the intervening topography blocking line-of-sight between the expanded excavation operations and receptors. Please see Appendix D for applicable barrier insertion loss calculations. This attenuation factor was also applied to proposed Recycle Plant and existing Aggregate Plant noise impacts at Receptors R2 (i.e., R2-A, R2-B, and R2-C) and R3 due to the mountain ridge blocking line-of-sight. Figure 4A, 4B and 4C (Appendix A) show the line-of-sight assessments for R2 and R3 receptors respectively. See Appendix D for assumptions and noise impact calculation.

Table 12 Non-Transportation Noise Impacts & Significance Determination (Prior to Mitigation)

Parameter	1-Hour (L _{eq} 1H) — Noise Level (dBA)					
rafaffieter	R1	R2-A	R2-B	R2-C	R3	
Baseline Noise Level (dBA):	41.6	44.8	44.8	44.8	44.8	
Mobile Excavation Equipment Noise Impacts						
Distance to Equipment Source (feet) ^D :	1,160	1,161	1,652	943	390	
Noise Reduction due to Shielding (dBA) ^A :		-10		-10	-10	
Equipment Noise Level (L _{eq} 1H) @ Receptor (dBA):	59.8	49.8	56.7	51.6	59.2	
Aggregate Plant Noise Impacts						
Distance to Equipment Source (feet):	2,474	2,728	2,781	2,703	2,201	
Noise Reduction due to Shielding (dBA) ^A :		-10	-10	-10	-10	
Equipment Noise Level (L _{eq} 1H) @ Receptor (dBA):	55.2	39.4	39.2	39.4	41.2	
Recycle Plant Noise Impacts						
Distance to Equipment Source (feet):	1,833	2,547	2,688	2,580	1,955	
Noise Reduction due to Shielding (dBA) ^A :		-10	-10	-10	-10	
Equipment Noise Level (L _{eq} 1H) @ Receptor (dBA):	52.8	40.0	39.5	39.8	42.3	
Total Non-Transportation Equipment Noise Impacts & Significance Determination						
Cumulative Noise Level (L _{eq} 1H) @ Receptor (dBA) ^c :	61.0	51.6	57.1	52.8	59.6	
Applicable Significance Threshold (dBA) ^B :	55.0	55.0	55.0	55.0	55.0	
Significant?	Yes	No	Yes	No	Yes	

Footnotes (see Table 12 on previous page):

- A See Figure 4A (Receptor 2-A) and Figure 4C (Receptor 2-C and 3) which show the line-of-sight assessment for these receptors.
- B Significance threshold shown are the Ventura County General Plan/CEQA Guidelines "fixed" noise standards for daytime hours (6:00 a.m. 7:00 p.m.). Onsite non-transportation operations would occur during daytime hours only (i.e., 7:00 a.m. 4:00 p.m.), and therefore only the daytime L_{eq} 1H criteria applies.
- C The total Project noise level represents the cumulative worst-case noise level experienced at Receptors R1, R2 and R3 due to operation of onsite equipment sources (i.e., mobile excavation equipment, proposed Recycle Plant, existing Aggregate Plant) operating simultaneously within a given hour.
- D As shown on Figure 5, the mining area with direct line-of-sight to Receptor 2-B is approximately 1,652-feet away. Mobile excavation equipment operating within this area (i.e., LoS-A) will produce worst-case noise impacts at R2-B and is therefore utilized to determine the significance of impacts.

As shown in Table 12 above, the predicted peak hour Project noise levels ($L_{eq}1H$) exceed the Ventura County General Plan/CEQA Guidelines daytime $L_{eq}1H$ noise threshold at Receptor 1 (R1), Receptor 2-B (R2-B), and Receptor 3 (R3) due to expanded excavation activities as well as the existing Aggregate Plant and proposed Recycle Plant operations. Therefore, unmitigated noise impacts at R1, R2-B, and R3 due to onsite non-transportation sources are considered potentially significant. Please refer to the following section for the recommended mitigation measures.

Level of Significance Before Mitigation

Potential for a significant noise impact is predicted at Receptor 1 (R1), Receptor 2-B (R2), and Receptor 3 (R3).

Mitigation Measures

As shown in Table 12, peak one hour (L_{eq}1H) Project noise levels from onsite non-transportation sources (i.e., expanded excavation operations, existing Aggregate Plant, proposed Recycle Plant) exceed the applicable Ventura County daytime significance criteria at Receptors 1 (R1), 2-B (R2-B) and 3 (R3). Therefore, to ensure noise generated by onsite non-transportation equipment sources does not exceed applicable significance thresholds at Receptors R1, R2-B and R3, the following mitigation measures are recommended. Please see Appendix D for more details regarding the proposed mitigation measures.

- NO-1. Blasting, excavation, and materials processing and recycling activities shall continue to occur during daytime operation hours (7:00 a.m. to 4:00 p.m.) only.
- NO-2. Excavation equipment (loader, dozer, excavator, rock drill, water truck) shall be fitted with a manufacturer's approved exhaust muffler.
- NO-3. Excavation equipment, including the drill rig, shall not idle for more than 30 minutes at any one time.
- NO-4. The existing Aggregate Plant and proposed Recycle Plant shall not operate simultaneously for any time period.
- NO-5. Neither the proposed Recycle Plant and nor the existing Aggregate Plant shall operate when excavation is occurring within 1,600-feet of the Conejo Mountain Funeral Home (Receptor 1).
- NO-6. The predicted noise impacts associated with onsite excavation equipment shall be verified with noise level measurements upon commencement of mining activities within line-of-sight of Receptor 1 (R1) and Receptor 2-B (R2-B). In the event that actual noise levels exceed the assumptions contained within this analysis, additional noise control measures shall be implemented.

Based on information in the Ventura County Construction Guidelines (Ventura County, 2010) and EPA's *Noise from Construction Equipment and Operations, Building Equipment, and Home Appliances* (U.S. Environmental Protection Agency, 1971) presented in Appendix B, the above mitigations have been determined to be sufficient to reduce the noise impacts to less than significant at nearby receptors. Furthermore, Pacific Rock has advised Sespe during preparation of this report that the above mitigations are feasible.

At Receptor 1 (R1), in some instances combined noise levels resulting from operations of mobile equipment and one of the processing plants (i.e., Aggregate or Recycle Plants) still exceed applicable Ventura County standards, even with the implementation of Mitigation Measures NO-1 through NO-4. Additional noise propagation calculations show that Project noise impacts at this receptor will fall below the applicable Ventura County General Plan daytime significance threshold if neither the proposed Recycle Plant nor the existing Aggregate Plant are operational when mining equipment is operating within 1,600-feet of the receptor (see Appendix D). Figure 8 depicts the potential excavation areas located less than 1,600-feet away from R1. Therefore, if excavation is occurring within 1,600 feet of R1, neither the Recycle Plant nor the Aggregate Plant shall be operated (as required by Mitigation Measure NO-5) to ensure Project noise impacts at R1 are less than significant.

Table 13 below presents the mitigated noise levels expected while onsite operations are occurring in the Project site areas closest to or within line-of-sight of the affected receptors. The mitigated noise levels for excavation equipment are based on the EPA's Noise from Construction Equipment and Operations, Building Equipment, and Home Appliances (Appendix B), which notes that installation of a manufacturer's "improved muffler" on each equipment's "exhaust" would result in a "probable noise reduction" of -10 dBA. The EPA document also notes these quieter equipment noise levels are obtainable by "implementing noise control features requiring no major redesign or extreme cost" (Appendix B). Since the exhaust stack is considered the dominant noise component on the front-end loader, dozer, and excavator (Appendix D), a -10 dBA reduction is assumed for these pieces of equipment due to the installation of an improved muffler. For the rock drill and water truck, conservatively it is assumed this control measure would achieve a -5 dBA noise reduction, as the exhaust stack is the secondary noisy component for these equipment pieces (Appendix D). Use of an improved muffler is also presented in the Ventura County Construction Guidelines as a feasible mitigation option, which states using "quieter methods or equipment and implementing feasible noise controls" can reduce equipment noise impacts. The Ventura County Construction Guidelines includes the EPA's mitigated equipment noise levels by reference. See Appendix B for applicable Ventura County/EPA mitigation references, and Appendix D for mitigated equipment noise levels and mitigated noise impact calculations.

Table 13 Non-Transportation Noise Impacts & Significance Determination with Mitigation

Receptor	Unmitigated Noise Level (L _{eq} 1H) @ Receptor (dBA) ^A	Mitigated Noise Level (L _{eq} 1H) @ Receptor (dBA) ^D	Ventura County Significance Criteria (dBA) ^B	Significant?
Receptor 1 (R1) ^c	61.0	54.9	55	No
Receptor 2-A (R2-A)	51.6	47.8	55	No
Receptor 2-B (R2-B)	57.1	51.4	55	No
Receptor 2-C (R2-C)	52.8	48.5	55	No
Receptor 3 (R3)	59.5	53.5	55	No

Notes (also see following page):

A - Prior to mitigation, noise impacts at R2-A and R2-C were shown to be below the applicable significance thresholds due to intervening

topography (see Table 12). However, since the proposed mitigation measures would apply to all excavation equipment, the mitigated noise levels at these receptors are also shown here for informational purposes.

B – Significance threshold shown is Ventura County General Plan noise criteria for daytime hours (6:00 a.m. – 7:00 p.m.).

C – While mitigated noise levels at R1 appear very close to the 55 dBA significance threshold, this is due to the design of the proposed mitigation measures. With mitigations NO-4 and NO-5 implemented, worst case noise impacts at R1 would occur when only mobile mining equipment (no Recycle and Aggregate Plant operations per Mitigation Measure NO-5) is operating and excavation is occurring within 1,600 feet of the receptor (this scenario produces an estimated 54.9 dBA level at R1). The majority of excavation operations would occur further than 1,600-feet from R1, and therefore noise impacts would usually be below those shown in Table 13. Please see the calculations in Appendix D for more detail.

D – Mitigated noise levels at receptors R1, R2 and R3 take into account predicted noise reductions resulting through the implementation of Mitigation Measures NO-1 through NO-4, while mitigated noise levels shown for R1 also take into account reductions resulting from the implementation of Mitigation Measure NO-5. Please see Appendix D for additional detail.

As shown in Table 13, non-transportation noise sources are expected to have a less than significant impact at Receptors R1, R2 and R3 with mitigation incorporated. It is also important to note this study was designed to produce conservative worst-case Project noise impacts to nearby receptors. For example, inclusion of the existing Aggregate Plant as a new noise source represents a conservative assumption. In reality, when taking into account the shielding or absorption effects from intervening topography/vegetation between source and receptor, as well as the fact that most excavation and processing operations will not occur simultaneously, near the outermost Project site boundary or within direct line-of-sight of affected receptors, as was assumed in this analysis, noise levels are expected to be less than those calculated within this NVIA. Furthermore, as mining progresses to a final depth and the pit walls deepen, additional noise attenuation can be assumed.

Per Mitigation Measure NO-6, to ensure noise impacts to nearby noise-sensitive receptors to the west (R1) and east (R2-B) are not significant, reference sound levels associated with onsite excavation and processing equipment would be verified through noise level measurements upon commencement of mining and processing activities in areas within line-of-sight of Receptor 1 and Receptor 2-B (Figure 5).

Level of Significance After Mitigation

Upon implementation of Mitigation Measures NO-1 through NO-6 described above, Project non-transportation impacts to nearby Receptors R1, R2 and R3 would be less than significant as shown in Table 13.

6.1.2 Transportation Impact Analysis

To address the CEQA Criteria *a)* for transportation Project impacts, prediction of noise impacts from Project transportation sources (i.e., haul trucks) is addressed in this section. Project traffic noise would result from aggregate delivery haul trucks on public roadways. Project traffic noise impacts on affected road segments of Howard Road, Pancho Road, and Pleasant Valley Road (Figure 3) were modeled using SoundPLAN Essential compute software. Please see Section 5.2 which summarizes the assumptions and methodologies utilized in the traffic noise model.

Figure 6 and Figure 7 (Appendix A) display the results of both the baseline and Project road noise model respectively. Table 14 summarizes the predicted cumulative CNEL noise levels experienced by the Project haul route Receptors R4 and R5 under the baseline and Project conditions. Haul truck noise impacts are below the applicable Ventura County Noise Element significance criteria. Please see Appendix E for more details regarding the transportation noise model and resulting impact assessment.

Table 14 Transportation Noise Level & Significance Determination

Parameter	R4 (CNEL – dBA)	R5-A (CNEL – dBA)	R5-B (CNEL – dBA)	R5-C (CNEL – dBA)
Baseline Outdoor Noise Level	50.3	59.7	60.3	61.3
Total Project Outdoor Noise Level	55.2	61.1	61.4	61.6
Significance Threshold	60.0	62.7	63.3	64.3
Significant?	No	No	No	No

See Figure 6 and Figure 7 (Appendix A) and the model output files in Appendix E for more detail.

Level of Significance Before Mitigation

Less than significant.

Mitigation Measures

None required.

Level of Significance After Mitigation

Not Applicable.

6.2 Generation of Excessive Groundborne Vibration

Impact Statement

Impact NO-2: Generation of excessive groundborne vibration or groundborne noise levels? (Appendix G Threshold Criteria (b))

As discussed in Section 5.1, total vibration impacts from blasting activities are determined in this NVIA based on the International Society of Explosives Engineers *Blasters' Handbook*, 17th Edition (International Society of Explosives Engineers, 1998), assuming the closest distance between the blasts and the receptors. See Appendix F for the calculations and additional information.

Blasting vibration impacts at Receptors R1, R2 and R3 are presented and compared to the applicable Caltrans significance criteria in Table 15. These estimates are conservative, as it assumes the blasts occur within the Project site area closest to each receptor. Blasting would continue to be conducted during the daytime hours only (see Section 3.1). Table 15 presents the results of the blasting vibration analysis in terms of peak particle velocity (PPV). Note that the peak blasting vibration impact would only slightly increase at R1 and R2 (R2-A, R2-B, R2-C) above the threshold of perception (i.e., 0.02 in/sec) due to the Project. While predicted vibration levels are slightly higher at R3, this may be considered acceptable due to the transitory use of the open space area and trails and the fact that no permanent structures are found in this location. Please see Appendix F for the vibration impact calculations.

Table 15 Peak Project Vibration Impacts and Significance Determination

Receptor	Project Vibration Impact – PPV (in/sec)	Structure Damage Threshold – PPV (in/sec)	Significant?	Annoyance Threshold – PPV (in/sec)	Significant?
Receptor 1	0.086	2.0	No	0.50	No
Receptor 2-A	0.086	2.0	No	0.50	No
Receptor 2-B	0.050	2.0	No	0.50	No
Receptor 2-C	0.120	2.0	No	0.50	No
Receptor 3	0.492	2.0	No	0.50	No

See Appendix F for more detail.

Level of Significance Before Mitigation

Less than significant.

Mitigation Measures

None required.

Level of Significance After Mitigation

Not Applicable.

6.3 Airport & Airstrip Vicinity Analysis

Impact Statement

Impact NO-3: For a project located within the vicinity of a private airstrip or an airport land use plan or, where such a plan has not been adopted, within two miles of a public airport or public use airport, would the project expose people residing or working in the project area to excessive noise levels? **(Appendix G Threshold Criteria (c))**

Impact Analysis

The proposed Project site is not located within the vicinity of a private airstrip or within 2.0 miles of any public airports or public airstrips. As discussed in Section 2.2, the closest airport/airstrip is Camarillo Airport, located approximately 4.5 miles northwest of the Project site. Additionally, per Figure 11-15 within the Ventura County 2040 General Plan – Health and Safety Background Report (Ventura County, 2020), neither the Project site nor the affected receptors are located within the CNEL contour areas for the Camarillo Airport, or the Point Mugu Naval Air Station located approximately 7.2 miles away to the southwest (see Figure 11-15 – Camarillo Airport Noise Contours and Figure 11-17 – NAWS at Point Mugu Noise Contours; Ventura County 2014 General Plan – Health and Safety Background Report, Section 11.6 – Noise and Vibration). Therefore, the Project would have no impact related to public or private airport/airstrip noise levels.

Level of Significance Before Mitigation

No impact.

Mitigation Measures

None required.

Level of Significance After Mitigation

Not Applicable.

7.0 FINDINGS

This NVIA finds that:

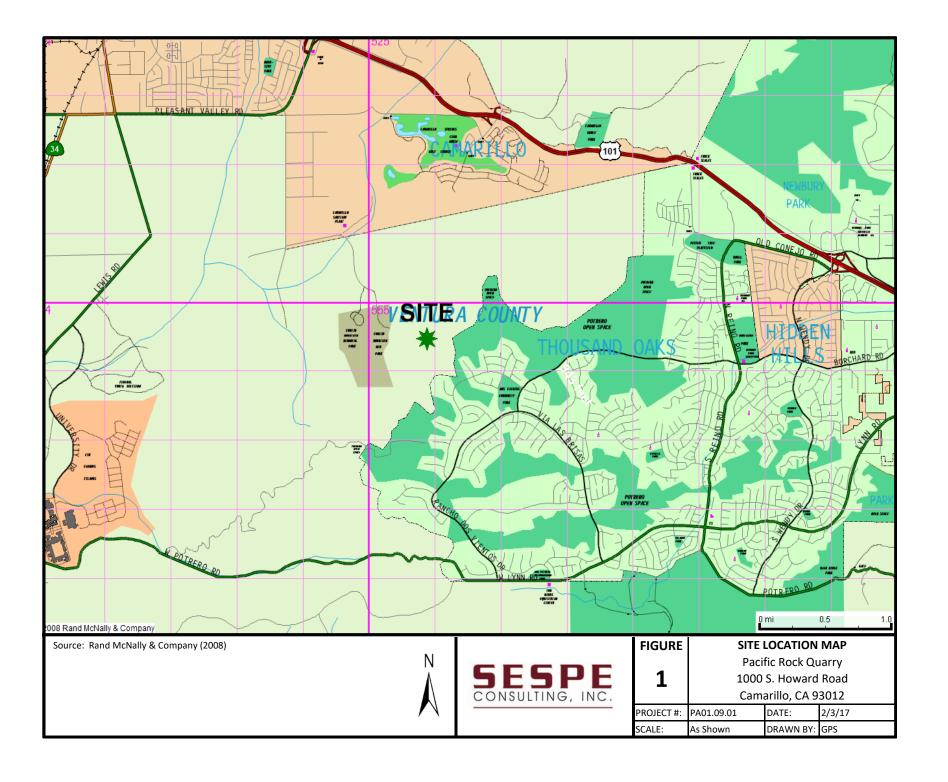
- Noise impacts from onsite sources ("Non-Transportation") are less than significant after mitigation.
- Noise impacts from traffic sources ("Transportation") are less than significant.
- Groundborne vibration impacts are less than significant.
- The Project would result in a Class II impact, significant but mitigable to less than significant levels.

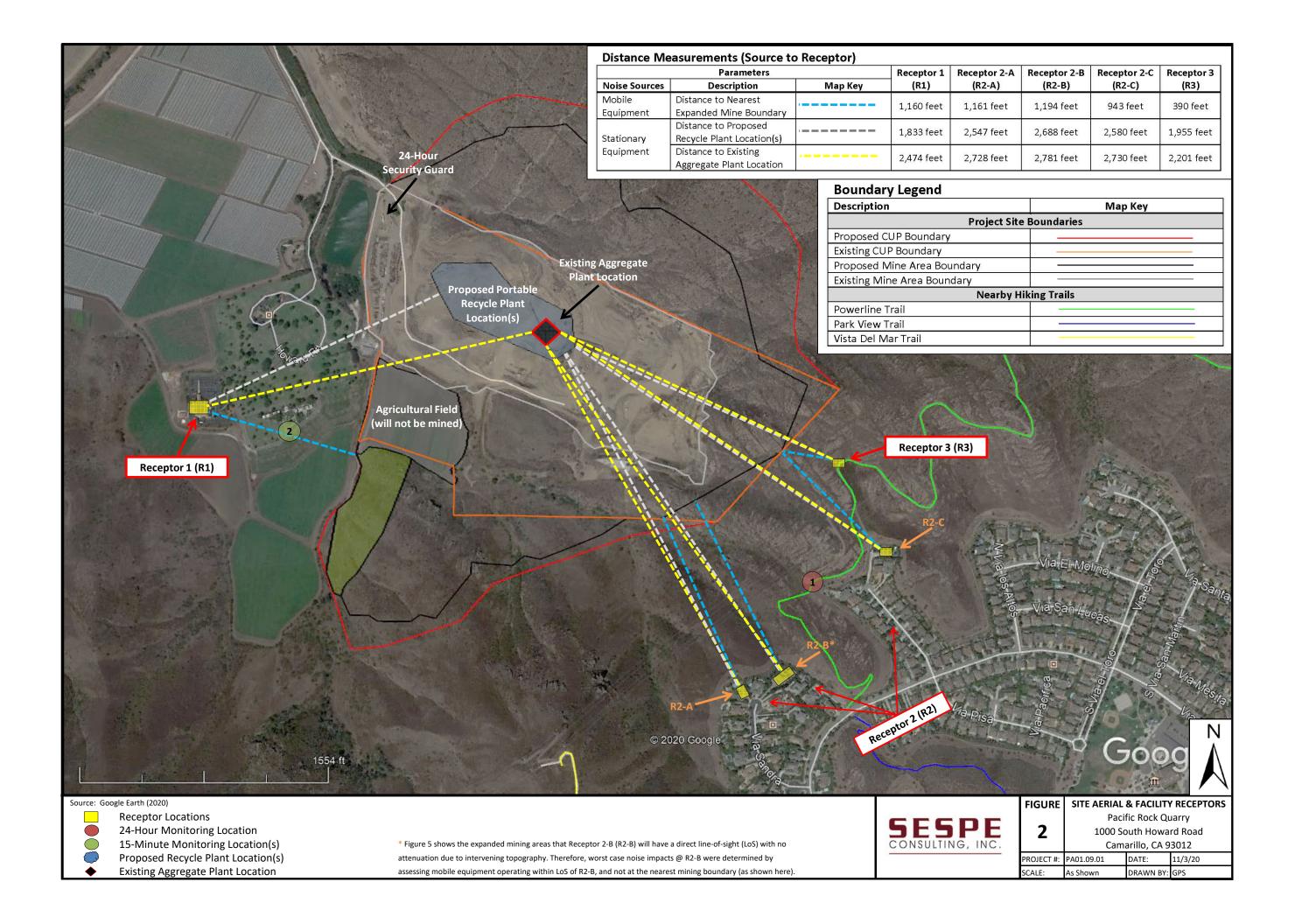
8.0 REFERENCES

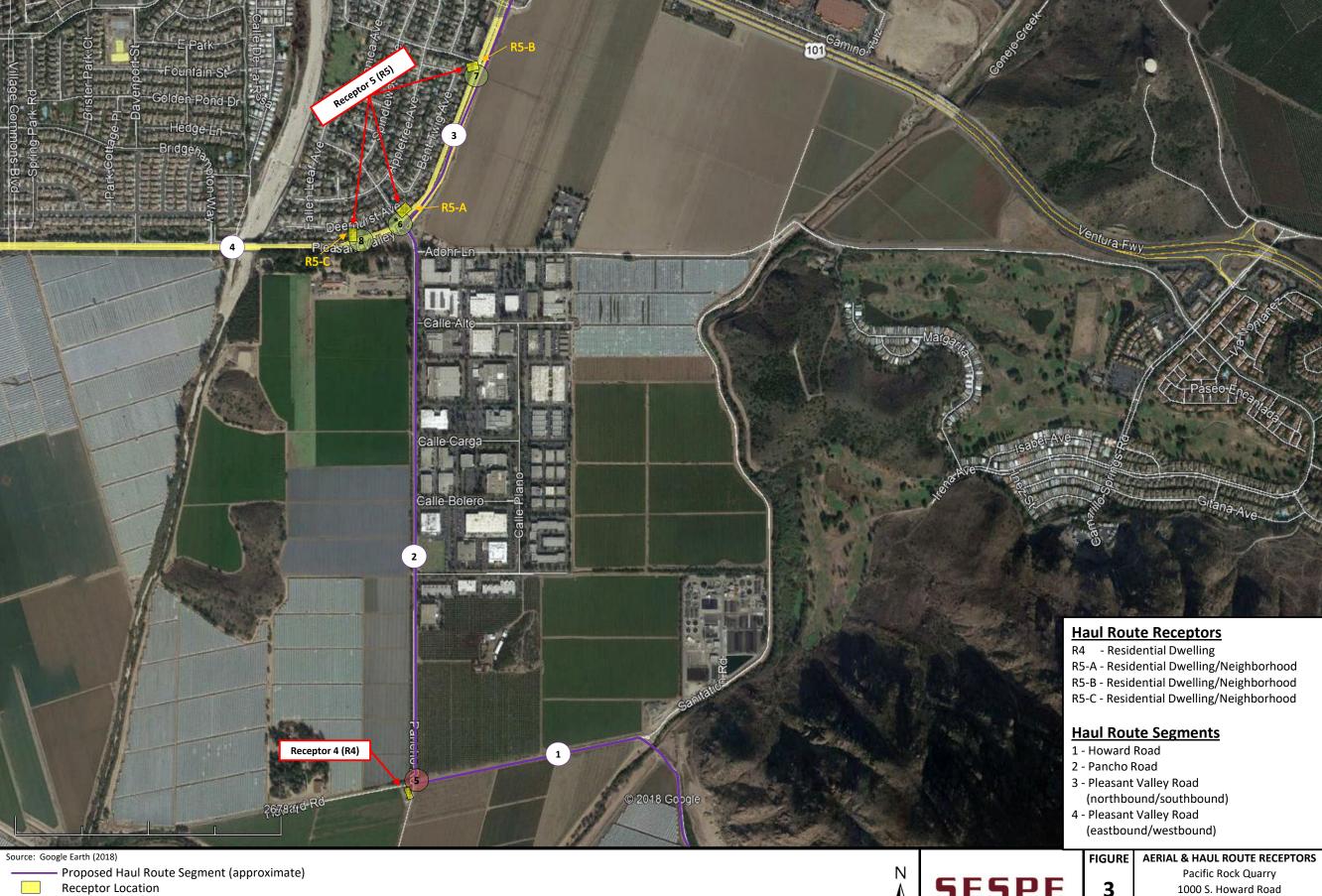
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Noise and Groundborne Vibration Impact Assessment	LU10-0003 Modification Application Pacific Rock Quarry
	APPENDIX A
	FIGURES







24-Hour Monitoring Location

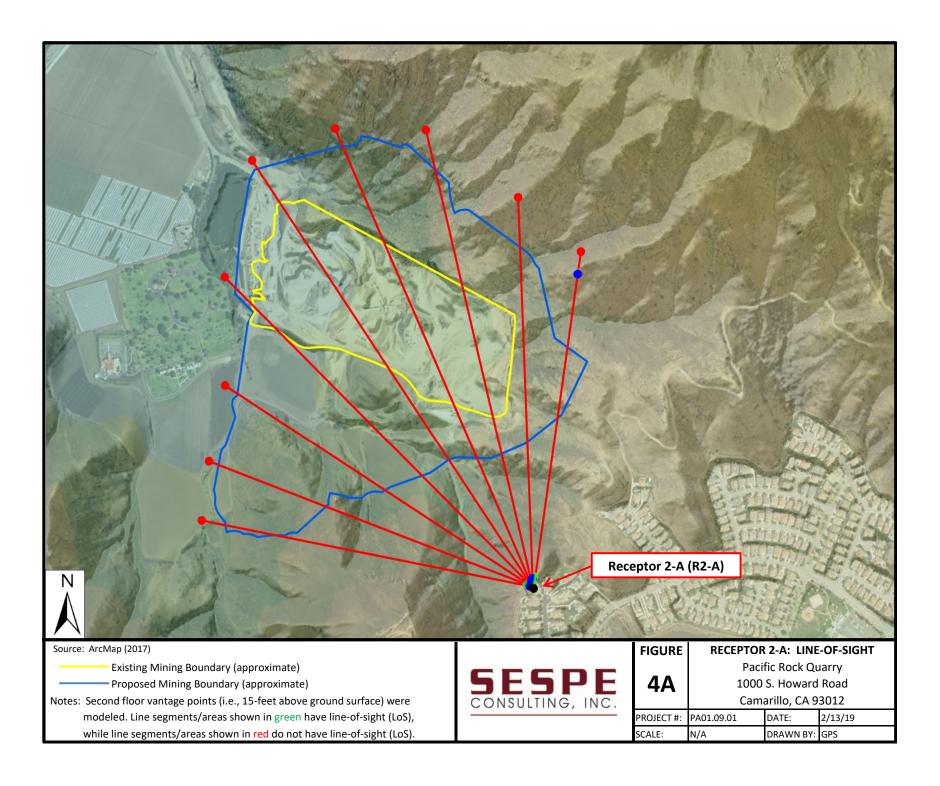
15-Minute Monitoring Locations

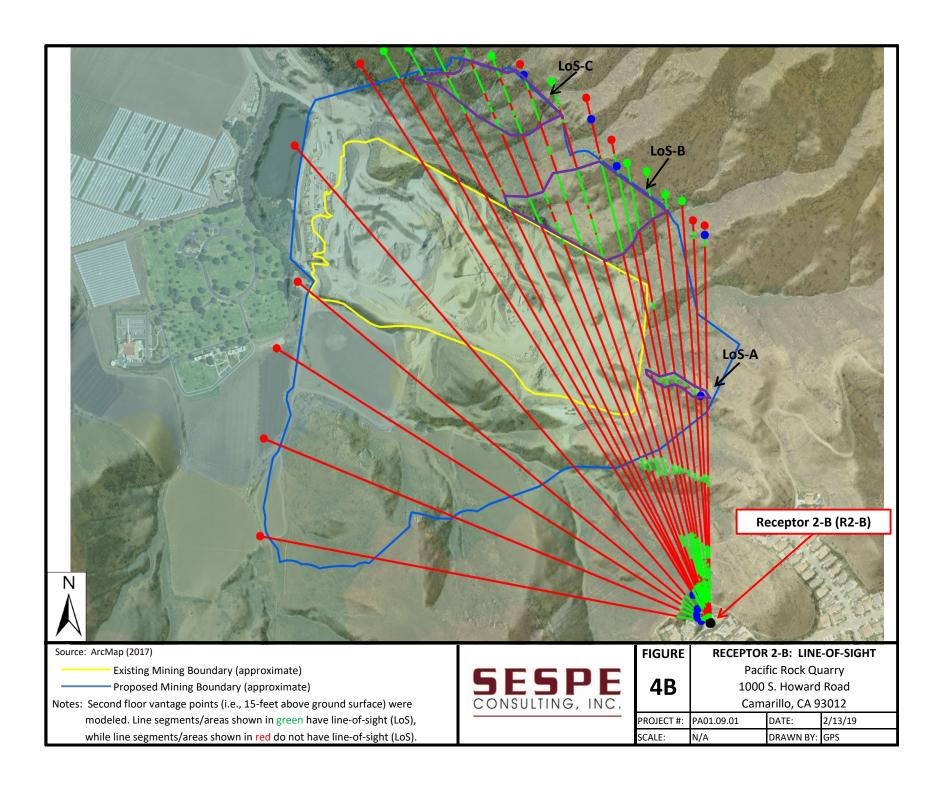
Three (3) separate 15-minute measurements were collected at Monitoring Location #6

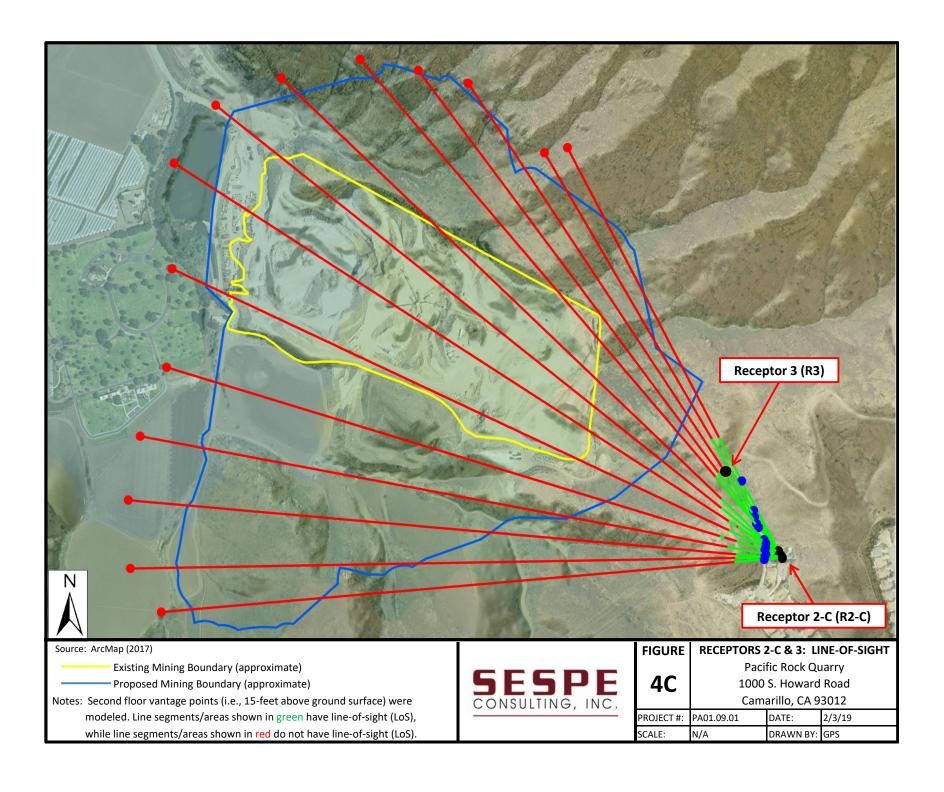


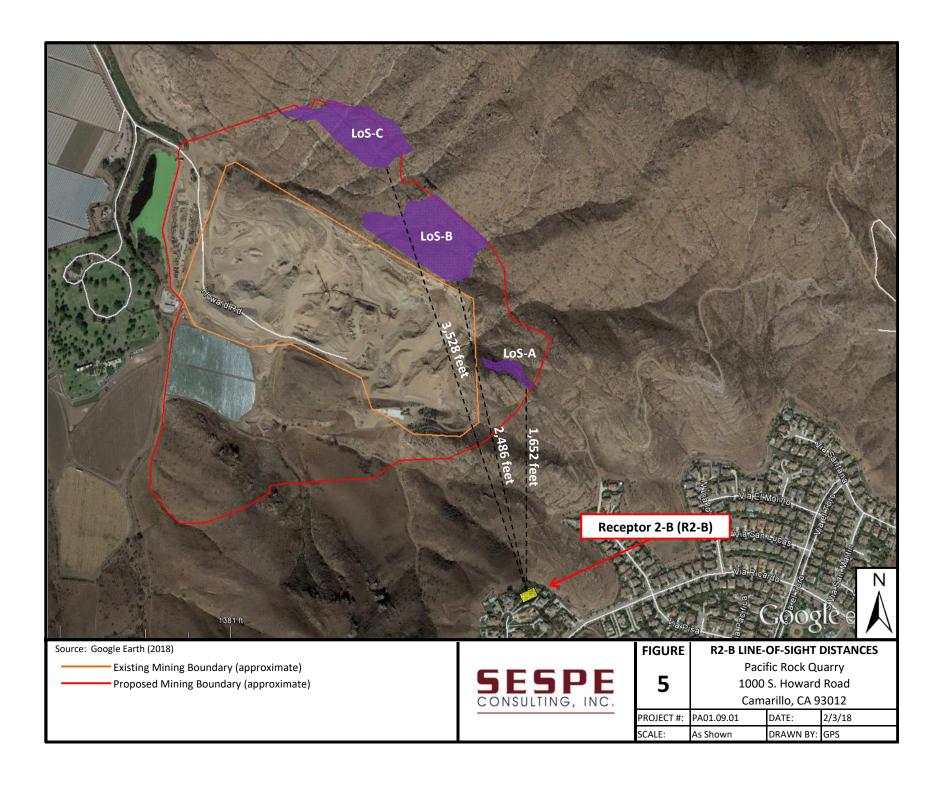
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PROJECT #:	PA01.09.01	DATE:	12/27/18		
SCALE:	As Shown	DRAWN BY:	GPS		



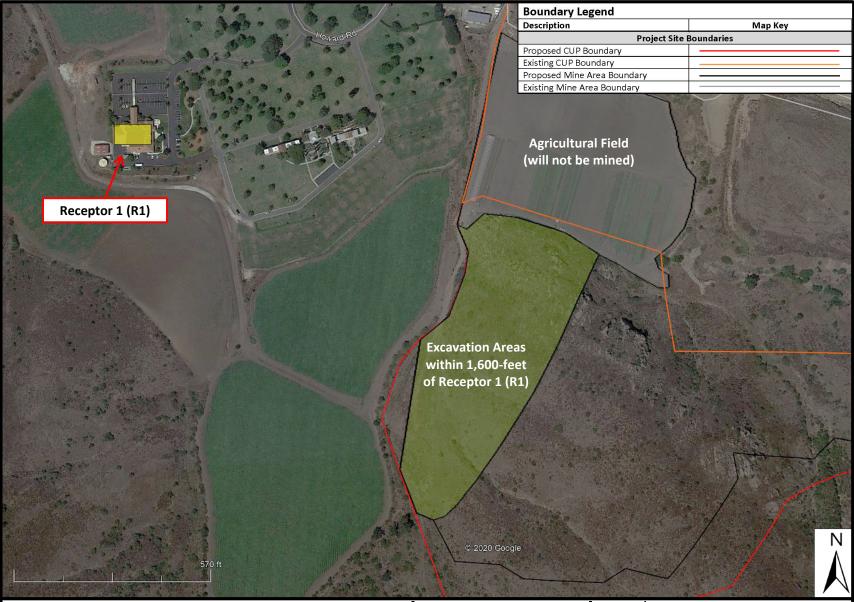












Source: Google Earth (2020)

Note: To ensure Project noise levels at Receptor 1 (R1) comply with the applicable General Plan noise criteria, neither the existing Aggregate Plant or the proposed Recycle Plant shall operate at any time when excavation is occurring within 1,600-feet of Receptor R1. Please see Mitigation Measure NO-5 and the propagation calculations presented in Appendix D.



FIGURE	MITIGATION MEASURE NO-5			
	Pacific Rock Quarry 1000 S. Howard Road Camarillo, CA 93012			
l 8				
PROJECT #:	PA01.09.01	DATE:	11/3/20	
SCALE:	As Shown	DRAWN BY:	GPS	

Noise and Groundborne Vibration Impact Assessment	LU10-0003 Modification Application Pacific Rock Quarry
	APPENDIX B
	REGULATORY REFERENCES



7.9 Noise

The predominant sources of noise in the county include traffic noise on major roadways, transit and freight trains, and aircraft. In addition to the information provided in Section 11.6, "Noise and Vibration," of the Background Report on existing conditions, Table 7-1 includes the calculated future noise levels at 50 feet from County roadways, as well as distances to the 60, 65, and 70 dBA CNEL noise contours for all modeled roadways.

Noise-sensitive land uses are generally considered to include those uses where noise exposure could result in health-related risks to individuals, as well as places where quiet is an essential element of their intended purpose. These uses include: residences; schools; historic sites; cemeteries; parks, recreation, and open space areas; hospitals and care facilities; sensitive wildlife habitats, including the habitat of rare, threatened, or endangered species; hotels and other short-term lodging (e.g., bed and breakfasts, and motels); places of worship; and libraries.

HAZ-9

To protect the health, safety, and general welfare of county residents by striving to eliminate or avoid the adverse noise impacts on existing and future noise sensitive uses.

HAZ-9.1 Limiting Unwanted Noise

The County shall prohibit discretionary development which would be impacted by noise or generate project-related noise which cannot be reduced to meet the standards prescribed in Policy Haz-9.2. This policy does not apply to noise generated during the construction phase of a project. (SO)

HAZ-9.2





Noise Compatibility Standards

The County shall review discretionary development for noise compatibility with surrounding uses. The County shall determine noise based on the following standards:

- New noise sensitive uses proposed to be located near highways, truck routes, heavy industrial activities and other relatively continuous noise sources shall incorporate noise control measures so that indoor noise levels in habitable rooms do not exceed Community Noise Equivalent Level (CNEL) 45 and outdoor noise levels do not exceed CNEL 60 or Leq1H of 65 dB(A) during any hour.
- 2. New noise sensitive uses proposed to be located near railroads shall incorporate noise control measures so that indoor noise levels in habitable rooms do not exceed Community Noise Equivalent Level (CNEL) 45 and outdoor noise levels do not exceed L10 of 60 dB(A)
- 3. New noise sensitive uses proposed to be located near airports:
 - Shall be prohibited if they are in a Community Noise Equivalent Level (CNEL) 65 dB or greater, noise contour; or
 - b. Shall be permitted in the Community Noise Equivalent Level (CNEL) 60 dB to CNEL 65 dB noise contour area only if means will be taken to ensure interior noise levels of CNEL 45 dB or less.

- 4. New noise generators, proposed to be located near any noise sensitive use, shall incorporate noise control measures so that ongoing outdoor noise levels received by the noise sensitive receptor, measured at the exterior wall of the building, does not exceed any of the following standards:
 - a. Leq1H of 55dB(A) or ambient noise level plus 3dB(A), whichever is greater, during any hour from 6:00 a.m. to 7:00 p.m.;
 - b. Leq1H of 50dB(A) or ambient noise level plus 3dB(A), whichever is greater, during any hour from 7:00 p.m. to 10:00 p.m.; and
 - c. Leq1H of 45dB(A) or ambient noise level plus 3dB(A), whichever is greater, during any hour from 10:00 p.m. to 6:00 a.m.
- 5. Construction noise and vibration shall be evaluated and, if necessary, mitigated in accordance with the Construction Noise Threshold Criteria and Control Plan (Advanced Engineering Acoustics, November 2005). (RDR)

Table 7-1 Projected 2040 Noise Levels and Contours

		Noise (dBA CNEL) at 50 feet from	Noise Contour Distance in Feet		
	Corridor and Segment		60 dBA	65 dBA	70 dBA
	Roadways	T			
1	Aggen Road north of Los Angeles Avenue (SR 118)	55.0	32	10	3
2	Balcom Canyon Road south of South Mountain Road	58.2	65	21	7
3	Balcom Canyon Road north of Los Angeles Avenue (SR 118)	57.1	51	16	5
4	Bardsdale Avenue east of Sespe Street	56.7	47	15	5
5	Beardsley Road north of Central Avenue	62.8	190	60	19
6	Box Canyon Road south of Santa Susana Pass Road	59.3	86	27	9
7	Bradley Road north of Los Angeles Avenue (SR 118)	62.2	166	52	17
8	Briggs Road south of Telegraph Road	62.9	197	62	20
9	Briggs Road north of Telegraph Road	58.8	75	24	8
10	Bristol Road west of Montgomery Avenue	65.9	387	123	39
11	Broadway Road west of Grimes Canyon Road (SR 23)	61.0	125	40	13
12	Burnham Road south of Baldwin Road (SR 150)	57.7	59	19	6
13	Burnham Road east of Santa Ana Road	57.3	54	17	5
14	Calle Yucca north of Camino Manzanas	54.2	26	8	3
15	Camino Dos Rios west of Lynn Road	57.2	52	17	5
16	Canada Larga Road east of Ventura Avenue	54.4	28	9	3
17	Casitas Vista Road west of Ojai Freeway (SR 33)	58.6	72	23	7
18	Center School Road south of Los Angeles Avenue (SR 118)	56.2	42	13	4
19	Center Street (Piru) west of Telegraph Road (SR 126)	54.7	29	9	3
20	Central Avenue west of Ventura Freeway (US 101)	67.9	619	196	62
21	Central Avenue west of Santa Clara Avenue	67.9	620	196	62
22	Central Avenue east of Vineyard Avenue (SR 232)	64.5	284	90	28
23	Channel Islands Boulevard west of Rice Avenue	68.4	693	219	69
24	Creek Road east of Country Club Drive	55.8	38	12	4
25	Creek Road east of Ventura Avenue (SR 33)	62.6	181	57	18
26	Donlon Road north of La Cumbre Road	52.0	16	5	2
27	Doris Avenue east of Victoria Avenue	64.9	311	98	31







		Noise (dBA CNEL) at 50	Noise Cont	our Distanc	e in Feet
	Corridor and Segment	feet from Roadway	60 dBA	65 dBA	70 dBA
28	El Roblar Drive west of Maricopa Highway (SR 33)	57.7	58	18	6
29	Etting Road east of Dodge Road	62.0	159	50	16
30	Fairview Road east of Maricopa Highway (SR 33)	51.4	14	4	1
31	Fairway Drive north of Valley Vista Drive	57.3	53	17	5
32	West Fifth Street east of North Harbor Boulevard	59.6	92	29	9
33	Foothill Road west of Peck Road	61.1	128	40	13
34	Foothill Road west of Briggs Road	56.2	42	13	4
35	Foothill Road east of North Wells Road	62.1	161	51	16
36	Foothill Road east of Saticoy Avenue	63.3	211	67	21
37	Gonzales Road east of North Harbor Boulevard	63.3	213	67	21
38	Grimes Canyon Road north of Los Angeles Avenue (SR 118)	61.5	142	45	14
39	Guiberson Road east of Chambersburg Road (SR 23)	57.7	58	18	6
40	Harbor Boulevard north of Gonzales Road	70.6	1,153	365	115
41	Harbor Boulevard south of Gonzales Road	70.3	1,074	340	107
42	Howe Road east of Torrey Road	51.6	14	5	107
43	Hueneme Road east of Las Posas Road	67.1	512	162	51
44	Hueneme Road east of Nauman Road	66.9	495	156	49
45	Hueneme Road east of Wood Road	66.2	417	132	49
46	Hueneme Road east of Olds Road	68.7	746	236	75
47	Kanan Road east of Lindero Canyon Road	66.6	460	145	46
48	Kanan Road east of Elitero Canyon Road Kanan Road east of Hollytree Drive/Oak Hills Drive	66.6	454	143	45
49	Kanan Road south of Tamarind Street	68.2	667	211	67
50	La Luna Avenue south of Lomita Avenue	56.4	44	14	4
51	Laguna Road east of Pleasant Valley Road	60.4	109	34	11
52	Las Posas Road north of East Fifth Street (SR 34)	67.7	587	186	59
53	Las Posas Road Horti of East Fifth Street (SR 34)	67.8	601	190	60
54	Las Posas Road south of Hueneme Road	65.6	361	114	36
55	Lewis Road south of Pleasant Valley Road	69.0	788	249	79
56	Lewis Road south of Preasant Valley Road	67.9	617	195	62
57	Lockwood Valley Road west of Kern County Line	56.8	48	15	5
58	Lockwood Valley Road east of Maricopa Highway (SR 33)	49.0	8	3	1
59	Lomita Avenue east of Tico Road	59.1	82	26	8
60	Main Street (Piru) north of Telegraph Road (SR 126)	56.7	46	15	5
61	Moorpark Road north of Santa Rosa Road	70.7	1,168	369	117
62	Old Telegraph Road west of Grand Avenue	59.2	82	26	8
63	Olds Road north of Hueneme Road	61.4	137	43	14
64	Olivas Park Drive west of Victoria Avenue	68.9	769	243	77
65	Pasadena Avenue east of Sespe Street	50.7	12	4	1
66	Patterson Road south of Doris Avenue	52.5	18	6	2
67	Pleasant Valley Road south of East Fifth Street (SR 34)	69.4	861	272	86
68	Pleasant Valley Road west of Las Posas Road	68.2	663	210	66
69	Portero Road east of Lake Sherwood Drive East	62.8	193	61	19
70	Portero Road west of Stafford Road	59.9	97	+	19
70	Portero Road west of Stafford Road Portero Road west of Hidden Valley Road	59.9	17	31	2
72	Portero Road at Milepost 2.75	58.6	73	23	7
73	Portero Road east of Lewis Road	62.7	188	59	19
74	Rice Avenue south of East Fifth Street (SR 34)	72.9		_	
		ł	1,936	612	194
75	Rice Avenue north of Channel Islands Boulevard	71.9	1,559	493	156

		Noise (dBA CNEL) at 50 feet from	Noise Cont	our Distanc	e in Feet
	Corridor and Segment	Roadway	60 dBA	65 dBA	70 dBA
76	Rice Avenue north of Hueneme Road	59.8	96	30	10
77	Rice Road south of Lomita Avenue	59.8	96	30	10
78	Rose Avenue south of Los Angeles Avenue (SR 118)	64.2	265	84	26
79	Rose Avenue south of Central Avenue	64.5	279	88	28
80	Rose Avenue north of Collins Street	67.3	540	171	54
81	Santa Ana Boulevard east of Ventura River	58.8	76	24	8
82	Santa Ana Road south of Baldwin Road (SR 150)	54.6	29	9	3
83	Santa Ana Road south of Santa Ana Boulevard	60.7	119	37	12
84	Santa Clara Avenue north of Friedrich Road	69.0	803	254	80
85	Santa Clara Avenue south of Los Angeles Avenue (SR 118)	69.9	983	311	98
86	Santa Rosa Road west of Moorpark Road	70.8	1,203	380	120
87	Santa Rosa Road west of East Las Posas Road	69.0	801	253	80
88	Santa Susana Pass Road east of Katherine Road	58.2	66	21	7
89	Sespe Street north of South Mountain Road	61.6	144	45	14
90	Sespe Street south of Pasadena Avenue	55.7	37	12	4
91	South Mountain Road east of Balcom Canyon Road	55.1	32	10	3
92	South Mountain Road south of Santa Clara River	58.4	69	22	7
93	Stockton Road east of Balcom Canyon Road	56.4	43	14	4
94	Sturgis Road west of Pleasant Valley Road	65.4	350	111	35
95	Tapo Canyon Road south of Bennett Road	52.8	19	6	2
96	Telegraph Road west of Briggs Road	65.2	331	105	33
97	Telegraph Road west of Olive Road	64.7	292	92	29
98	Tico Road north of Ventura Avenue (SR 150)	56.6	46	14	5
99	Tierra Rejada Road east of Moorpark Freeway (SR 23)	71.8	1,526	483	153
100	Torrey Road south of Telegraph Road (SR 126)	56.9	49	16	5
101	Valley Vista Drive south of Calley Aurora	59.5	88	28	9
102	Ventura Avenue north of Canada Larga Road	57.5	57	18	6
103	Ventura Avenue north of Shell Road	60.2	105	33	10
104	Victoria Avenue south of Olivas Park Drive	73.8	2,386	755	239
105	Walnut Avenue north of Los Angeles Avenue (SR 118)	53.3	21	7	2
106	Wendy Drive north of Gerald Drive	63.6	229	72	23
107	Wood Road south of Hueneme Road	58.8	75	24	7
108	Wood Road south of East Fifth Street (SR 34)	67.8	601	190	60
109	Wooley Road west of Rice Avenue	68.4	694	219	69
110	Yerba Buena Road north of Pacific Coast Highway (SR 1)	49.4	9	3	1
	Freeways / Highw	ays		•	•
111	SR 1 at Calleguas Creek	73.7	2,368	749	237
112	SR 1 at Seacliff Colony, Junction SR 101	66.9	488	154	49
113	SR 1 at Las Cruces, SR 101, Mobil Oil Pier	59.1	81	26	8
114	SR 23 at Grimes Canyon Road	69.9	987	312	99
115	SR 23 at Junction SR 126, Ventura Road	67.7	585	185	59
116	SR 33 at West Junction SR 150, Baldwin Road	66.7	465	147	47
117	SR 33 at Los Padres National Forest Boundary	55.5	35	11	4
118	SR 33 at Sespe Gorge Maintenance Station	51.0	13	4	1
119	SR 33 at Ventura/Santa Barbara County Line	53.9	25	8	2
120	SR 34 at Junction SR 118, Los Angeles Avenue	68.4	692	219	69
121	U.S. Highway 101 at Victoria Avenue	80.9	12,207	3,860	1221
122	U.S. Highway 101 at Ventura/Santa Barbara County Line	79.5	8,815	2,787	881



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		Noise (dBA CNEL) at 50 Noise Cor		our Distance in Feet	
	Corridor and Segment	feet from Roadway	60 dBA	65 dBA	70 dBA
123	SR 118 at Junction SR 232 (Westbound)	75.8	3,761	1,189	376
124	SR 118 at SR 34, Somis Road (Westbound)	72.5	1,787	565	179
125	SR 118 at Grimes Canyon Road	72.8	1,919	607	192
126	SR 118 at West Junction SR 23, Moorpark Avenue	71.7	1,475	466	147
127	SR 118 at East Junction SR 23, Spring Road	72.5	1,780	563	178
128	SR 150 at Santa Barbara/Ventura County Line	49.1	8	3	1
129	SR 150 at Junction SR 33 South (South)	63.0	197	62	20
130	SR 150 at Santa Paula North City Limit	59.0	80	25	8
131	SR 232 and Junction SR 118	65.8	381	120	38

Notes: SR = State Route; dBA = a-weighted decibels;

Gray shaded cells reflect roadway segments exceeding 60 dBA CNEL at 50 feet from the roadway centerline.

All modeling assumes average pavement, level roadways (less than 1.5% grade), constant traffic flow, and does not account for shielding of any type or finite roadway adjustments. All noise levels are reported as A-weighted noise levels.

Source: Modeled by Ascent Environmental in 2019; based on traffic data provided by GHD (2019).

HAZ-9.3 Development Along Travel Routes

The County shall evaluate discretionary development for noise generated by project-related traffic along the travel route to the nearest intersection which allows for movement of traffic in multiple directions. In all cases, the evaluation of project-related roadway noise shall be evaluated along the travel route(s) within 1,600 feet of the project site. (RDR)

HAZ-9.4 Acoustical Analysis Required

The County shall require an acoustical analysis by a qualified acoustical engineer for discretionary development involving noise exposure or noise generation in excess of the established standards. The analysis shall provide documentation of existing and projected noise levels at on-site and off-site receptors and shall recommend noise control measures for mitigating adverse impacts. (RDR)

HAZ-9.5



Site and Building Design

The County shall require discretionary development and County-initiated projects to comply with adopted noise standards through proper site and building design features, such as building location and orientation, setbacks, natural barriers and vegetation, and building construction. The County shall only consider sound walls if noise mitigation measures have been evaluated or integrated into the project and found infeasible. (RDR)

HAZ-9.6



Airport Noise Compatibility

The County shall use the aircraft noise analysis prepared for local airports or the noise contours from the current NBVC-Point Mugu Air Installations Compatible Use Zones (AICUZ) study, as most appropriate for a project location, as an accurate mapping of the long-term noise impact of the airport's aviation activity. The County shall restrict new discretionary residential land uses to areas outside of the 60 decibel Community Noise Equivalence Level (dB CNEL) aircraft noise contour unless interior noise levels can be mitigated to meet a maximum 45 dB CNEL. (RDR)

HAZ-9.7 Noise Control Priorities

The priorities for noise control for discretionary development shall be as follows:

- 1. Reduction of noise emissions at the source.
- 2. Attenuation of sound transmission along its path, using barriers, landform modification, dense plantings, building orientation and placement, and the like.
- 3. Rejection of noise at the reception point using noise control building construction, hearing protection or other means.

(RDR)

HAZ-9.8 Implement Noise Control Measures for Traffic Noise

The County shall require noise control measures to be implemented along roadways for new discretionary development generating traffic noise if either of the following circumstances would exist:

- The discretionary development would result in traffic noise levels above a County noise compatibility standard stated in Policy HAZ 9.2 in an area where traffic noise levels, under existing conditions, do not exceed the County noise compatibility standard; or,
- The discretionary development would result in an increase in traffic noise levels of 3 dBA or greater in an area where traffic noise levels under existing conditions exceed a County noise compatibility standard stated in Policy HAZ 9.2.

Noise control measures may include increased vegetation, roadway pavement improvements and maintenance, and site and building design features. If such measures are not sufficient to reduce a new discretionary development's fair-share of traffic-generated noise at sensitive receptors, a sound wall barrier may be constructed. All feasible¹ noise reduction measures shall be implemented to ensure the development's fair-share of traffic-generated noise is reduced, consistent with Policy HAZ 9.2. (RDR)

EIR

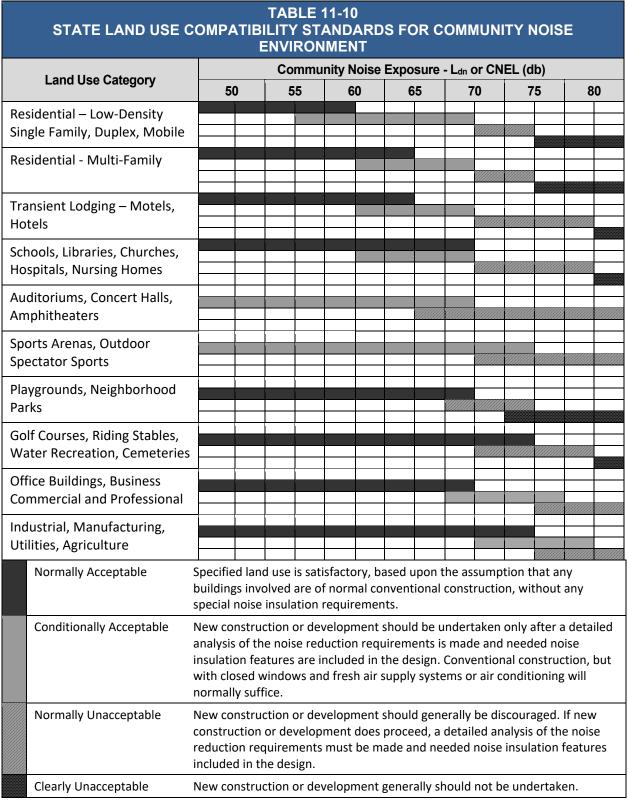




¹ "Feasible" means that this mitigation measure shall be applied to future discretionary projects under the 2040 General Plan when and to the extent it is "capable of being accomplished in a successful manner within a reasonable period of time, taking into account economic, environmental, legal, social, and technological factors" as determined by the County in the context of such future projects based on substantial evidence. This definition is consistent with the definition of "feasible" set forth in CEQA (Pub. Res. Code, § 21066.1) and the CEQA Guidelines (§ 15164). The County shall be solely responsible for making this feasibility determination in accordance with CEQA.

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2040 General Plan



Source: California Governor's Office of Planning and Research 2003

21. Noise and Vibration

A. Definition of Issue

Noise is defined as any unwanted sound that is undesirable because it interferes with speech and hearing, or is intense enough to damage hearing, or is otherwise annoying. Noise impacts can occur during the construction and/or operational phases of a project.

With the exception of a few large-scale construction projects that last a period of years, most projects involve only short term construction noise impacts. The severity of construction noise impacts varies based on the location of sensitive receptors; type or phase of construction; combination of equipment used; site layout; and, construction methods that are employed.

Operational noise typically includes long-term impacts—that is, impacts that persist throughout the life of a project. Impacts from operational noise vary based on the: location of sensitive receptors; type of equipment or machinery that is used; site layout; and, duration and times during which noise-generating uses occur.

Vibration is defined as a motion that repeatedly reverses itself. The most common type of environmental impact involving vibration consists of ground vibration, which is the periodic displacement of earth, which creates vibration waves that move through soil and rock strata, foundations of nearby buildings, and then throughout the parts of the building structure. Ground-borne vibration can result in sensible movement of the building floors, rattling of windows, shaking of items on shelves or hanging on walls, and rumbling sounds. The rumbling sound caused by the vibration of room surfaces is called ground-borne noise.

The operation of construction equipment and construction techniques (e.g., pile driving, blasting, or excavation) can generate temporary ground vibration impacts. Moreover, heavy duty vehicles traveling along roadways with potholes and bumps, steel-wheeled/steel-rail vehicles (e.g., trains), and equipment used in industrial operations which are related to a proposed project can generate recurring ground vibration impacts throughout the life of a project. If the amplitudes are high enough, ground vibration can: cause damage to buildings, ranging from more severe (yet uncommon) structural damage to less severe cosmetic damage (e.g., cracked plaster); and, generate ground-borne noise that is discomforting or a nuisance to individuals who live or work close to vibration-generating activities.

B. Definition of Terms

The following is a partial glossary of acoustic and vibration terminology. For a more comprehensive glossary of noise-related terms, see the Ventura County General Plan Hazards Appendix (§2.16.2). For a more comprehensive glossary of vibration-related terms, see the Transit Noise and Vibration Impact Assessment.¹

Ambient Noise - The noise that results from the combination of all sources, near and far, which constitutes the existing environmental setting for the purposes of evaluating noise impacts. The ambient noise levels are expressed as L_{eqT} or CNEL as judged appropriate to the situation.

A-weighted Sound Level $[L_A - dB(A)]$ - Sound pressure level measured using the A-weighting network, a filter which discriminates against low and very high frequencies in a manner similar to the human hearing mechanism at moderate sound levels (ANSI S1.4).

Community Noise Equivalent Level [CNEL - dB(A)] - The long-term time average sound level, weighted as follows:

- Frequency response is filtered using the A-weighting network.
- Sounds occurring between 7 p.m. and 10 p.m. are weighted by 5 dB (in effect, the number of noise events is multiplied by 3.15).

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¹ Hanson, Carl E., David A. Towers, and Lance D. Meister. (May 2006). *Transit Noise and Vibration Impact Assessment*. Federal Transit Administration, Office of Planning and Environment. FTA-VA-90-1003-06. Available on-line at: http://www.fta.dot.gov/documents/FTA Noise and Vibration Manual.pdf.

 Sounds occurring between 10 p.m. and 7 a.m. are weighted by 10 dB (in effect, the number of noise events is multiplied by 10).

Decibel (dB) - A unit of sound measurement equal to 10 times the base-10 logarithmic ratio squared of the magnitude of acoustic pressure divided by and relative to a specified reference level. The airborne acoustic pressure reference level is the threshold of hearing of an average human, which is equal to 20 micropascals (μ Pa or 2×10^{-5} Pa) and is equivalent to 0 dB, the quietest sound a human can hear. A 3 dB increase is barely detectable. A 10 dB increase represents a doubling of loudness.

Noise Contour - A line on a map that indicates locations of constant ambient sound level near or around known sources of noise. In practice, noise contours are often shown as calculated for the dominant source of noise only.

Noise Sensitive Uses - Dwellings, schools, hospitals, nursing homes, churches and libraries.

Time Average Sound Level (L $_{eqT}$ - dB) - The level, in decibels, of the mean sound pressure averaged over time period T. This is often referred to as "equivalent sound level" and hence the "eq" subscript. The "equivalence" is to a sound of constant level that has the same total acoustic energy content.

Vibration Category 1 (High Sensitivity Use) - Buildings where vibration would interfere with operations within the building, including levels that may be well below those associated with human annoyance. Examples include: concert halls; vibration-sensitive research and manufacturing; hospitals with vibration-sensitive equipment; and, university research operations.

Vibration Category 2 (Residential) - All residential land uses and any buildings where people sleep, such as hotels and hospitals.

Vibration Category 3 (Institutional) - Schools, churches, other institutions, and quiet offices that do not have vibration-sensitive equipment, but still have the potential for activity interference.

C. Applicable General Plan Goals and Policies

The following goals and policies of the Ventura County General Plan are applicable to this issue:

Countywide Goals, Policies and Programs: Ojai Valley Area Plan:

Goal 2.16.1 Goals 2.4.1-1 & -2

Policies 2.16.2-1 through -3 Policies 2.4.2-1 through -3

Lake Sherwood/Hidden Valley Area Plan: Piru Area Plan:

Goals 3.3.1-1 & -2 Goals 2.4.1-1 & -2

Policies 3.3.2-1 through 5 Policies 2.4.2-1 through -3

Thousand Oaks Area Plan:

Oak Park Area Plan:

Goals 2.4.1-1 & -2 Goals 2.3.1-1 & -2

Policies 2.4.2-1 through -5 Policy 2.3.2

D. Threshold of Significance Criteria

Noise Thresholds:

Any project that produces noise in excess of the standards for noise in the Ventura County General Plan Goals, Policies and Programs (Section 2.16) or the applicable Area Plan, has the potential to cause a significant noise impact. Noise-generating uses that either individually or when combined with other recently approved, pending, and probable future projects, exceeds the noise thresholds of General Plan Noise Policy 2.16.2-1(4) are considered to have a potentially significant impact.

Vibration Thresholds:

1. Construction Threshold - Any project that either individually or when combined with other recently approved, pending, and probable future projects, includes construction activities involving blasting, pile-driving, vibratory compaction, demolition, and drilling or excavation which exceed the threshold

criteria provided in the Transit Noise and Vibration Impact Assessment (Section 12.2),² is considered to have a potentially significant impact.

Table 1 - Screening Distances for Vibration Assessment

Vibration-Generating Transit Use	Distan	categories* stance from Right-of-Way or Property Line (feet)			
	Category 1	Category 2	Category 3		
Steel-Wheeled/Steel-Rail Vehicle Transit Uses					
Conventional Commuter Railroad	600	200	120		
Rail Rapid Transit	600	200	120		
Light Rail Transit	450	150	100		
Intermediate Capacity Transit	200	100	50		
Rubber-Tire Heavy Vehicle Uses					
Rubber-Tire Heavy Vehicles (if not previously screened out)**	100	50			

*See the "Definition of Technical Terms" (above) for the land uses that fall within each of the Categories, as well as the Transit Noise and Vibration Impact Assessment, Appendix A, for the definitions of vibration-generating transit uses listed in this table. For the purposes of screening procedures, concert halls and television studios should be evaluated as Category 1, and theaters and auditoriums should be evaluated as Category 2.

Source: Transit Noise and Vibration Impact Assessment, Table 9.2.

- 2. Transit Use Thresholds Table 1 lists the thresholds for vibration-generating transit uses, based on the type of transit use and the location of the transit use in relation to sensitive use categories. If a project would result in a transit use located within any of the critical distances of the vibration-sensitive uses listed in Table 1, the project has the potential to result in a significant impact and must be evaluated using the Transit Noise and Vibration Impact Assessment (Chapters 8 through 11).³
- 3. Commercial/Industrial Use Vibration Thresholds:
 - a. Any project that would generate new heavy vehicle (e.g., semi truck or bus) trips on uneven roadways located within proximity to sensitive uses has the potential to either individually or when combined with other recently approved, pending, and probable future projects, exceed the threshold criteria of the Transit Use Thresholds for rubber-tire heavy vehicle uses (Item No. 3 and Table 1, above), thereby resulting in a potentially significant impact.

-

^{**}See the discussion below.

² Ibid

³ Ibid.

b. Any project that involves blasting, pile-driving, vibratory compaction, demolition, drilling, excavation, or other similar types of vibration-generating activities has the potential to either individually or when combined with other recently approved, pending, and probable future projects, exceed the threshold criteria⁴ provided in the *Transit Noise and Vibration Impact Assessment* (Section 12.2),⁵ thereby resulting in a potentially significant impact.

E. Methodology

Noise

Construction noise impacts shall be evaluated using the assessment methodology, criteria, and reporting procedures provided in the Construction Noise Threshold Criteria and Control Measures.⁶ All other types of noise impacts shall be evaluated pursuant to the following procedures.

Step 1 - Preliminary Noise Assessment

A preliminary noise assessment shall be conducted by the County Agency responsible for administering the proposed development project. The purpose of the preliminary noise assessment is to determine if a consultant prepared acoustical analysis is required. (See Step 2, below) The preliminary noise assessment shall consist of the following:

- a. **Determine if the Proposed Use is Noise Sensitive or a Noise Generator -** If the proposed use is *noise sensitive*, see Steps 1.b, 1c and 1.d below. If the proposed use is a potential noise generator, see Step 1.e below.
- b. **Consult) GIS Noise Exposure/Contour Maps** Using Planning GIS, view the project site with the noise layers turned on, in order to determine whether or not the noise-sensitive use site is within the 60 dB(A) CNEL contour of a highway or airport. If the project is located within this contour, the noise impact is potentially significant and a consultant prepared acoustical analysis must be completed.
- c. Consult Land Use Maps Locate the project area on the General Land Use, Existing Community and Area Plan Maps (as appropriate) of the General Plan, which are available from the Resource Management Agency, GIS Development and Mapping Services Division. If the project is noise-sensitive and is within 500 feet of an industrially designated area, the noise impact is potentially significant and a consultant prepared acoustical analysis must be completed.
- d. **Consult GIS Aerial Imagery –** Using Planning GIS, view the project site with the most current aerial imagery layer turned on to determine if a railroad exists within the vicinity of the project site. If a railroad exists, use the measuring tool to determine the distance between the noise-sensitive use site and the railroad. If the noise-sensitive project site is located within 3,400⁷ feet of a railroad, the noise impact is potentially significant and a consultant prepared acoustical analysis must be completed.
- e. **Estimate Potential Noise Impact -** If the project is a noise-generator, it will be necessary to determine:
 - The noise-generating equipment's and activities' estimated noise levels and the times at which the noise levels would occur; and,

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⁴ The severity of vibration-related impacts to buildings and humans are the same regardless of the source of the vibration, be it from construction or operational activities, provided that the equipment is equivalent in terms of their vibration-generating potential. Therefore, the construction-related threshold criteria are to be used for commercial/industrial operations.

⁵ Hanson, Carl E., David A. Towers, and Lance D. Meister. (May 2006).

⁶ Advanced Engineering Acoustics. (November 2005). *County of Ventura Construction Noise Threshold and Criteria Plan.* Available on-line at: http://www.ventura.org/rma/planning/pdf/ceqa/Construction_Noise_Thresholds.pdf..

⁷ This distance was determined based on: (1) the maximum indoor noise level for habitable rooms (45 CNEL) stated in the Ventura County General Plan *Goals, Policies and Programs*, Noise Policy 2.16.2-1(1)a; and, (2) the calculated distance in feet between main line railroad tracks and the 45 CNEL contours, for railroads within Ventura County (Ventura County General Plan *Hazards Appendix*, 2005, 94).

• The proximity of the noise-generating equipment to the noise-sensitive uses using the project plans, information gathered during a site visit, aerial imagery, and land use maps that are available from the Resource Management Agency, GIS Development and Mapping Services Division.

In general, noise decreases by 5 dB for each doubling of the distance from the noise source. If the noise from the proposed project is estimated to exceed any of the following standards at the nearest *noise sensitive use*, the noise impact is deemed to have a potentially significant noise impact and a consultant prepared acoustical analysis must be completed:

55 dB(A) between 6:00 a.m. and 7:00 p.m.,

50 dB(A) between 7:00 p.m. and 10:00 p.m., or

45 dB(A) between 10:00 p.m. and 6:00 a.m.

If the preliminary noise assessment reveals that the project does not have the potential to create a significant noise impact and an acoustical analysis is not required, the agency that is responsible for administering the project shall complete the Initial Study Checklist and discussion of responses to the checklist pursuant to the "Instructions for Preparing an Initial Study" provided in the Ventura County Initial Study Assessment Guidelines. However, if the preliminary noise assessment reveals that the project has the potential to create a significant noise impact, a consultant prepared acoustical analysis must be prepared pursuant to the criteria provided in Step 2 (below).

Step 2 - Consultant Prepared Acoustical Analysis

If it is determined that a quantitative assessment is required, a qualified noise consultant shall prepare the analysis (see attached Noise Consultant Qualifications). The agency that is responsible for administering the project will ensure that the consultant meets the minimum qualifications.

Acoustical Analysis Requirements

The purpose of the consultant prepared acoustical analysis is to: determine if the project would result in any potentially significant noise impacts; identify any feasible mitigation measures that might exist to reduce the severity of the noise impacts; and, determine if the noise impacts, after mitigation, are still potentially significant. As such, the acoustical analysis must include a(n):

- Discussion of the existing environmental setting (e.g., a description of the noise sources and ambient noise levels of the project site and surrounding area);
- Discussion of recently approved, pending, and probable future noise-generating projects⁸ that
 have the potential to contribute to cumulative impacts to the noise environment and, as such,
 are included in the acoustical analysis;
- Discussion of the methodology used in collecting noise data (e.g., noise equipment and metrics used). Noise measurements should be taken using standard industry practices, after taking into consideration site-specific characteristics (e.g., buildings, walls, topography, and the location of existing and potential future noise-sensitive receptors in relation to noise generators) which might have an influence on the noise measurements;
- Discussion of the methodology used in calculating project-specific and cumulative noise impacts (e.g., noise models used);
- Presentation of the data on the existing noise environment, as well as data on projected noise levels; and.
- Initial Study checklist and discussion pursuant to the requirements of the "Instructions for Preparing an Initial Study" in the Ventura County Initial Study Assessment Guidelines.

⁸ The list of recently approved, pending, and probable future projects is available on-line at: http://www.ventura.org/rma/planning/Permits/projects.html.

Step 3 - Environmental Document Determination

If the acoustical analysis shows that there would be no significant impact, the Initial Study Checklist should be checked LS. If the study shows that there would be potentially significant noise impacts, but feasible mitigation measures could be incorporated into the project which could reduce the impact to a less than significant level, then the Initial Study Checklist should be checked PS-M. If the study shows that there would be significant, immitigable noise impacts (except construction related noise), the project could not be approved because of the General Plan noise policies.

Step 4 - Update Data Base

In a continuing effort to update County noise data, a copy of all consultants' acoustical analysis shall be sent to the Planning Director.

Vibration:

Construction-Related Vibration

The agency that is responsible for administering the project shall request from the applicant information regarding the: types of construction activities that will be required; duration of each construction phase; and, types and number of construction equipment that will be used during each phase of construction. Using the list of recently approved, pending, and probable future projects, the agency also shall identify other vibration-generating projects located within the vicinity of the project site that have the potential to contribute to cumulative impacts relating to vibration. Once this information is obtained, the agency that is responsible for administering the project shall evaluate potential construction-related vibration impacts using the assessment methodology provided in the Transit Noise and Vibration Impact Assessment (Section 12.2 et seq). 10

As discussed in the Transit Noise and Vibration Impact Assessment, many projects will not have the potential to create prolonged annoyance or damage from construction vibrations and, therefore, will only require a qualitative assessment of potential construction-related vibration impacts. In these cases, the agency that is responsible for administering the project shall prepare the Initial Study checklist and discussion pursuant to the requirements of the "Instructions for Preparing an Initial Study" in the Ventura County Initial Study Assessment Guidelines.

Steel-Wheeled/Steel-Rail Vehicle Transit Uses

In order to determine if a project has the potential to generate a significant impact using the threshold criteria provided above (Threshold Criterion No. 3 and Table 1), the agency that is responsible for administering the project will need to determine if any vibration-sensitive uses are located within proximity to the project site. This information can be gathered by observation during a site visit and using the aerial imagery in Planning GIS. During the site visit, the agency that is responsible for administering the project shall identify any vibration-sensitive uses located within proximity to the project site. Using Planning GIS, the agency that is responsible for administering the project should view the project site with the most current aerial imagery data layer, identify the location of the vibration sensitive use that was identified during the site visit vis-à-vis the project site, and use the measuring tool to determine the distance between the vibration-sensitive use and the project site.

If the project site is located outside of the critical distance for the vibration-sensitive use specified in Table 1 (above), the project would have a less-than-significant impact, and the agency that is responsible for administering the project shall complete the Initial Study checklist and discussion pursuant to the requirements of the "Instructions for Preparing an Initial Study" in the Ventura County Initial Study Assessment Guidelines.

If the project site is located within the critical distance specified in Table 1 (above), the project shall be evaluated for potential vibration impacts using the assessment methodology, criteria, and reporting procedures provided in the Transit Noise and Vibration Impact Assessment (Chapters 9 through 11, and

⁹ See Footnote 13 (above).

¹⁰ Hanson, Carl E., David A. Towers, and Lance D. Meister. (May 2006).

13).¹¹ Both project-specific and the project's contribution to cumulative impacts shall be evaluated. Cumulative impacts shall be evaluated by incorporating into the assessment all recently approved, pending, and probable future projects located within the vicinity of the project site that have the potential to contribute to cumulative impacts relating to vibration.¹² A qualified engineer must prepare the analysis. The agency that is responsible for administering the project will be responsible for selecting the consultant, and shall develop its own contract procedures with which to hire consultants. The consultants must meet the qualifications discussed in the Construction-Related Vibration Section (above). The analysis must include an Initial Study checklist and discussion that meets the requirements of the "Instructions for Preparing an Initial Study" in the Ventura County Initial Study Assessment Guidelines.

Rubber-Tire Heavy Vehicle Transit Uses

Rubber-tire heavy vehicles traveling on roadways typically will not produce a significant vibration impact, except in situations where a large number of heavy vehicles (e.g., semi trucks or buses) are traveling along uneven roadways within proximity to sensitive uses. Therefore, if a project would build, place or expand vibration-sensitive uses in close proximity to roadways on which a large number of rubber-tire heavy vehicles travel, the following initial screening questions must be asked to determine if the project would result in a potentially significant vibration impact:

- Will the project result in the location of vibration-sensitive uses in close proximity to roadways with expansion joints, speed bumps, or other design features that result in unevenness in the road? Such roadway irregularities can result in perceptible ground-borne vibration at distances up to 75 feet away.
- 2. Will the project result in buses, trucks or other heavy vehicles operating near a vibration-sensitive use? Research using electron microscopes and manufacturing of computer chips are examples of vibration-sensitive uses.
- 3. Will the project result in the operation of vehicles inside or directly underneath buildings that are vibration-sensitive? Special considerations are often required for shared-use facilities such as a bus station located inside an office building complex.

If the answer is "no" to all three of the initial screening questions, the project would have a less-thansignificant impact, and the agency that is responsible for administering the project shall complete the Initial Study checklist and discussion that meets the requirements of the "Instructions for Preparing an Initial Study" in the Ventura County Initial Study Assessment Guidelines.

If the answer is "yes" to any one of the initial screening questions, the project must be evaluated using the screening criteria in Table 1 (above). If the project would result in the location of rubber-tire heavy vehicle uses within any of the critical distances of the sensitive use categories listed in Table 1, the project has the potential to generate a significant impact, and must be evaluated using the Transit Noise and Vibration Impact Assessment. Both project-specific and the project's contribution to cumulative noise impacts shall be evaluated. Cumulative impacts shall be evaluated by incorporating into the assessment all recently approved, pending, and probable future projects located within the vicinity of the project site that have the potential to contribute to cumulative impacts relating to vibration. A qualified engineer must prepare the analysis. The agency that is responsible for administering the project will be responsible for selecting the consultant, and shall develop its own contract procedures with which to hire consultants. The consultants must meet the qualifications discussed in the Construction-Related Vibration Section (above). The analysis must include an Initial Study checklist and discussion that meets the requirements of the "Instructions for Preparing an Initial Study" in the Ventura County Initial Study Assessment Guidelines.

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¹¹ Hanson, Carl E., David A. Towers, and Lance D. Meister. (May 2006).

¹² See Footnote 13 (above).

¹³ Hanson, Carl E., David A. Towers, and Lance D. Meister. (May 2006).

¹⁴ See Footnote 13 (above).

Commercial- or Industrial-Generated Vibration

Any project that would generate new heavy vehicle (e.g., semi truck or bus) trips on uneven roadways located within proximity to sensitive uses shall be evaluated using the methodology prescribed for rubber-tire heavy vehicle transit uses (above).

Any project that involves blasting, pile-driving, vibratory compaction, demolition, drilling, excavation, or other similar types of vibration-generating activities shall be evaluated using the methodology prescribed for construction-related vibration (above).

Adopted by the Board of Supervisors on July 27, 2010

Attachment Noise Consultant Qualifications

The Environmental Quality Advisory Committee has established the following minimum qualifications for noise consultants for the purpose of conducting acoustical analysis. Noise consultants must demonstrate that they meet the minimum qualifications as defined below:

Education - Consultants should hold an advanced degree from an accredited institution (e.g., M.A., M.S., or Ph.D.) in Physics, Mathematics, Engineering or related discipline. Consultants without an advanced degree in these fields must provide documentation of at least five years of relevant research or field work in acoustical engineering.

Experience - All consultants must possess a working knowledge of physics, acoustical principles, utilization of sound level meters, and applicable state codes. Experience with CEQA is highly desirable. Consultants also must have experience in the following:

- Acquiring and evaluating data;
- Creating mitigation monitoring and reporting programs; and,
- Evaluating designs for compliance with standards relative to land use.

Local and State Expertise - Consultants must provide evidence of expertise in community/industrial noise (e.g., the preparation of Noise Elements of General Plans, technical reports, studies, mitigation measures, or noise ordinances).

Professional Certification - Evidence of professional certification is highly desirable though not required.

Vibration Consultant Qualifications

Environmental Quality Advisory Committee has established the following minimum qualifications for vibration consultants for the purpose of conducting vibration analyses. Vibration consultants must demonstrate that they meet the minimum qualifications for vibration consultants as defined below:

Education - Consultants should hold an advanced degree from an accredited institution (e.g., M.A., M.S., or Ph.D.) in Physics, Mathematics, Engineering or related discipline. Consultants without an advanced degree in these fields must provide documentation of at least five years of relevant research or field work in engineering activities involving vibration impact assessment.

Experience: All consultants must possess a working knowledge of physics, vibration principles, and applicable state codes. Experience with CEQA is highly desirable. Consultants also must have at least five years experience in the following:

- Acquiring and evaluating data;
- Creating mitigation monitoring and reporting programs; and,
- Evaluating designs for compliance with standards relative to land use.

Local and State Expertise - Consultants must provide evidence of expertise in transportation, construction, and/or industrial vibration (e.g., the preparation of environmental assessments, technical reports, studies, or mitigation measures).

Professional Certification - Evidence of professional certification is highly desirable though not required.

TABLE IV. IMMEDIATE ABATEMENT POTENTIAL OF CONSTRUCTION EQUIPMENT

Equipment	in dB(/	se Level A) at 50 ft With Feasible Noise Control ¹	Important Noise Sources ²	Usage ³
Earthmoving front loader backhoes dozers tractors scrapers graders truck paver	79 85 80 80 88 85 91	75 75 75 75 80 75 75 80	E C F I H E C F I H E C F I W E C F I W E C F I W E C F I T E D F I	.4 .16 .4 .4 .4 .08
Materials Handling concrete mixer concrete pump crane derrick	85 82 83 88	75 75 75 75	E C F W T E C H E C F I T E C F I T	.4 .4 .16 .16
Stationary pumps generators compressors	76 78 81	75 75 75	E C E C H I	1.0 1.0 1.0
Impact pile drivers jack ha mers rock drills pneumatic tools	101 88 98 86	95 75 80 80	W P E P W E C W E P P W E C	.04 .1 .04 .16
Other saws vibrator	78 76	75 75	W W E C	.04 .4

Notes:

- 1. Estimated levels obtainable by selecting quieter procedures or machines and implementing noise control features requiring no major redesign or extreme cost.
- 2. In order of importance:
 - T Power Transmission System, F Cooling Fan Gearing

- C Engine Casing
- W Tool-Work Interaction
- E Engine Exhaust
- H Hydraulics
- P Pneumatic Exhaust
- I Engine Intake
- 3. Percentage of time equipment is operating at noisiest mode in most used phase on site.

this table, one may determine that control of engine noise, and particularly of engine exhaust noise, will affect many items of equipment with high usage factors and thus should be given high priority.

Table V presents a brief listing of the noise control techniques applicable to the sources indicated in Table IV, together with an estimate of the noise reductions that may readily be achieved by means of these techniques.

2.2 Home Appliances

The use of convenient and sometimes necessary appliances constitutes a growing noise problem within the home. Almost without exception, appliances could be significantly quieter. However, manufacturers offer three primary arguments for opposing quieter redesign; they believe

- that the public associates the noise generated by a device with its power;
- that quieter appliances would be marketed at a price disadvantage and since the public has not objected to noise, that the public, in general, is satisfied;
- that since appliances are generally controlled by the operator, the option, as with air conditioners, "to have quiet or to be cool" is "option enough".

Yet, in keeping with the public's growing awareness of noise, many appliances are advertised as being "noiseless", "quiet", "vibration-free".

Although many manufacturers have made detailed acoustic measurements of the noise output of their appliances, very little data has been reported in the open literature. Some of the

TABLE V. NOISE CONTROL FOR CONSTRUCTION EQUIPMENT

Source	Control Techniques	Probable Noise Reduction in dB(A)*
Engine		
exhaust	improved muffler	10
casing	improved design of block	2
	enclosure	10
fan (cooling)	redesign	5
	silencers, ducts and mufflers	5
intake	silencers	5
Transmission	redesign, new materials	7
	enclosure	7
Hydraulics	redesign, new materials	7
	enclosure	10
Exhaust		
(pneumatic)	muffler	5-10
Tool-Work		
interaction	enclosure	7-20
	change in principle	10-30

^{*}Note that noise reductions are not additive. Incremental reductions can be realized only by simultaneous quieting of all sources of equal strength.

Relationship Between Indoor and Outdoor Levels

The contribution of outdoor noise to indoor noise levels is usually small. That pan of a sound level within a building caused by an outdoor source obviously depends on the source's intensity and the sound level reduction afforded by the building. Although the sound level reduction provided by different buildings differs greatly, dwellings can be categorized into two broad classes— those built in warm climates and those built in cold climates. Further, the sound level reduction of a building is largely determined by whether its windows are open or closed. Table II shows typical sound level reductions for these categories of buildings and window conditions, as well as an approximate national average sound level reduction.

Table II
Typical Sound Level Reductions of Buildings

	Windows Opened	Windows Closed
Warm Climate	12dB	24dB
Cold Climate	17dB	27dB
Approximate National Average	15dB	25dB

Sample measurements of outdoor and indoor noise levels during 24-hour periods are depicted in Figure 7. Despite the sound level reduction of buildings, indoor levels are often comparable to or higher than levels measured outside. Thus, indoor levels often are influenced primarily by internal noise sources such as appliances, radio and television, heating and ventilating equipment, and people. However, many outdoor noises may still annoy people in their homes more than indoor noises do. Indeed, people sometimes turn on indoor sources to mask the noise coming from outdoors.

An example of the range of hourly sound levels measured inside living areas in plotted for each hour of the day in Figure 8. The figure shows the median levels and the range of levels observed for 80% of the data. During late night hours the typical hourly sound level was approximately 36 dB. This level was probably dominated by outdoor noise. However, during the day, the hourly average levels ranged from about 40 to 70 dB, indicating the wide range of activities in which people engage.

INDIVIDUAL NOISE EXPOSURE PATTERNS

During a 24-hour period, people are exposed to a wide range of noises, including noise at home, work, school, places of recreation, shopping establishments, and while enroute to these or other. locations. Clearly, no single exposure pattern can be typical of all people, or even of those people who follow a common life style. Figure 9 shows hypothetical exposure patterns for broad classes of people. From these levels and some assumptions about the hours spent at different daytime activities, 24-hour average sound levels can be estimated for factory and office workers, housewives, and preschool and school-age children. Estimates based on these assumptions are found in Table III.

3.5.1 Barrier Design Goals and Insertion Loss.

The first step in barrier design is to establish the design goals. Design goals may not be limited simply to noise reduction at receivers, but may also include other considerations of safety and maintenance as well. These other considerations are discussed later in Sections 4 through 13.

In this section, the acoustical design goals of noise reduction will be discussed. Acoustical design goals are usually referred to in terms of barrier *Insertion Loss* (IL). IL is defined as the sound level at a given receiver before the construction of a barrier minus the sound level at the same receiver after the construction of the barrier. The construction of a noise barrier usually results in a partial loss of softground attenuation. This is due to the barrier forcing the sound to take a higher path relative to the ground plane. Therefore, barrier IL is the net effect of barrier diffraction, combined with this partial loss of soft-ground attenuation.

Typically, a 5-dB(A) IL can be expected for receivers whose line-of-sight to the roadway is just blocked by the barrier. A general rule-of-thumb is that each additional 1 m of barrier height above line-of-sight blockage will provide about 1.5 dB(A) of additional attenuation (see Figure 13).

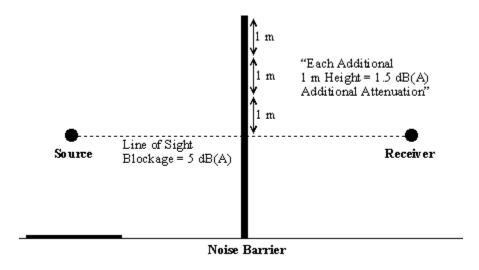


Figure 13. Line-of-sight

Properly-designed noise barriers should attain an IL approaching 10 dB(A), which is equivalent to a perceived halving in loudness for the first row of homes directly behind the barrier. For those residents not directly behind the barrier, a noise reduction of 3 to 5 dB(A) can typically be provided, which is just slightly perceptible to the human ear. Table 4 shows the relationship between barrier IL and design feasibility.

Table 4. Relationship between barrier insertion loss and design feasibility.

Barrier Insertion Loss	Design Feasibility	Reduction in Sound Energy	Relative Reduction in Loudness
5 dB(A)	Simple	68%	Readily perceptible
10 dB(A)	Attainable	90%	Half as loud
15 dB(A)	Very difficult	97%	One-third as loud
20 dB(A)	Nearly impossible	99%	One-fourth as loud

Aggregate & Recycle Plant/Rock Crushing Source Noise

		M	easured Noise Levels	Re	eference Noise Leve	els			
Location #	Measurement #	Measured L _{eq}	Measured L _{eq} (Combined)	Distance from Source (ft.)	Reference Distance (ft.) ^A	L _{eq} @ 50-feet ^B	L _{eq} @ 50-feet (Combined)		
	#1	76.0				88.3			
Location #1	#2	80.6	81.2	205	50	92.9	93.4		
	#3	83.8				96.1			
	#1	79.4				88.6			
Location #2	#2	76.3	76.4	145	50	85.5	85.7		
	#3	61.9				71.1			
Location #3	#1	77.7	80.7	42		76.2	79.2		
	#2	83.7			50	82.2			
	#3	77.9				76.4			
	#1	83.4	81.5				83.6		
Location #4	#2	74.3		51	50	74.5	81.6		
	#3	82.4				82.6			
	#1	83.0						83.7	
Location #5	#2	82.2	81.0	54	50	82.9	81.7		
	#3	72.1				72.8			
Location #6	#1	85.1	82.7	78	50	89.0	86.5		
Location #0	#2	76.7	82.7	76	30	80.6	80.5		
Location #7	#1	79.0	78.2	96	50	84.7	83.8		
Location #7	#2	77.1	78.2	30	30	82.8	03.0		
Location #8	#1	83.4	83.4	140	50	92.3	92.3		
Location #9	#1	73.3	73.3	120	50	80.9	80.9		
Location #10	#1	72.8	72.8	70	50	75.7	75.7		

Note: The noise levels shown above were measured while a rock crushing plant was operating "at maximum capacity". In addition to the plant, mobile equipment (haul trucks, loaders) were also operating in the vicinity during the measurements. Despite the addition of mobile sources, these measured sound levels are conservatively utilized to represent Pacific Rock's existing Aggregate Plant and the proposed Recycle Plant.

A - Distances (feet) estimated using Google Earth ™.

 $B - L_{eq} Calc = Selected_L_{eq} - 20*log(D/50). \ "Selected_L_{eq}" = reference \ noise \ level @ 50-feet. \ D = distance \ to \ location/receptor (feet). \\ Source: Ventura County's \ \textit{Construction Noise Threshold and Control Plan} \ \ \text{and FHWA's Roadway Construction Noise Model} \ .$

C - A total of 22 noise measurements (3-minutes each) were collected at 10 locations surrounding the crushing/processing equipment at an Otay Mesa aggregate facility.

Measurements were collected while the crushing equipment was operating at approximately full capacity and within line-of-sight of the noise source(s).

Noise measurements were then statistically combined/averaged to determine an average source noise level (L_{eq} = 84.1 dBA) at a reference distance of 50-feet.

2.1.3.8 White and Pink Noise

White noise is noise with a special frequency spectrum that has the same amplitude (level) for each frequency interval over the entire audible frequency spectrum. It is often generated in laboratories for calibrating sound level measuring equipment, specifically its frequency response. One might expect that the octave or one-third-octave band spectrum of white noise would be a straight line, but this is not true. Beginning with the lowest audible octave, each subsequent octave spans twice as many frequencies than the previous ones, and therefore contains twice the energy. This corresponds with a 3-dB step increase for each octave band, and 1 dB for each one-third-octave band.

Pink noise, in contrast, is defined as having the same amplitude for each octave band (or one-third-octave band), rather than for each frequency interval. Its octave or one-third-octave band spectrum is truly a straight "level" line over the entire audible spectrum. Therefore, pink noise generators are conveniently used to calibrate octave or one-third-octave band analyzers.

Both white and pink noise sound somewhat like the static heard from a radio that is not tuned to a particular station.

2.1.4 Sound Propagation

From the source to receiver, noise changes both in level and frequency spectrum. The most obvious is the decrease in noise as the distance from the source increases. The manner in which noise reduces with distance depends on the following important factors.

- Geometric spreading from point and line sources.
- Ground absorption.
- Atmospheric effects and refraction.
- Shielding by natural and manmade features, noise barriers, diffraction, and reflection.

2.1.4.1 Geometric Spreading from Point and Line Sources

Sound from a small localized source (approximating a point source) radiates uniformly outward as it travels away from the source in a spherical pattern. The sound level attenuates or drops off at a rate of

6 dBA for each doubling of the distance (6 dBA/DD). This decrease, resulting from the geometric spreading of the energy over an everincreasing area, is referred to as the inverse square law. Doubling the distance increases each unit area, represented by squares with sides "a" in Figure 2-7, from a² to 4a².

Because the same amount of energy passes through both squares, the energy per unit area at 2D is reduced four times from that at distance D. Therefore, for a point source the energy per unit area is inversely proportional to the square of the distance. Taking $10\log_{10}(1/4)$ results in a 6-dBA/DD reduction. This is the point source attenuation rate for geometric spreading.

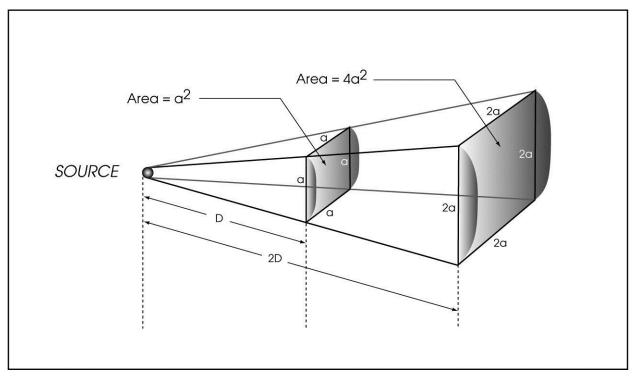


Figure 2-7. Point Source Propagation (Spherical Spreading)

As seen in Figure 2-8, based on the inverse square law the change in noise level between any two distances because of spherical spreading can be found using the following equation:

$$dBA_2 = dBA_1 + 10log_{10}[(D_{1}/D_{2})]^2 = dBA_1 + 20log_{10}(D_{1}/D_{2})$$
 (2-13)

Where:

 dBA_1 = noise level at distance D_1

 dBA_2 = noise level at distance D_2

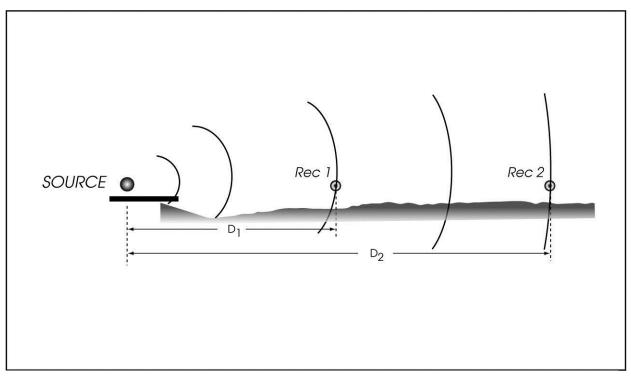


Figure 2-8. Change in Noise Level with Distance from Spherical Spreading

However, highway traffic noise is not a single, stationary point source. The movement of the vehicles makes the source of the sound appear to emanate from a line (line source) rather than a point when viewed over a time interval (Figure 2-9). This results in cylindrical spreading rather than spherical spreading. Because the change in surface area of a cylinder only increases by two times for each doubling of the radius instead of the four times associated with spheres, the change in sound level is 3 dBA/DD. The change in noise levels for a line source at any two different distances from cylindrical spreading is determined using the following equation:

$$dBA_2 = dBA_1 + 10\log_{10} (D_1/D_2)$$
 (2-14)

Where:

 dBA_1 = noise level at distance D_1 and conventionally the known noise level dBA_2 = noise level at distance D_2 and conventionally the unknown noise level \underline{Note}

The expression $10\log_{10}(D_1/D_2)$ is negative when D_2 is more than D_1 and positive when D_1 is more than D_2 . Therefore, the equation automatically accounts for the receiver being farther or closer with respect to the source— \log_{10} of a number less than 1 gives a negative result, \log_{10} of a number more than 1 is positive, and $\log_{10}(1) = 0$.

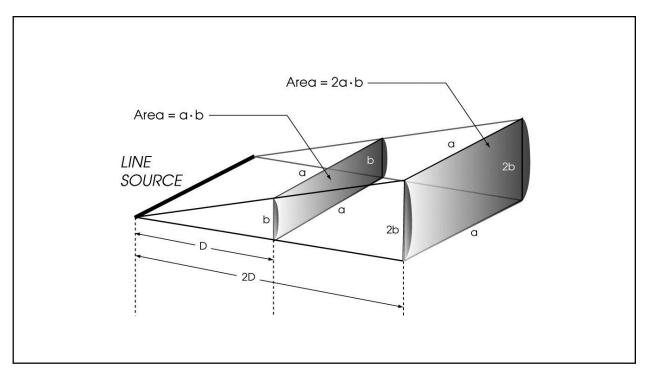


Figure 2-9. Line Source Propagation (Cylindrical Spreading)

2.1.4.2 Ground Absorption

Most often, the noise path between the highway and observer is very close to the ground. Noise attenuation from ground absorption and reflective wave cancellation adds to the attenuation from geometric spreading. Traditionally, this excess attenuation has been expressed in terms of decibels of attenuation per doubling of distance. This approximation is done for simplification only; for distances of less than 200 feet, the prediction results based on this scheme are sufficiently accurate. The sum of the geometric spreading attenuation and excess ground attenuation (if any) is referred to as the attenuation or dropoff rate. For distances of 200 feet or more, the approximation causes excessive inaccuracies in predictions. The amount of excess ground attenuation depends on the height of the noise path and characteristics of the intervening ground or site. In practice, excess ground attenuation may vary from 0 to 8–10 dBA/DD or more. In fact, it varies as the noise path height changes from the source to receiver and with vehicle type because the source heights are different. The complexity of terrain also influences the propagation of sound by potentially increasing the number of ground reflections.

The FHWA TNM is the model that is currently approved by FHWA for use in noise impact studies. The TNM has complex algorithms that directly calculate excess ground attenuation based on ground type and site geometry.

Noise and Groundborne Vibration Impact Assessment	LU10-0003 Modification Application Pacific Rock Quarry
	APPENDIX C
SIGNIFICANCE THRESHOLDS & AMBIEN	IT NOISE MEASUREMENT LOGS

Measure Ambient Noise Levels + Ventura County Significance Thresholds

Ventura Co	unty Noise Significance	Thresholds			
	Facility Th	resholds	Haul Ro	oute Thresi	nolds
Standard	Hours	urs Noise Threshold (L _{eq} 1H) Outdoor		oor	Indoor
Standard	Hours	Noise Illieshold (L _{eq} IH)	CNEL	L _{eq} 1H	CNEL
Daytime	6:00 a.m 7:00 p.m.	55 dBA or ambient +3 dBA			
Evening	7:00 p.m 10:00 p.m.	50 dBA or ambient +3 dBA	60 dBA	65 dBA	45 dBA
Nighttime	10:00 p.m 6:00 a.m.	45 dBA or ambient +3 dBA			

Source: Ventura County 2040 General Plan Noise Element (September 2020) / Ventura County Initial Study Assessment Guidelines (April 2011)

CNEL = Community Noise Equivalent Level, is a long-term average sound level with a +5 dBA penalty added to evening (7:00 p.m. - 10:00 p.m.) noise and a +10 dBA penalty added to nighttime (10:00 p.m. - 7:00 a.m.) noise.

Facility Rec	eptors - Ambient/Baseli	ine Measurements & Ventura Cou	unty Significar	nce Thresh	olds				
Receptor Measurement / Study		Receptor Type	Date(s)) Ambient Noise Levels (L _{eq} 1H)			County Thresholds (L _{eq} 1H) ^{C, D}		
Receptor	ivieasurement / Study	кесеріої Туре	Measured	Daytime	Evening	Nighttime	Daytime	Evening	Nighttime
R1 ^B	Study #2	Cemetery	12/20/2018	41.6	32.9	32.7	55	50	45
R2 ^A	Ctudy #1	Posidoneo(s)	12/20/2018	44.8	36.2	36.0	55	50	45
R2	Study #1	Residence(s)	12/21/2018	44.8	30.2	36.0	55	50	45
R3 ^A	Study #1	Recreation/Open Space	12/20/2018	44.8	36.2	36.0	55	50	45
K3	Study #1	Recreation/Open Space	12/21/2018	44.0	30.2	30.0	33	30	43

Haul Route	Receptors - Ambient/B	aseline Measurements & Ventura	a County Signi	ficance Thr	esholds								
			Date(s)	An	nbient Noi	se Levels (L _e	_q 1H & CNE	L) ^E		County Thresholds (L _{eq} 1H & CNEL) ^{C, E}			
Receptor Measurement / Study	Receptor Type	Measured	s .: -	F	5 · N. L	CNEL		D +		Ni -l-ssi	CNEL		
			ivieasureu Da	Daytime	Daytime Evening	Nighttime	Outdoor	Indoor ^F	Daytime	Evening	Nighttime	Outdoor	Indoor
R4 ^A	Study #3	Residence	1/23/2019 1/24/2019	59.8	50.7	47.9	58.9	38.9	62.8	53.7	50.9	61.9	45.0
R5 ^B	Study #4 Study #5 Study #6	Residence(s)	1/23/2019 1/24/2019	77.4	66.3	65.4	62.2	42.2	80.4	69.3	68.4	65.2	45.2

- A Ambient noise levels at Receptors 2 (R2), 3 (R3) and 4 (R4) represent actual Lea1H noise levels measured during the daytime, evening, and nighttime timeframes over a 24-hours period.
- B For Receptors 1 (R1) and 5 (R5) where 24-hour measurements were not collected, a dBA ±change was calculated by comparing measured short-duration (15-minute) L_{eq} values at these locations to the measured L_{eq} noise level at the appropriate 24-hour reference location during the identical time period. The difference between these values (i.e., correction factor) is is then applied to the applicable daytime, evening, nighttime, and CNEL 24-hour L_{eq} 1H measurements to estimate the noise levels at Receptors R1 and R5.
- C Ambient noise levels at Facility receptors (R1, R2 and R3) are below the Ventura County "fixed" thresholds, and therefore the "fixed" thresholds are utilized to determine the significance of Facility noise impacts on these receptors. However, because the ambient noise levels at haul route receptors (R4 and R5) already exceeds the "fixed" thresholds, per Ventura County guidance the measured "ambient noise level +3 decibels (dBA)" is utilized to determine the significance of haul route noise impacts at Receptors R4 and R5.
- D Because the Facility (i.e., mining and processing operations) will operate during daytime hours (i.e., 7:00 a.m. 4:00 p.m.) only, the daytime threshold is utilized to determine the significance of Facility noise impacts. The evening and nighttime ambient noise levels and significance thresholds are shown for information purposes only.
- E As discussed in Appendix E, the applicable CNEL thresholds are utilized to determine the significance of Project haul truck impacts. The measured leq1H noise levels, and adjusted thresholds, for the daytime, evening, and nighttime timeframes are shown for informational purposes only.
- F Based on the EPA's *Protective Noise Levels* document (March, 1974), an outdoor to indoor attenuation of -20 dBA is assumed. This takes into account the average noise reduction provided while windows are closed (-25 dBA) and while windows are open (-15 dBA). This is a conservatively low estimate of noise attenuation as residences are expected to generally keep windows closed, especially those facing sources of noise. The -20 dBA attenuation is applied to the CNEL values. See Appendix B for the applicable excerpt from the EPA guidance document.

Study #1 - Facility Long-Duration (24-Hours)

Study Study Time Study #1 0.01: Receptors 2 & 3 0.02: 0.03: 0.03: 0.04: 0.05: 0.06: 0.05: 0.06: 0.07: 0.08: 0.09: 0.09: 0.10: 0.10: 0.11: 0.12: 0.13: 0.14: 0.15: 0.16: 0.17: 0.16: 0.17: 0.16: 0.17: 0.18: 0.19: 0.20	Time	Status Meter1	9 50.99 50.91 45.71 45.71 35.55 38.75 38.45 3 36.65 38.75 38.75 38.75 38.76 6 35.77 1 41.66 5 38.31 38.33 1 38.37 35.47	30.9 30.2 31.4 31.6 32.6 30.8 31.9 32.2 32.7 30.7 35.1 32.7 35.1 32.7
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	Time	Date
Start:	1:00:53 PM	12/20/2018
End:	1:00:53 PM	12/21/2018

Baseline Noise Level (24-Hour)
24-Hour L_{eq}: 42.6

Baseline Noise Level (L _{eq} 1H) @ R2					
Daytime:	44.8				
Evening:	36.2				
Nighttime:	36.0				

	15-Min L _{eq}	24-Hour L _{eq}	Difference
Study #2 (R1)	46.9	50.1	-3.2

_	Daytime	Evening	Nighttime
Study #2 (R1)	41.6	32.9	32.7

Time (10 ^(teq/10)) 1:01:53 PM 39810.71706 1:02:53 PM 4073.802778 1:03:53 PM 2041.737945 1:05:53 PM 2137.96209 1:06:53 PM 1995.262315 1:07:53 PM 3715.352291 1:08:53 PM 1995.262315 1:09:53 PM 3162.27766 1:10:53 PM 3890.45145 1:11:53 PM 2454.708916 1:12:53 PM 2290.867653 1:14:53 PM 2290.867653 1:14:53 PM 2454.708916 1:13:53 PM 2570.395783 1:15:53 PM 3162.27766 1:17:53 PM 3162.27766 1:17:53 PM 2570.395783 1:18:53 PM 2570.395783 1:18:53 PM 2570.395783 1:18:53 PM 218.20.27766 1:12:53 PM 3162.27766 1:20:53 PM 3162.27766 1:21:53 PM 2570.395783 1:18:53 PM 3162.27766 1:21:53 PM 1995.262315 1:24:53 PM 3162.27766 1:25:53 PM 15135.61248 1:26:53 PM 5248.074602 1:27:53 PM 6760.829754 1:28:53 PM 7943.282347 1:30:53 PM 588.436554 1:35:53 PM 588		Baseline SPL
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	1:25:00	1:25:00		39.8	44.7	37	2:25:53 PN	
	1:26:00	1:26:00		37.7	38.7	36.8	2:26:53 PN	
	1:27:00	1:27:00		37.6	41	36.6	2:27:53 PN	
	1:28:00	1:28:00		35	37.7	31.7	2:28:53 PN	
	1:29:00	1:29:00		35.3	38.2	32.2	2:29:53 PN	3388.
	1:30:00	1:30:00		36.4	39.4	33.6	2:30:53 PN	4365.
	1:31:00	1:31:00		33.8	36.3	31.3	2:31:53 PN	2398.
	1:32:00	1:32:00		33.7	37.3	30.7	2:32:53 PN	2344.
	1:33:00	1:33:00		33.7	37.5	30.8	2:33:53 PN	2344.
	1:34:00	1:34:00		33.9	36.9	32.1	2:34:53 PN	
	1:35:00	1:35:00		33	35.3	31.4	2:35:53 PM	
	1:36:00	1:36:00		35.2	38.1	32	2:36:53 PN	
	1:37:00	1:37:00		34.8	39.8	32	2:37:53 PN	
	1:38:00	1:38:00		37	40.2	34.5	2:38:53 PN	
	1:39:00	1:39:00		36	39.5	32.1	2:39:53 PN	
	1:40:00	1:40:00		34.8	38	32.2	2:40:53 PN	
	1:41:00	1:41:00		34.9	38.2	32.6	2:41:53 PN	
	1:42:00	1:42:00		35.7	40.4	32.9	2:41:33 FN 2:42:53 PN	
	1:43:00	1:43:00		34.5	39	32.5		
							2:43:53 PN	
	1:44:00	1:44:00		35.7	39.4	32.2	2:44:53 PN	
	1:45:00	1:45:00		35.3	39.7	32.5	2:45:53 PN	
	1:46:00	1:46:00		33.7	37.3	31.6	2:46:53 PN	
	1:47:00	1:47:00		33.5	35.7	31.5	2:47:53 PN	
	1:48:00	1:48:00		33.1	35.4	31.7	2:48:53 PN	
	1:49:00	1:49:00		34.3	37.1	32.7	2:49:53 PN	
	1:50:00	1:50:00		33.8	39.2	31.7	2:50:53 PN	
	1:51:00	1:51:00		55.2	72.4	32	2:51:53 PN	
	1:52:00	1:52:00		38.9	46.3	33.4	2:52:53 PN	7762.
	1:53:00	1:53:00		37.6	44.7	32.1	2:53:53 PN	5754.
	1:54:00	1:54:00		37.5	47.3	33.2	2:54:53 PN	5623.
	1:55:00	1:55:00		38.9	50.2	31.7	2:55:53 PN	7762.
	1:56:00	1:56:00		37.1	46.4	32	2:56:53 PN	5128
	1:57:00	1:57:00		40.3	54	32.5	2:57:53 PN	10715
	1:58:00	1:58:00		33.5	39.3	31.7	2:58:53 PM	
	1:59:00	1:59:00		32.9	35	31.4	2:59:53 PN	
	2:00:00	2:00:00		33	35.3	31.4	3:00:53 PN	
	2:01:00	2:01:00		33.7	36	31.7	3:01:53 PN	
	2:02:00	2:02:00		36.6	46.3	31.5	3:02:53 PN	
	2:03:00	2:03:00		34.1	42.8	31.7	3:03:53 PN	
	2:04:00	2:04:00		38.2	44.3	32.2	3:04:53 PN	
	2:05:00	2:05:00		39.8	44.7	33.7	3:05:53 PN	
	2:06:00	2:06:00		33.9	37.2	31.7	3:06:53 PN	
	2:07:00	2:07:00		33.9	37.2	31.8	3:07:53 PN	
	2:08:00	2:08:00		33.2	43.4	31.2		
	2:09:00	2:09:00		32.5	34.2	31.4	3:08:53 PN	
	2:10:00	2:10:00		34.6	40.2	32.2	3:09:53 PN	
				33.7			3:10:53 PN	
	2:11:00	2:11:00			36	32.6	3:11:53 PN	
	2:12:00	2:12:00		33.5	34.8	32.2	3:12:53 PN	
	2:13:00	2:13:00		33.5	34.7	32.3	3:13:53 PN	
	2:14:00	2:14:00		33.4	35.3	32.4	3:14:53 PN	
	2:15:00	2:15:00		33	34.2	31.9	3:15:53 PN	
	2:16:00	2:16:00		34.4	36.2	32.8	3:16:53 PN	
	2:17:00	2:17:00		35.8	37.2	34.7	3:17:53 PM	
	2:18:00	2:18:00		34.6	36.7	33	3:18:53 PN	
	2:19:00	2:19:00		38.3	44.5	33.5	3:19:53 PN	6760.
	2:20:00	2:20:00		38.9	45.3	34.4	3:20:53 PN	7762.
	2:21:00	2:21:00		35.5	42.1	33.3	3:21:53 PN	3548.
	2:22:00	2:22:00		37.9	49	33.8	3:22:53 PN	6165.
	2:23:00	2:23:00		35.6	38.4	34	3:23:53 PM	
	2:24:00	2:24:00		41.6	45.6	34.1	3:24:53 PN	
	2:25:00	2:25:00		38	47.9	35.1	3:25:53 PN	
	2:26:00	2:26:00		38	42.5	35.8	3:26:53 PN	
	2:27:00	2:27:00		37	43.5	34.4	3:27:53 PN	
	2:28:00	2:28:00		41	47.3	36.1	3:28:53 PN	
	2:29:00	2:29:00		62.7	70.9	39.7	3:29:53 PN	
	2:30:00	2:30:00		47.6	54	37.2	3:30:53 PN	
	2:31:00	2:31:00		39.5	42.2	36.1	3:31:53 PN	
	2:32:00	2:32:00		39.5	42.2	35.9	3:32:53 PN	
	2:33:00	2:33:00		38.4	42.9	34.6		
				37.1	40.1	35.1	3:33:53 PN	
	2:34:00	2:34:00					3:34:53 PN	
	2:35:00	2:35:00		39.5		35.2	3:35:53 PN	
	2:36:00	2:36:00		40.6	47.3	36	3:36:53 PN	
	2:37:00	2:37:00		37.4	44.7	35.5	R1 Study #2 3:37:53 PN	
	2:38:00	2:38:00		37.7	44.2	35.3	R1 Study #2 3:38:53 PN	
	2:39:00	2:39:00		37.4	45	35.1	R1 Study #2 3:39:53 PN	
	2:40:00	2:40:00		38.2	46.7	35.3	R1 Study #2 3:40:53 PN	
	2:41:00	2:41:00		38.1	46.3	35.4	R1 Study #2 3:41:53 PN	6456
	2:42:00	2:42:00		43.5	60.8	36	R1 Study #2 3:42:53 PN	
	2:43:00	2:43:00		58.8	70.2	35.8	R1 Study #2 3:43:53 PN	
	2:44:00	2:44:00		53.6	66.3	36.1	R1 Study #2 3:44:53 PN	
	2:45:00	2:45:00		50.6	62.3	35.2	R1 Study #2 3:45:53 PN	
		2:46:00		53.4	65.7	34.8	R1 Study #2 3:46:53 PN	
	2:46:00							

Study	Study	Session	OL	L _{avg}	L _{max}	L _{min}			Deseline CDI
	2:48:00	2:48:00	Status	Meter1 36.5	Meter1 38.3	Meter1 35.2	R1 Study #2	3:48:53 PM	4466.835922
	2:49:00	2:49:00		37.6	44.8		R1 Study #2	3:49:53 PM	5754.399373
	2:50:00	2:50:00		35.1	39.2		R1 Study #2	3:50:53 PM	3235.936569
	2:51:00	2:51:00		35.1	38.1	34.2	R1 Study #2	3:51:53 PM	3235.936569
	2:52:00	2:52:00		35.2	36.7	34.1	R1 Study #2	3:52:53 PM	3311.311215
	2:53:00	2:53:00		35.4	36.8			3:53:53 PM	3467.368505
	2:54:00	2:54:00		35.9	39.4			3:54:53 PM	3890.45145
	2:55:00	2:55:00		35.2	37.5			3:55:53 PM	3311.311215
	2:56:00 2:57:00	2:56:00 2:57:00		35.1 35.4	36.4 40.6			3:56:53 PM	3235.936569 3467.368505
	2:58:00	2:58:00		36.8	42.2			3:57:53 PM 3:58:53 PM	4786.300923
	2:59:00	2:59:00		38.9	43.9			3:59:53 PM	7762.471166
	3:00:00	3:00:00		37.1	42.3	33.8		4:00:53 PM	5128.61384
	3:01:00	3:01:00		42.5	54.1	33.8		4:01:53 PM	17782.7941
	3:02:00	3:02:00		36.2	38.7			4:02:53 PM	4168.693835
	3:03:00	3:03:00		34.7	36.2			4:03:53 PM	2951.209227
	3:04:00	3:04:00		34	36.5			4:04:53 PM	2511.886432
	3:05:00 3:06:00	3:05:00 3:06:00		33.8 37.9	35.2 47.7			4:05:53 PM 4:06:53 PM	2398.832919 6165.950019
	3:07:00	3:07:00		41.1	51.7	34.2		4:07:53 PM	12882.49552
	3:08:00	3:08:00		42.4	55.9			4:08:53 PM	17378.00829
	3:09:00	3:09:00		38.7	47.1	33.9		4:09:53 PM	7413.102413
	3:10:00	3:10:00		39.9	49.7	34.2		4:10:53 PM	9772.37221
	3:11:00	3:11:00		35.9	39.1			4:11:53 PM	3890.45145
	3:12:00	3:12:00		35.2	38.4			4:12:53 PM	3311.311215
	3:13:00	3:13:00		35.1	37.7			4:13:53 PM	3235.936569
	3:14:00 3:15:00	3:14:00 3:15:00		34.6 35.4	36.3 36.8			4:14:53 PM	2884.031503 3467.368505
	3:16:00	3:16:00		35.5	37.7	34.3		4:15:53 PM 4:16:53 PM	3548.133892
	3:17:00	3:17:00		34.3	37.5			4:17:53 PM	2691.534804
	3:18:00	3:18:00		34.1	36.4			4:18:53 PM	2570.395783
	3:19:00	3:19:00		34.3	36.9	33		4:19:53 PM	2691.534804
	3:20:00	3:20:00		35.6	41.8			4:20:53 PM	3630.780548
	3:21:00	3:21:00		37.9	42.7	34.6		4:21:53 PM	6165.950019
	3:22:00	3:22:00		43.7	55.1			4:22:53 PM	23442.28815
	3:23:00 3:24:00	3:23:00 3:24:00		41.3 37.1	52.3 46.8	34.1 33.8		4:23:53 PM 4:24:53 PM	13489.62883 5128.61384
	3:25:00	3:25:00		35.3	39.6			4:25:53 PM	3388.441561
	3:26:00	3:26:00		34	35	32.9		4:26:53 PM	2511.886432
	3:27:00	3:27:00		35	37.6			4:27:53 PM	3162.27766
	3:28:00	3:28:00		36.1	40.9	33.2		4:28:53 PM	4073.802778
	3:29:00	3:29:00		35.6	40.5			4:29:53 PM	3630.780548
	3:30:00	3:30:00		37.4	40.1			4:30:53 PM	5495.408739
	3:31:00	3:31:00		33.8	35.8			4:31:53 PM	2398.832919
	3:32:00 3:33:00	3:32:00 3:33:00		34.4 33.4	40 34.6			4:32:53 PM	2754.228703 2187.761624
	3:34:00	3:34:00		33.3	35.4			4:33:53 PM 4:34:53 PM	2137.96209
	3:35:00	3:35:00		33.4	35.8			4:35:53 PM	2187.761624
	3:36:00	3:36:00		34.3	38.8	32.4		4:36:53 PM	2691.534804
	3:37:00	3:37:00		36.3	41.3	32.6		4:37:53 PM	4265.795188
	3:38:00	3:38:00		37.8	42.5			4:38:53 PM	6025.595861
	3:39:00	3:39:00		43.7	53	33.3		4:39:53 PM	23442.28815
	3:40:00	3:40:00		36.9	44.8			4:40:53 PM	4897.788194
	3:41:00 3:42:00	3:41:00 3:42:00		43.9 37.2	52.9 45.8			4:41:53 PM 4:42:53 PM	24547.08916 5248.074602
	3:43:00	3:42:00		33.5	35.2			4:42:53 PM 4:43:53 PM	2238.721139
	3:44:00	3:44:00		33.6	34.4			4:44:53 PM	2290.867653
	3:45:00	3:45:00		33.3	34			4:45:53 PM	2137.96209
	3:46:00	3:46:00		33.7	35.2			4:46:53 PM	2344.228815
	3:47:00	3:47:00		34.4	36.7			4:47:53 PM	2754.228703
	3:48:00	3:48:00		39.8	43.2			4:48:53 PM	9549.92586
	3:49:00 3:50:00	3:49:00 3:50:00		35.1 33.8	38.1 35.2			4:49:53 PM 4:50:53 PM	3235.936569 2398.832919
	3:51:00	3:51:00		35.4	37.4			4:50:53 PM 4:51:53 PM	3467.368505
	3:52:00	3:52:00		37.3	44.4			4:52:53 PM	5370.317964
	3:53:00	3:53:00		34.5	38.4			4:53:53 PM	2818.382931
	3:54:00	3:54:00		35.2	41.9	32.8		4:54:53 PM	3311.311215
	3:55:00	3:55:00		34.2	35.6			4:55:53 PM	2630.267992
	3:56:00	3:56:00		47.6	55.5			4:56:53 PM	57543.99373
	3:57:00	3:57:00		54.2	59.9			4:57:53 PM	263026.7992
	3:58:00 3:59:00	3:58:00 3:59:00		34 34 3	44			4:58:53 PM	2511.886432
	4:00:00	4:00:00		34.3 33.1	40.7 35.6			4:59:53 PM 5:00:53 PM	2691.534804 2041.737945
	4:01:00	4:01:00		32.1	33.0			5:01:53 PM	1621.810097
	4:02:00	4:02:00		38.2	45.5			5:02:53 PM	6606.93448
	4:03:00	4:03:00		48.3	60			5:03:53 PM	67608.29754
	4:04:00	4:04:00		43.3	51.3			5:04:53 PM	21379.6209
	4:05:00	4:05:00		37.2	40.7			5:05:53 PM	5248.074602
	4:06:00	4:06:00		35.2	39.2			5:06:53 PM	3311.311215
	4:07:00 4:08:00	4:07:00 4:08:00		49.7 40.9	56.4 49.5			5:07:53 PM	93325.43008
	4:08:00	4:08:00 4:09:00		40.9 33.9	49.5 34.6			5:08:53 PM 5:09:53 PM	12302.68771 2454.708916
	4:10:00	4:10:00		33.9 34.6	42.7			5:10:53 PM	2884.031503
	4:11:00	4:11:00		33.2	37.2			5:11:53 PM	2089.296131

tudy	Study	Session	OL	L _{avg}	L _{max}	L _{min}
	4:12:00	4:12:00	Status	Meter1 38.8	Meter1 49.3	Meter1 31
	4:13:00	4:13:00		37.8	46.6	
	4:14:00	4:14:00		33.3	41.9	31.7
	4:15:00	4:15:00		32.5	33.3	31.9
	4:16:00	4:16:00		33.7	35.9	32.3
	4:17:00	4:17:00		32.8	34.6	31.9
	4:18:00	4:18:00		33.4	38.7	31.9
	4:19:00	4:19:00		34.5	45	31.8
	4:20:00	4:20:00		32.7	34.8	31.9
	4:21:00	4:21:00		32.8	35	31.9
	4:22:00	4:22:00		33.6	36.2	32.6
	4:23:00	4:23:00		34.5	36.8	33
	4:24:00	4:24:00 4:25:00		34.3	35.5	33.5
	4:25:00 4:26:00	4:25:00		40.1 38	45.3 42.8	34 35.7
	4:27:00	4:27:00		41.3	46.7	33.7
	4:28:00	4:28:00		34.5	36	33.6
	4:29:00	4:29:00		38.7	41.9	35.4
	4:30:00	4:30:00		42.9	46.5	39
	4:31:00	4:31:00		37.2	39.4	36
	4:32:00	4:32:00		37.3	39.2	35.6
	4:33:00	4:33:00		34.6	35.9	33.7
	4:34:00	4:34:00		34.6	35.6	33.7
	4:35:00	4:35:00		35.1	36	33.8
	4:36:00	4:36:00		36.3	42.7	34.1
	4:37:00	4:37:00		35.3	38.1	34
	4:38:00	4:38:00		35.1	36.5	34.1
	4:39:00	4:39:00		35	37.2	34
	4:40:00	4:40:00		34.8	35.4	34.1
	4:41:00	4:41:00		35.4	36.5	34.5
	4:42:00	4:42:00		35.3	37.9	34
	4:43:00 4:44:00	4:43:00 4:44:00		34.8 34.4	36.2 35.4	33.8 33.7
	4:45:00	4:45:00		34.4	36.5	33.3
	4:46:00	4:46:00		35.2	39.6	33.9
	4:47:00	4:47:00		37	47.7	33.7
	4:48:00	4:48:00		34.4	35.8	
	4:49:00	4:49:00		34.1	35.3	32.8
	4:50:00	4:50:00		33.7	34.6	32.7
	4:51:00	4:51:00		34.3	36.1	33.2
	4:52:00	4:52:00		33.9	37.4	32.9
	4:53:00	4:53:00		34.6	36.6	33.2
	4:54:00	4:54:00		34.5	35.9	
	4:55:00	4:55:00		35	38.2	34.3
	4:56:00	4:56:00		34	34.8	33.4
	4:57:00	4:57:00		35	37.6	33.6
	4:58:00	4:58:00		34.7	35.6	
	4:59:00 5:00:00	4:59:00		35.1	36.2	33.9
	5:00:00	5:00:00		42.4	49.5	34.6
	5:02:00	5:01:00 5:02:00		35.8 36.5	37.8 37.7	34.4 35.2
	5:03:00	5:03:00		35.8	36.9	34.9
	5:04:00	5:04:00		37.6		
	5:05:00	5:05:00		35.8	39.3	
	5:06:00	5:06:00		34.9		
	5:07:00	5:07:00		35.5	36.1	
	5:08:00	5:08:00		35	36.4	
	5:09:00	5:09:00		39.9	46.9	
	5:10:00	5:10:00		44.2	52.4	33.8
	5:11:00	5:11:00		35.1	43.7	33
	5:12:00	5:12:00		38.2	41	
	5:13:00	5:13:00		35.1	36.8	
	5:14:00	5:14:00		34.2	35.5	
	5:15:00	5:15:00		35.5	37.6	
	5:16:00	5:16:00		35.9	38	
	5:17:00	5:17:00		35.7	37.9	
	5:18:00	5:18:00		34	35.9	
	5:19:00 5:20:00	5:19:00 5:20:00		34.2 36.4	37.2 38.6	
	5:20:00	5:20:00		35.4	38.6 37.3	
	5:22:00	5:22:00		34.2	36.2	
	5:23:00	5:23:00		33.8	35.7	
	5:24:00	5:24:00		33.9	34.9	
	5:25:00	5:25:00		32.6	33.8	
	5:26:00	5:26:00		32.9	34.8	
	5:27:00	5:27:00		33.7	35.2	
	5:28:00	5:28:00		37.6	41.6	
	5:29:00	5:29:00		39.3	42.3	
	5:30:00	5:30:00		38.6	40	
	5:31:00	5:31:00		38.2	39.6	
	5:32:00	5:32:00		38.2	39.8	37.5
	5:33:00	5:33:00		38	38.7	37.4
	5:34:00	5:34:00		38.4	40.9	37.5

	Baseline SPL
5:12:53 PM	7585.77575
5:13:53 PM	6025.595861
5:14:53 PM	2137.96209
5:15:53 PM 5:16:53 PM	1778.27941 2344.228815
5:17:53 PM	1905.460718
5:18:53 PM	2187.761624
5:19:53 PM	2818.382931
5:20:53 PM	1862.087137
5:21:53 PM 5:22:53 PM	1905.460718 2290.867653
5:23:53 PM	2818.382931
5:24:53 PM	2691.534804
5:25:53 PM	10232.92992
5:26:53 PM	6309.573445
5:27:53 PM 5:28:53 PM	13489.62883 2818.382931
5:29:53 PM	7413.102413
5:30:53 PM	19498.446
5:31:53 PM	5248.074602
5:32:53 PM 5:33:53 PM	5370.317964 2884.031503
5:34:53 PM	2884.031503
5:35:53 PM	3235.936569
5:36:53 PM	4265.795188
5:37:53 PM	3388.441561
5:38:53 PM 5:39:53 PM	3235.936569 3162.27766
5:40:53 PM	3019.95172
5:41:53 PM	3467.368505
5:42:53 PM	3388.441561
5:43:53 PM 5:44:53 PM	3019.95172 2754.228703
5:44:53 PM 5:45:53 PM	2884.031503
5:46:53 PM	3311.311215
5:47:53 PM	5011.872336
5:48:53 PM	2754.228703
5:49:53 PM 5:50:53 PM	2570.395783 2344.228815
5:51:53 PM	2691.534804
5:52:53 PM	2454.708916
5:53:53 PM	2884.031503
5:54:53 PM 5:55:53 PM	2818.382931 3162.27766
5:55:53 PM 5:56:53 PM	2511.886432
5:57:53 PM	3162.27766
5:58:53 PM	2951.209227
5:59:53 PM	3235.936569
6:00:53 PM 6:01:53 PM	17378.00829 3801.893963
6:02:53 PM	4466.835922
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6:12:53 PM	6606.93448
6:13:53 PM	3235.936569
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study	Time	Time	Status	Meter1	Meter1	Meter1
	5:36:00	5:36:00	•	38.2	38.9	37.6
	5:37:00	5:37:00		38.2	39.7	37.9
	5:38:00	5:38:00		38.7	39.6	38.3
	5:39:00	5:39:00		38.7	40	38.3
	5:40:00	5:40:00		39.4	40.8	38
	5:41:00	5:41:00		38.6	39.6	37.
	5:42:00 5:43:00	5:42:00 5:43:00		39.1 38.3	40.5 39.2	38.2
	5:44:00	5:44:00		39.3	40.5	37.6 37.8
	5:45:00	5:45:00		38.9	40.7	37.3
	5:46:00	5:46:00		39.5	42.7	37.5
	5:47:00	5:47:00		38.1	38.8	37.3
	5:48:00	5:48:00		38.3	39.9	37.
	5:49:00	5:49:00		38.2	39.2	37.
	5:50:00	5:50:00		38.3	40.1	37.6
	5:51:00	5:51:00		37.9	38.3	37.5
	5:52:00	5:52:00		38.1	38.8	37.4
	5:53:00	5:53:00		38.2	40.3	37.6
	5:54:00	5:54:00		38.5	39.4	37.
	5:55:00	5:55:00		38.6	39.6	3
	5:56:00	5:56:00		44.2	47	39.
	5:57:00	5:57:00		41.1	46.3	38.
	5:58:00	5:58:00		38.1	38.8	37.0
	5:59:00 6:00:00	5:59:00 6:00:00		38 38.3	39.3 41.5	37. 37.
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	6:02:00	6:02:00		36.4 37.6	37.9	37. 37.
	6:03:00	6:03:00		37.9	39.8	3
	6:04:00	6:04:00		38.6	41	37.
	6:05:00	6:05:00		39.1	41.7	37.
	6:06:00	6:06:00		39.1	43.1	37.
	6:07:00	6:07:00		38.4	40.1	37.
	6:08:00	6:08:00		38.6	41.1	37.
	6:09:00	6:09:00		39.7	41.2	38.
	6:10:00	6:10:00		38.4	39.4	37.
	6:11:00	6:11:00		38.4	39.5	37.
	6:12:00	6:12:00		38.4	39.1	37.
	6:13:00	6:13:00		38.1	39.7	37.
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	6:17:00	6:17:00		38	38.8	37.
	6:18:00	6:18:00		38.3	41	37.
	6:19:00	6:19:00		37.9	40.2	37.
	6:20:00	6:20:00		37.5	38.4	3
	6:21:00	6:21:00		37.9	40.9	36.
	6:22:00	6:22:00		37.8	39.3	3
	6:23:00	6:23:00		38.2	40.9	37.
	6:24:00	6:24:00		38.5	41.7	37.
	6:25:00	6:25:00		38.7	40.8	37.
	6:26:00	6:26:00		38.3	39.4	37.
	6:27:00	6:27:00		38.8	41.3	37.
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	6:32:00	6:32:00		36.2	38.5	34.
	6:33:00	6:33:00		35.9	39.4	32.
	6:34:00	6:34:00		37.9	40.9	
	6:35:00	6:35:00		39.1	41.5	35.
	6:36:00	6:36:00		38.2	43.2	34.
	6:37:00	6:37:00		35.7	38.4	33.
	6:38:00	6:38:00		35.7	40.6	31.
	6:39:00	6:39:00		40	52.6	33.
	6:40:00	6:40:00		34.5	37.9	32.
	6:41:00	6:41:00		34.6	37.8	32.
	6:42:00	6:42:00		34.7	36.5	32.
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	6:48:00	6:48:00		33.2	35.1	31.
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	6:50:00	6:50:00		32.4	33.0	31.
	6:51:00	6:51:00		32.1	32.9	31.
	6:52:00	6:52:00		32.5	33.8	31.
	6:53:00	6:53:00		32.5	33.7	31.
	6:54:00	6:54:00		32.5	33.9	
	6:55:00	6:55:00		31.8	32.5	31.
	6:56:00	6:56:00		36.6	47.7	31.
	6:57:00	6:57:00		32.7	36.8	31.
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Study	Study	Session	OL	Lavg	L _{max}	L _{min}
Study	Time	Time	Status	Meter1	Meter1	Meter1
	7:00:00	7:00:00		34.3	38.2	32.3
	7:01:00 7:02:00	7:01:00 7:02:00		34.7 34.8	38.2 40.9	32.9 31.9
	7:02:00	7:02:00		33.2	34.7	31.9
	7:04:00	7:04:00		32.7	34.2	31.9
	7:05:00	7:05:00		32.9	34.5	31.7
	7:06:00	7:06:00		33.4	35.3	32.6
	7:07:00	7:07:00		34.8	39.3	32.3
	7:08:00	7:08:00		33.9	35.9	32.5
	7:09:00	7:09:00		37.6	45.1	32.7
	7:10:00	7:10:00		36.6	40.8 33.7	32.4 31.4
	7:11:00 7:12:00	7:11:00 7:12:00		32.7 32.9	34.3	31.4
	7:13:00	7:13:00		32.9	34.2	32.2
	7:14:00	7:14:00		33.9	36.9	32.5
	7:15:00	7:15:00		40	43.9	35.2
	7:16:00	7:16:00		36.9	43.4	32.3
	7:17:00	7:17:00		33.2	34.3	32.2
	7:18:00	7:18:00		35.4	36.9	33.3
	7:19:00 7:20:00	7:19:00 7:20:00		39.2 40.9	44.7 47.7	35.2 34.4
	7:21:00	7:21:00		34.7	36.5	33.2
	7:22:00	7:22:00		33	34.8	32.2
	7:23:00	7:23:00		45.3	51.4	33.3
	7:24:00	7:24:00		33.6	37.7	32.5
	7:25:00	7:25:00		34.6	39.5	32.3
	7:26:00	7:26:00		33.1	33.8	32.6
	7:27:00 7:28:00	7:27:00 7:28:00		32.7 33.4	34 35.1	31.9 31.9
	7:29:00	7:28:00		33.4	35.1	32.2
	7:30:00	7:30:00		33.1	35.2	32.4
	7:31:00	7:31:00		34.3	40.9	32.1
	7:32:00	7:32:00		33.4	34.7	32.3
	7:33:00	7:33:00		33.5	35.7	32.3
	7:34:00	7:34:00		33.6	39.3	32.3
	7:35:00 7:36:00	7:35:00 7:36:00		33.8 34.6	35.1 36.5	32.5 33.5
	7:37:00	7:37:00		34.0	35.3	33.2
	7:38:00	7:38:00		37.8	39.6	34.1
	7:39:00	7:39:00		35.8	39.3	34.1
	7:40:00	7:40:00		36.1	40.9	33.3
	7:41:00	7:41:00		34.5	36.1	32.9
	7:42:00	7:42:00		34.1	35.6	32.9
	7:43:00 7:44:00	7:43:00 7:44:00		33.7 33.4	35.1 34.4	32.7 32.5
	7:45:00	7:44:00		33.4	34.4	32.3
	7:46:00	7:46:00		34.6	38	33
	7:47:00	7:47:00		35.7	38	34.4
	7:48:00	7:48:00		36.5	41.6	34.8
	7:49:00	7:49:00		36.8	41.7	34.6
	7:50:00	7:50:00		36.5	38.1	35.5
	7:51:00 7:52:00	7:51:00 7:52:00		37 36.5	38.7 40	35.2 34.5
	7:53:00	7:53:00		36.7	38.2	34.3
	7:54:00	7:54:00		37.7	40.2	
	7:55:00	7:55:00		36.9	39.3	
	7:56:00	7:56:00		37.6	45.9	
	7:57:00	7:57:00		35	39.5	
	7:58:00	7:58:00		33.8	35.5	
	7:59:00 8:00:00	7:59:00 8:00:00		34.6 35.2	35.8 37.9	
	8:00:00	8:00:00		35.2 35.3	37.9 37	
	8:02:00	8:02:00		35.2	37.8	
	8:03:00	8:03:00		35.2	36.5	34.3
	8:04:00	8:04:00		35.9	39.6	33.4
	8:05:00	8:05:00		34.6	36.6	33.3
	8:06:00	8:06:00		33.1	34.5	32
	8:07:00	8:07:00		33.4	35.5	
	8:08:00 8:09:00	8:08:00 8:09:00		34 33.1	36.9 36.8	
	8:10:00	8:10:00		33.7	35.8	
	8:11:00	8:11:00		35.9	37.3	
	8:12:00	8:12:00		35.1	35.9	
	8:13:00	8:13:00		34.8	36.8	
	8:14:00	8:14:00		34.9	36.8	
	8:15:00	8:15:00		35.3	37.8	
	8:16:00	8:16:00		34.8	37.1	32.6
	8:17:00	8:17:00		34.9	38.4	
	8:18:00 8:19:00	8:18:00 8:19:00		32.8 33.5	35.5 35.9	31.7 31.6
	8:19:00	8:19:00		33.5	35.9 38.5	
	8:21:00	8:21:00		34.6		
	8:22:00	8:22:00		34	37.4	
				33.1		

	Baseline SPL
8:00:53 PM	2691.534804
8:01:53 PM	2951.209227
8:02:53 PM	3019.95172
8:03:53 PM 8:04:53 PM	2089.296131 1862.087137
8:05:53 PM	1949.8446
8:06:53 PM	2187.761624
8:07:53 PM 8:08:53 PM	3019.95172 2454.708916
8:09:53 PM	5754.399373
8:10:53 PM	4570.881896
8:11:53 PM 8:12:53 PM	1862.087137 1949.8446
8:12:53 PM 8:13:53 PM	1949.8446
8:14:53 PM	2454.708916
8:15:53 PM	10000
8:16:53 PM 8:17:53 PM	4897.788194 2089.296131
8:18:53 PM	3467.368505
8:19:53 PM	8317.637711
8:20:53 PM 8:21:53 PM	12302.68771 2951.209227
8:22:53 PM	1995.262315
8:23:53 PM	33884.41561
8:24:53 PM	2290.867653
8:25:53 PM	2884.031503 2041.737945
8:26:53 PM 8:27:53 PM	1862.087137
8:28:53 PM	2187.761624
8:29:53 PM	2290.867653
8:30:53 PM 8:31:53 PM	2041.737945 2691.534804
8:32:53 PM	2187.761624
8:33:53 PM	2238.721139
8:34:53 PM	2290.867653
8:35:53 PM 8:36:53 PM	2398.832919 2884.031503
8:37:53 PM	2630.267992
8:38:53 PM	6025.595861
8:39:53 PM 8:40:53 PM	3801.893963 4073.802778
8:41:53 PM	2818.382931
8:42:53 PM	2570.395783
8:43:53 PM 8:44:53 PM	2344.228815 2187.761624
8:45:53 PM	2187.761624
8:46:53 PM	2884.031503
8:47:53 PM	3715.352291
8:48:53 PM 8:49:53 PM	4466.835922 4786.300923
8:50:53 PM	4466.835922
8:51:53 PM	5011.872336
8:52:53 PM 8:53:53 PM	4466.835922 4677.351413
8:54:53 PM	5888.436554
8:55:53 PM	4897.788194
8:56:53 PM 8:57:53 PM	5754.399373 3162.27766
8:58:53 PM	2398.832919
8:59:53 PM	2884.031503
9:00:53 PM 9:01:53 PM	3311.311215 3388.441561
9:02:53 PM	3311.311215
9:03:53 PM	3311.311215
9:04:53 PM 9:05:53 PM	3890.45145 2884.031503
9:06:53 PM	2041.737945
9:07:53 PM	2187.761624
9:08:53 PM	2511.886432 2041.737945
9:09:53 PM 9:10:53 PM	2344.228815
9:11:53 PM	3890.45145
9:12:53 PM	3235.936569
9:13:53 PM 9:14:53 PM	3019.95172 3090.295433
9:15:53 PM	3388.441561
9:16:53 PM	3019.95172
9:17:53 PM 9:18:53 PM	3090.295433 1905.460718
9:19:53 PM	2238.721139
9:20:53 PM	3019.95172
9:21:53 PM 9:22:53 PM	2884.031503 2511.886432
9:23:53 PM	2041.737945

Study	Study	Session	OL Status	L _{avg}	L _{max}	L _{min}
	8:24:00	8:24:00	Status	Meter1 34.7	Meter1 37.2	Meter1 32.8
	8:25:00	8:25:00		37.1	40.3	32.8
	8:26:00	8:26:00		35	38.8	32.3
	8:27:00	8:27:00		36.5	40	33.2
	8:28:00	8:28:00		34.1	36.2	32.1
	8:29:00	8:29:00		34.9	38.8	31.2
	8:30:00	8:30:00		35.2	39.1	31.6
	8:31:00	8:31:00		32	33.2	31.1
	8:32:00	8:32:00		31.7	33	30.9
	8:33:00	8:33:00		31.7	32.8	30.5
	8:34:00	8:34:00		32.9	33.9	31.7
	8:35:00	8:35:00		32.6	36.7	31
	8:36:00	8:36:00		31.5	32.8	30.7
	8:37:00	8:37:00		31.9	32.7	31
	8:38:00	8:38:00		32.5	34.3	31.3
	8:39:00	8:39:00		32.1	33.5	31.5
	8:40:00	8:40:00		32.5	35.1	31.4
	8:41:00	8:41:00		32.5	34.3	31.7
	8:42:00 8:43:00	8:42:00 8:43:00		41.5 40.5	46.8 46.5	33.2 34.6
	8:44:00	8:44:00		35	36.3	33.6
	8:45:00	8:45:00		33.6	35.9	31.7
	8:46:00	8:46:00		32.2	33.9	31.3
	8:47:00	8:47:00		34.6	38.3	32.2
	8:48:00	8:48:00		36.1	40.5	31.8
	8:49:00	8:49:00		35.2	40.9	32.8
	8:50:00	8:50:00		35.8	41.9	31.3
	8:51:00	8:51:00		34.1	37.7	31.6
	8:52:00	8:52:00		35	38.6	32
	8:53:00	8:53:00		33.8	39.3	31.4
	8:54:00	8:54:00		32.8	36.8	31.5
	8:55:00	8:55:00		33.5	36.5	31.3
	8:56:00	8:56:00		33	35.5	31.3
	8:57:00	8:57:00		34.9	38.5	32.4
	8:58:00	8:58:00		36.9	39.9	33.4
	8:59:00	8:59:00		37.4	41.3	34.4
	9:00:00	9:00:00		37.3	40.8	33.7
	9:01:00	9:01:00		37.6	41.5	33.8
	9:02:00	9:02:00		35.4	37.9	33
	9:03:00	9:03:00		36.3	38.8	34
	9:04:00 9:05:00	9:04:00		37.7	40.6	35.3
	9:06:00	9:05:00 9:06:00		35.1 35	38.1 38.1	32.5 33.4
	9:07:00	9:07:00		34.8	36.4	33.6
	9:08:00	9:08:00		35.5	42	33.0
	9:09:00	9:09:00		33.8	35.6	32.4
	9:10:00	9:10:00		36.2	39	33.6
	9:11:00	9:11:00		35.2	38.6	33.:
	9:12:00	9:12:00		34.9	37.7	32.8
	9:13:00	9:13:00		34.9	39.1	32.9
	9:14:00	9:14:00		35.4	37.6	33.3
	9:15:00	9:15:00		33.8	38.2	31.4
	9:16:00	9:16:00		33.9	36.7	31.9
	9:17:00	9:17:00		33.8	35.9	32
	9:18:00	9:18:00		33	35.1	30.6
	9:19:00	9:19:00		34	36.1	32.4
	9:20:00	9:20:00		34.8	37.2	32.8
	9:21:00	9:21:00		35.9	40.5	33.4
	9:22:00	9:22:00		37.3	40.9	32.
	9:23:00	9:23:00		34.1	37.1	32.5
	9:24:00	9:24:00		33.9	37.2	32.3
	9:25:00	9:25:00		37.7	41	33.3
	9:26:00	9:26:00		36.5	40.2	33.3
	9:27:00 9:28:00	9:27:00 9:28:00		34 36.2	38 39	31.8 33.9
	9:29:00	9:29:00		35.9	41.3	32.
	9:30:00	9:30:00		37.5	41.6	33.
	9:31:00	9:31:00		35.6	39.4	32.
	9:32:00	9:32:00		36.5	39.4	32.
	9:33:00	9:33:00		36.4	40	33.
	9:34:00	9:34:00		40.1	46.7	33.
	9:35:00	9:35:00		37	39.2	34.
	9:36:00	9:36:00		34.8	38.6	31.4
	9:37:00	9:37:00		35.4	39.1	3
	9:38:00	9:38:00		33.8	37	3:
	9:39:00	9:39:00		35.8	39.9	31.
	9:40:00	9:40:00		34.2	37.3	3:
	9:41:00	9:41:00		34.5	36.4	32.
	9:42:00	9:42:00		33.8		31.
	9:43:00	9:43:00		33.4	36.5	31.
	9:44:00	9:44:00		32.4	35.1	30.
	9:45:00	9:45:00		35.5	39.1	33.5
	9:46:00	9:46:00		37.7	41.8	32.
	5.40.00	3.40.00				

	Baseline SPL
9:24:53 PM	2951.209227
9:25:53 PM	5128.61384
9:26:53 PM	3162.27766
9:27:53 PM 9:28:53 PM	4466.835922 2570.395783
9:29:53 PM	3090.295433
9:30:53 PM	3311.311215
9:31:53 PM 9:32:53 PM	1584.893192 1479.108388
9:33:53 PM	1479.108388
9:34:53 PM	1949.8446
9:35:53 PM	1819.700859
9:36:53 PM 9:37:53 PM	1412.537545 1548.816619
9:38:53 PM	1778.27941
9:39:53 PM	1621.810097
9:40:53 PM 9:41:53 PM	1778.27941 1778.27941
9:42:53 PM	14125.37545
9:43:53 PM	11220.18454
9:44:53 PM 9:45:53 PM	3162.27766 2290.867653
9:46:53 PM	1659.586907
9:47:53 PM	2884.031503
9:48:53 PM	4073.802778
9:49:53 PM 9:50:53 PM	3311.311215 3801.893963
9:51:53 PM	2570.395783
9:52:53 PM	3162.27766
9:53:53 PM 9:54:53 PM	2398.832919 1905.460718
9:55:53 PM	2238.721139
9:56:53 PM	1995.262315
9:57:53 PM 9:58:53 PM	3090.295433 4897.788194
9:59:53 PM	5495.408739
10:00:53 PM	5370.317964
10:01:53 PM 10:02:53 PM	5754.399373 3467.368505
10:02:53 PM	4265.795188
10:04:53 PM	5888.436554
10:05:53 PM	3235.936569
10:06:53 PM 10:07:53 PM	3162.27766 3019.95172
10:08:53 PM	3548.133892
10:09:53 PM	2398.832919
10:10:53 PM 10:11:53 PM	4168.693835 3311.311215
10:12:53 PM	3090.295433
10:13:53 PM	3090.295433
10:14:53 PM 10:15:53 PM	3467.368505 2398.832919
10:16:53 PM	2454.708916
10:17:53 PM	2398.832919
10:18:53 PM 10:19:53 PM	1995.262315 2511.886432
10:20:53 PM	3019.95172
10:21:53 PM	3890.45145
10:22:53 PM 10:23:53 PM	5370.317964 2570.395783
10:24:53 PM	2454.708916
10:25:53 PM	5888.436554
10:26:53 PM 10:27:53 PM	4466.835922 2511.886432
10:28:53 PM	4168.693835
10:29:53 PM	3890.45145
10:30:53 PM 10:31:53 PM	5623.413252 3630.780548
10:32:53 PM	4466.835922
10:33:53 PM	4365.158322
10:34:53 PM 10:35:53 PM	10232.92992 5011.872336
10:36:53 PM	3011.872330
10:37:53 PM	3467.368505
10:38:53 PM 10:39:53 PM	2398.832919 3801.893963
10:39:53 PM	2630.267992
10:41:53 PM	2818.382931
10:42:53 PM	2398.832919 2187.761624
10:43:53 PM 10:44:53 PM	1737.800829
10:45:53 PM	3548.133892
10:46:53 PM 10:47:53 PM	5888.436554 2951.209227
_0	2551.205221

tudy	Study	Session	OL	L _{avg}	L _{max}	L _{min}
,	Time	Time	Status	Meter1	Meter1	Meter1
	9:48:00 9:49:00	9:48:00 9:49:00		35.5 34.4	39.5 37.9	32.1 31.7
	9:50:00	9:50:00		34.4	37.9	31.4
	9:51:00	9:51:00		34.8	36.8	31.8
	9:52:00	9:52:00		34.1	36.8	32.2
	9:53:00	9:53:00		35.4	38.4	32.5
	9:54:00	9:54:00		34.8	39.1	30.9
	9:55:00	9:55:00		33.6	37.6	30.4
	9:56:00	9:56:00		34	38.1	31.2
	9:57:00	9:57:00		32.3	34.5	30.5
	9:58:00	9:58:00		31.7	34	30.3
	9:59:00	9:59:00		31.2	33.6	29.8
	10:00:00	10:00:00		33.7	38.6	30.3
	10:01:00	10:01:00		35.3	40.4	31.6
	10:02:00	10:02:00		34.2	37.6	32.
	10:03:00	10:03:00		34.4	37.5	32
	10:04:00	10:04:00		33.6	35.8	31.
	10:05:00	10:05:00		34.7	37.6	31.9
	10:06:00	10:06:00		33.8	38.9	30.
	10:07:00	10:07:00		31.1	32.7	30
	10:08:00	10:08:00		33.2	36.8	30.
	10:09:00	10:09:00		34.4	38.8	30.4
	10:10:00	10:10:00		36.7	39	31.
	10:11:00	10:11:00		32.8	36.6	29.
	10:12:00	10:12:00		34	39.2	29.
	10:13:00	10:13:00		31.8	36	29.
	10:14:00 10:15:00	10:14:00 10:15:00		33.5 34.1	38.7	29. 32.
	10:15:00	10:15:00		34.2	36.4 37.9	32.4
	10:17:00	10:17:00		37.1	39.7	34.9
	10:18:00	10:18:00		37.2	39.7	34
	10:19:00	10:19:00		36.4	43.5	30.
	10:20:00	10:20:00		31.1	33.2	29.4
	10:21:00	10:21:00		32.5	37.5	29.
	10:22:00	10:22:00		36.9	39.8	30.
	10:23:00	10:23:00		32.4	37.3	29.
	10:24:00	10:24:00		30.7	33.7	29
	10:25:00	10:25:00		31.9	35.9	29.
	10:26:00	10:26:00		29.7	32.4	2
	10:27:00	10:27:00		28.6	29.4	2
	10:28:00	10:28:00		28.8	30.6	28.
	10:29:00	10:29:00		28.3	29.6	27.
	10:30:00	10:30:00		29	31.6	27.
	10:31:00	10:31:00		31.3	37.8	28.
	10:32:00	10:32:00		30.2	34.4	28.
	10:33:00	10:33:00		31	34.1	29.
	10:34:00	10:34:00		32.9	36.7	29.
	10:35:00	10:35:00 10:36:00		30.1	34.3	29.:
	10:36:00 10:37:00			31.7 30	34.4	30 28.
	10:37:00	10:37:00 10:38:00		29	33.2 29.5	28.
	10:39:00	10:39:00		29.2	30.1	28.
	10:40:00	10:33:00		29.6	30.1	28.
	10:41:00	10:41:00		29.6	32.1	
		10:42:00		29.9		
		10:43:00		31.8		
		10:44:00		35.9	43.3	
	10:45:00	10:45:00		38.5	45.2	
	10:46:00	10:46:00		37.4	44.2	29.
	10:47:00	10:47:00		29.7	34.5	28.
	10:48:00	10:48:00		29.4	30.3	28.9
	10:49:00	10:49:00		29.4	30	28.
	10:50:00	10:50:00		29.7	30.9	2
	10:51:00	10:51:00		30.9	32.5	29.
	10:52:00	10:52:00		30.3	32.9	2
	10:53:00	10:53:00		30.4	33.3	
		10:54:00		29.1	29.8	
	10:55:00			30.1	31.9	
	10:56:00			29.6	32.4	
		10:57:00		29	29.7	
	10:58:00	10:58:00		29.2	33	
	10:59:00			29.3	32	
		11:00:00		29.2	30.5	
	11:01:00	11:01:00		31.3	33.7	
	11:02:00	11:02:00		29.8		
	11:03:00			29.7	32.7	
	11:04:00			30	33.2	28.
	11:05:00	11:05:00		29.2	30.9	
	11:06:00	11:06:00		29.8	31.7	
		11:07:00		30.3	32.4	
	11:08:00	11:08:00		29.5	31	
	11:09:00	11:09:00		30.9	34.3	28.
	11:10:00 11:11:00	11:10:00 11:11:00		30.2 30.9	33.5 34.9	

	Baseline SPL
10:48:53 PM	3548.133892
10:49:53 PM	2754.228703
10:50:53 PM	2691.534804
10:51:53 PM 10:52:53 PM	3019.95172 2570.395783
10:53:53 PM	3467.368505
10:54:53 PM	3019.95172
10:55:53 PM 10:56:53 PM	2290.867653 2511.886432
10:57:53 PM	1698.243652
10:58:53 PM	1479.108388
10:59:53 PM 11:00:53 PM	1318.256739 2344.228815
11:01:53 PM	3388.441561
11:02:53 PM	2630.267992
11:03:53 PM 11:04:53 PM	2754.228703 2290.867653
11:05:53 PM	2951.209227
11:06:53 PM	2398.832919 1288.249552
11:07:53 PM 11:08:53 PM	2089.296131
11:09:53 PM	2754.228703
11:10:53 PM	4677.351413
11:11:53 PM 11:12:53 PM	1905.460718 2511.886432
11:13:53 PM	1513.561248
11:14:53 PM 11:15:53 PM	2238.721139 2570.395783
11:16:53 PM	2630.267992
11:17:53 PM	5128.61384
11:18:53 PM 11:19:53 PM	5248.074602 4365.158322
11:20:53 PM	1288.249552
11:21:53 PM	1778.27941
11:22:53 PM 11:23:53 PM	4897.788194 1737.800829
11:24:53 PM	1174.897555
11:25:53 PM	1548.816619
11:26:53 PM 11:27:53 PM	933.2543008 724.4359601
11:28:53 PM	758.577575
11:29:53 PM	676.0829754
11:30:53 PM 11:31:53 PM	794.3282347 1348.962883
11:32:53 PM	1047.128548
11:33:53 PM 11:34:53 PM	1258.925412 1949.8446
11:35:53 PM	1023.292992
11:36:53 PM	1479.108388
11:37:53 PM 11:38:53 PM	1000 794.3282347
11:39:53 PM	831.7637711
11:40:53 PM	912.0108394
11:41:53 PM 11:42:53 PM	912.0108394 977.237221
11:43:53 PM	1513.561248
11:44:53 PM 11:45:53 PM	3890.45145
11:45:53 PM	7079.457844 5495.408739
11:47:53 PM	933.2543008
11:48:53 PM 11:49:53 PM	870.96359 870.96359
11:50:53 PM	933.2543008
11:51:53 PM	1230.268771
11:52:53 PM 11:53:53 PM	1071.519305 1096.478196
11:54:53 PM	812.8305162
11:55:53 PM	1023.292992
11:56:53 PM 11:57:53 PM	912.0108394 794.3282347
11:58:53 PM	831.7637711
11:59:53 PM	851.1380382
12:00:53 AM 12:01:53 AM	831.7637711 1348.962883
12:02:53 AM	954.992586
12:03:53 AM	933.2543008 1000
12:04:53 AM 12:05:53 AM	831.7637711
12:06:53 AM	954.992586
12:07:53 AM 12:08:53 AM	1071.519305 891.2509381
12:08:53 AM	1230.268771
12:10:53 AM	1047.128548
12:11:53 AM	1230.268771

Study	Study	Session	OL .	Lavg	L _{max}	L _{min}
• • •	Time	Time	Status	Meter1	Meter1	Meter1
	11:12:00 11:13:00	11:12:00 11:13:00		29.8 31.1	32.6 33.9	29.5 29.5
	11:14:00	11:14:00		30.4	34.4	28.7
	11:15:00	11:15:00		30	32.6	28.7
	11:16:00	11:16:00		29.1	30.7	28.2
	11:17:00	11:17:00		29.3	31.1	28.3
	11:18:00	11:18:00		31.3	33	29.7
	11:19:00	11:19:00		30.7	36.8	29
	11:20:00 11:21:00	11:20:00 11:21:00		29.9	31	28.8
	11:21:00	11:22:00		30.1 31.4	31.5 33.8	29.3 29.8
	11:23:00	11:23:00		32.4	34.8	30.4
	11:24:00	11:24:00		33	36.9	30.2
	11:25:00	11:25:00		31.4	33.4	29.9
	11:26:00	11:26:00		39.3	51	30.5
	11:27:00	11:27:00		31.5	33.7	29.2
	11:28:00	11:28:00		30.3	32	28.9
	11:29:00 11:30:00	11:29:00 11:30:00		30.9 30.9	34.5 34.2	29.3 28.9
	11:31:00	11:31:00		37.9	47.5	29.8
	11:32:00	11:32:00		31.2	36.8	28.6
	11:33:00	11:33:00		30.3	36.2	28.2
	11:34:00	11:34:00		30.7	34	28.2
	11:35:00	11:35:00		30.3	32.8	28.
	11:36:00	11:36:00		28.5	29.4	28.:
	11:37:00 11:38:00	11:37:00 11:38:00		28.5	29	28.:
	11:38:00	11:39:00		28.3 29.8	29.2 33.3	27.8 28.3
	11:40:00	11:40:00		29.8	33.6	28.
	11:41:00	11:41:00		30.6	34.6	28.
	11:42:00	11:42:00		29	31.9	28.
	11:43:00	11:43:00		28.4	29.2	28
	11:44:00	11:44:00		29.5	36.3	27.
	11:45:00	11:45:00		28.4	31.2	27.
	11:46:00 11:47:00	11:46:00 11:47:00		28.1 28	31.5 30	27. 27.
	11:48:00	11:48:00		28.4	31.7	27.
	11:49:00	11:49:00		28.4	32.3	27.
	11:50:00	11:50:00		27.5	28.6	27.
	11:51:00	11:51:00		27.5	27.9	27.
	11:52:00	11:52:00		27.6	28	27.
	11:53:00	11:53:00		27.8	28.2	27.
	11:54:00	11:54:00		28.3	29.3	27.
	11:55:00 11:56:00	11:55:00 11:56:00		29.4 29.9	32.1 32	27.9 28.8
	11:57:00	11:57:00		33.3	40.6	29.
	11:58:00	11:58:00		35.7	39.8	31.
	11:59:00	11:59:00		33.2	37.3	30.
	12:00:00	12:00:00		33.5	37	30.
	12:01:00	12:01:00		31.8	34	30.
	12:02:00	12:02:00		30.4	32.6	28.
	12:03:00	12:03:00 12:04:00		29	30 33.3	28. 28.
		12:04:00		29.9 29.4	30.7	28.
		12:06:00		29.3		
		12:07:00		30.1	32.6	
	12:08:00	12:08:00		29.8	31.1	28.
		12:09:00		29	30.1	28.
		12:10:00		28.8	29.4	
		12:11:00		28.7	29.3	28.
		12:12:00 12:13:00		28.5 28.6	29	
		12:14:00		28.9	29.1 29.4	28. 28.
		12:15:00		28.7	29.6	2
		12:16:00		28.6	29	
	12:17:00	12:17:00		28.1	28.4	27.
		12:18:00		29	31.3	28.
		12:19:00		28.7	30.1	28.
		12:20:00		28.3	28.9	
		12:21:00 12:22:00		28 28.1	28.4 28.6	27. 27.
		12:22:00		28.4	31.3	
		12:24:00		28.3	29.5	27.
		12:25:00		29.1	30.5	28.
		12:26:00		30.4	31.8	
		12:27:00		29	29.9	
	12:28:00	12:28:00		28.6	29.4	2
		12:29:00		28	28.6	
		12:30:00		28.4	29.2	
		12:31:00		28.6	29.5	2
		12:32:00		29.1	29.9	
		12:33:00 12:34:00		30.6 28.4	34.7 29.5	2 27.

	Baseline SPL
12:12:53 AM	954.992586
12:13:53 AM	1288.249552
12:14:53 AM	1096.478196
12:15:53 AM 12:16:53 AM	1000 812.8305162
12:17:53 AM	851.1380382
12:18:53 AM	1348.962883
12:19:53 AM	1174.897555
12:20:53 AM	977.237221 1023.292992
12:21:53 AM 12:22:53 AM	1380.384265
12:23:53 AM	1737.800829
12:24:53 AM	1995.262315
12:25:53 AM 12:26:53 AM	1380.384265 8511.380382
12:27:53 AM	1412.537545
12:28:53 AM	1071.519305
12:29:53 AM	1230.268771
12:30:53 AM 12:31:53 AM	1230.268771 6165.950019
12:32:53 AM	1318.256739
12:33:53 AM	1071.519305
12:34:53 AM	1174.897555
12:35:53 AM 12:36:53 AM	1071.519305 707.9457844
12:37:53 AM	707.9457844
12:38:53 AM	676.0829754
12:39:53 AM	954.992586
12:40:53 AM 12:41:53 AM	954.992586 1148.153621
12:42:53 AM	794.3282347
12:43:53 AM	691.8309709
12:44:53 AM	891.2509381
12:45:53 AM 12:46:53 AM	691.8309709 645.654229
12:47:53 AM	630.9573445
12:48:53 AM	691.8309709
12:49:53 AM 12:50:53 AM	691.8309709 562.3413252
12:51:53 AM	562.3413252
12:52:53 AM	575.4399373
12:53:53 AM	602.5595861
12:54:53 AM 12:55:53 AM	676.0829754 870.96359
12:56:53 AM	977.237221
12:57:53 AM	2137.96209
12:58:53 AM 12:59:53 AM	3715.352291 2089.296131
1:00:53 AM	2238.721139
1:01:53 AM	1513.561248
1:02:53 AM 1:03:53 AM	1096.478196 794.3282347
1:04:53 AM	977.237221
1:05:53 AM	870.96359
1:06:53 AM	851.1380382 1023.292992
1:07:53 AM 1:08:53 AM	954.992586
1:09:53 AM	794.3282347
1:10:53 AM	758.577575
1:11:53 AM 1:12:53 AM	741.3102413 707.9457844
1:13:53 AM	724.4359601
1:14:53 AM	776.2471166
1:15:53 AM	741.3102413
1:16:53 AM 1:17:53 AM	724.4359601 645.654229
1:18:53 AM	794.3282347
1:19:53 AM	741.3102413
1:20:53 AM 1:21:53 AM	676.0829754 630.9573445
1:22:53 AM	645.654229
1:23:53 AM	691.8309709
1:24:53 AM	676.0829754
1:25:53 AM 1:26:53 AM	812.8305162 1096.478196
1:27:53 AM	794.3282347
1:28:53 AM	724.4359601
1:29:53 AM	630.9573445 691.8309709
1:30:53 AM 1:31:53 AM	724.4359601
1:32:53 AM	812.8305162
1:33:53 AM	1148.153621
1:34:53 AM 1:35:53 AM	691.8309709 933.2543008

Study	Study	Session	OL	L _{avg}	L _{max}	L _{min}
Study	Time	Time	Status	Meter1	Meter1	Meter1
	12:36:00	12:36:00		29.3	32.4	28
	12:37:00	12:37:00		28.6	32.9	27.9
	12:38:00	12:38:00		30.6	38.1	28.1
	12:39:00	12:39:00		29.7	31.8	28.6
	12:40:00	12:40:00		29.1	30.7	27.9
	12:41:00	12:41:00		28.3	28.9	27.8
	12:42:00 12:43:00	12:42:00 12:43:00		28.3	30	27.6
	12:43:00	12:44:00		27.6	28 28	27.3 27.4
	12:44:00	12:45:00		27.6 27.6	28	27.4
	12:45:00	12:46:00		27.3	28	27.3
	12:47:00	12:47:00		27.3	27.8	27
	12:48:00	12:48:00		27.4	27.9	27.1
	12:49:00	12:49:00		29.7	37.3	27.2
	12:50:00	12:50:00		28.1	34.1	27
	12:51:00	12:51:00		27.6	28.8	27.1
	12:52:00	12:52:00		28.4	31.5	27.5
	12:53:00	12:53:00		28.1	29	27.7
	12:54:00	12:54:00		27.9	28.7	27.2
	12:55:00	12:55:00		27.6	28	27.2
	12:56:00	12:56:00		27.6	28.8	27.3
	12:57:00	12:57:00		28.1	29.1	27.5
	12:58:00	12:58:00		28	28.5	27.6
	12:59:00	12:59:00		28.3	28.8	27.7
	13:00:00	13:00:00		28.1	28.5	27.7
	13:01:00	13:01:00		28	29.5	27.5
	13:02:00	13:02:00		28	28.3	27.7
	13:03:00	13:03:00		28.3	29.7	27.7
	13:04:00	13:04:00		28.3	29.3	27.7
	13:05:00	13:05:00		28.3	28.9	27.8
	13:06:00	13:06:00		28.4	29.3	27.9
	13:07:00 13:08:00	13:07:00 13:08:00		28.2 28	29.2 28.9	27.7
	13:09:00	13:09:00		27.9	28.4	27.6 27.4
	13:10:00	13:10:00		28.3	30.4	27.4
	13:11:00	13:11:00		27.8	29.3	27.5
	13:12:00	13:12:00		28	28.7	27.7
	13:13:00	13:13:00		28.3	29.7	27.7
	13:14:00	13:14:00		28	28.6	27.6
	13:15:00	13:15:00		27.8	28.2	27.5
	13:16:00	13:16:00		28.1	29	27.6
	13:17:00	13:17:00		28.3	29.6	27.7
	13:18:00	13:18:00		27.9	28.7	27.4
	13:19:00	13:19:00		28.3	32.2	27.5
	13:20:00	13:20:00		32.2	41	27.5
	13:21:00	13:21:00		27.9	28.4	27.4
	13:22:00	13:22:00		28.6	30.3	27.8
	13:23:00	13:23:00		29.8	30.7	29.1
	13:24:00	13:24:00		29.6	31.3	28.7
	13:25:00	13:25:00		28.6	29.7	28
	13:26:00	13:26:00		28.6	29.2	28.1
	13:27:00	13:27:00		28.8	29.6	28.1 28.4
	13:28:00	13:28:00		29	30.2	
	13:29:00 13:30:00	13:29:00 13:30:00		29.8 29.4	31.9 30	29.2 28.8
	13:31:00	13:31:00		29.4	29.8	28.2
		13:32:00		29.1	29.8	28.4
	13:32:00	13:32:00		29.1		28.9
	13:34:00	13:34:00		29.3	33.1	27.9
	13:35:00	13:35:00		28.1	29.1	27.6
	13:36:00	13:36:00		28.2	30.6	27.6
	13:37:00	13:37:00		28	28.6	27.6
	13:38:00	13:38:00		27.9	28.7	27.5
	13:39:00	13:39:00		27.7	28	27.3
	13:40:00	13:40:00		28.2	29.1	27.5
	13:41:00	13:41:00		30		28.7
	13:42:00	13:42:00		29.7	36.8	28.5
	13:43:00	13:43:00		29.9	31.2	28.9
	13:44:00	13:44:00		33.9	36.8	30.5
	13:45:00	13:45:00		30.5	34	29
	13:46:00	13:46:00		29.2	30.3	28.4
	13:47:00	13:47:00		28.9		28.1
		13:48:00		28.5	29.4	28.1
	13:49:00	13:49:00		28.2	29.4	27.9
	13:50:00	13:50:00		28.5	31.9	27.8
	13:51:00	13:51:00		28.6	30.8	27.6
	13:52:00	13:52:00		28.5	30.9	27.6
	13:53:00	13:53:00		28.7	30.4	27.9
	13:54:00	13:54:00		29.9		29
		13:55:00		30.5	35.6	29.2
						28.8
						28.7
						29
	13:59:00	13:59:00		29.8	30.6	29.2
	13:54:00 13:55:00 13:56:00 13:57:00 13:58:00 13:59:00				35.6 32.5	

	Baseline SPL
1:36:53 AM	851.1380382
1:37:53 AM	724.4359601
1:38:53 AM 1:39:53 AM	1148.153621 933.2543008
1:40:53 AM	812.8305162
1:41:53 AM	676.0829754
1:42:53 AM	676.0829754
1:43:53 AM 1:44:53 AM	575.4399373 575.4399373
1:45:53 AM	575.4399373
1:46:53 AM	537.0317964
1:47:53 AM 1:48:53 AM	537.0317964 549.5408739
1:49:53 AM	933.2543008
1:50:53 AM	645.654229
1:51:53 AM 1:52:53 AM	575.4399373 691.8309709
1:52:53 AM	645.654229
1:54:53 AM	616.5950019
1:55:53 AM	575.4399373
1:56:53 AM 1:57:53 AM	575.4399373 645.654229
1:58:53 AM	630.9573445
1:59:53 AM	676.0829754
2:00:53 AM 2:01:53 AM	645.654229 630.9573445
2:01:55 AM	630.9573445
2:03:53 AM	676.0829754
2:04:53 AM	676.0829754
2:05:53 AM 2:06:53 AM	676.0829754 691.8309709
2:00:53 AM	660.693448
2:08:53 AM	630.9573445
2:09:53 AM	616.5950019
2:10:53 AM 2:11:53 AM	676.0829754 602.5595861
2:12:53 AM	630.9573445
2:13:53 AM	676.0829754
2:14:53 AM 2:15:53 AM	630.9573445 602.5595861
2:16:53 AM	645.654229
2:17:53 AM	676.0829754
2:18:53 AM	616.5950019 676.0829754
2:19:53 AM 2:20:53 AM	1659.586907
2:21:53 AM	616.5950019
2:22:53 AM	724.4359601
2:23:53 AM 2:24:53 AM	954.992586 912.0108394
2:25:53 AM	724.4359601
2:26:53 AM	724.4359601
2:27:53 AM 2:28:53 AM	758.577575 794.3282347
2:29:53 AM	954.992586
2:30:53 AM	870.96359
2:31:53 AM 2:32:53 AM	741.3102413 812.8305162
2:33:53 AM	977.237221
2:34:53 AM	851.1380382
2:35:53 AM 2:36:53 AM	645.654229 660.693448
2:37:53 AM	630.9573445
2:38:53 AM	616.5950019
2:39:53 AM	588.8436554 660.693448
2:40:53 AM 2:41:53 AM	1000
2:42:53 AM	933.2543008
2:43:53 AM	977.237221
2:44:53 AM 2:45:53 AM	2454.708916 1122.018454
2:46:53 AM	831.7637711
2:47:53 AM	776.2471166
2:48:53 AM 2:49:53 AM	707.9457844 660.693448
2:50:53 AM	707.9457844
2:51:53 AM	724.4359601
2:52:53 AM	707.9457844
2:53:53 AM 2:54:53 AM	741.3102413 977.237221
2:55:53 AM	1122.018454
2:56:53 AM	954.992586
2:57:53 AM 2:58:53 AM	1230.268771 912.0108394
2:59:53 AM	954.992586

Study	Study	Session	OL	Lavg	L _{max}	L _{min}
	Time	Time	Status	Meter1	Meter1	Meter1
	14:00:00	14:00:00		29.7	30.1	29.3
	14:01:00 14:02:00	14:01:00 14:02:00		29.6 29.7	30.4 30.1	29 29.4
	14:02:00	14:02:00		30.3	31.4	29.4
	14:04:00	14:04:00		30.6	31.6	29.9
	14:05:00	14:05:00		31	33.3	29.9
	14:06:00	14:06:00		31.1	32.3	30.2
	14:07:00	14:07:00		31.7	34	30.5
	14:08:00	14:08:00		30.7	31.4	30.2
	14:09:00	14:09:00		30.6	32.2	29.7
	14:10:00 14:11:00	14:10:00 14:11:00		29.7 29.5	31.4 30.7	29 28.9
	14:12:00	14:12:00		30.4	32.5	29.5
	14:13:00	14:13:00		29.8	31.8	29
	14:14:00	14:14:00		29.7	30.3	29
	14:15:00	14:15:00		30.5	32	29.7
	14:16:00	14:16:00		29.9	31.2	29.3
	14:17:00	14:17:00		29.3	30.4	28.5
	14:18:00	14:18:00		29	29.5	28.3
	14:19:00 14:20:00	14:19:00 14:20:00		29 28.4	30.2 29	28.3 27.9
	14:21:00	14:21:00		28.2	28.6	27.7
	14:22:00	14:22:00		28.3	29.1	27.7
	14:23:00	14:23:00		28.3	30.2	27.7
	14:24:00	14:24:00		29.1	30.8	27.9
	14:25:00	14:25:00		29.6	31.8	28.4
	14:26:00	14:26:00		29.8	32.6	28.5
	14:27:00 14:28:00	14:27:00 14:28:00		28.6 29	29.2 29.9	28.1 28.3
	14:29:00	14:29:00		28.8	29.9	28.3
	14:30:00	14:30:00		28.7	29.6	28.2
	14:31:00	14:31:00		28.8	30	28.3
	14:32:00	14:32:00		28.6	28.9	28.2
	14:33:00	14:33:00		28.7	29.7	28.2
	14:34:00	14:34:00		28.7	29.4	28.2
	14:35:00	14:35:00		28.8	29.7	28.4
	14:36:00	14:36:00		29	29.5	28.3
	14:37:00 14:38:00	14:37:00 14:38:00		28.9 48.2	30.2 65.4	28.4 29
	14:39:00	14:39:00		31.4	42.7	29.6
	14:40:00	14:40:00		30	31.3	29.3
	14:41:00	14:41:00		29.9	31.4	29.2
	14:42:00	14:42:00		30.1	31	29.3
	14:43:00	14:43:00		30.5	31.1	29.9
	14:44:00	14:44:00		31	31.8	30.3
	14:45:00 14:46:00	14:45:00 14:46:00		31.7 31.6	32.6 32.6	30.6 30.7
	14:47:00	14:47:00		30.8	31.8	30.7
	14:48:00	14:48:00		31.5	32.2	30.7
	14:49:00	14:49:00		31.1	31.7	30.5
	14:50:00	14:50:00		32	33.5	30.9
	14:51:00	14:51:00		33.1	35.8	31.8
	14:52:00	14:52:00		33.4	34.8	32.3
	14:53:00	14:53:00		33.9	38.6	31.3
	14:54:00 14:55:00	14:54:00 14:55:00		33.3 32.6	35.3 33.3	32.2 32
	14:56:00	14:56:00		32.3	33.1	31.5
	14:57:00	14:57:00		35.6	42.9	
	14:58:00	14:58:00		32.2	34.1	30.6
	14:59:00	14:59:00		31.4	32.5	30.5
	15:00:00	15:00:00		30.6	31.9	
	15:01:00	15:01:00		29.9	30.6	
	15:02:00 15:03:00	15:02:00 15:03:00		30.2 29.6	31.9 30.7	29 28.9
	15:04:00	15:04:00		30.5	30.7	
	15:05:00	15:05:00		30.4	31.1	29.7
	15:06:00	15:06:00		32.3	34.6	30.5
	15:07:00	15:07:00		31.2	33.7	30.3
	15:08:00	15:08:00		30.8	34	29.9
	15:09:00	15:09:00		30	31.3	29.5
	15:10:00	15:10:00		30	31	29.3
	15:11:00 15:12:00	15:11:00 15:12:00		29.7 29.5	30.5 32	29.1 28.8
	15:12:00 15:13:00	15:12:00 15:13:00		29.5 29.7	32 30.1	28.8 29.1
	15:13:00 15:14:00	15:13:00 15:14:00		29.7 30.4	30.1 31.2	29.1 29.6
	15:14:00	15:14:00		31.2	32.5	30.3
	15:16:00	15:16:00		32.4	33.2	31.4
	15:17:00	15:17:00		32.1	33.5	31.2
	15:18:00	15:18:00		32	33.6	
	15:19:00	15:19:00		31.3	32.3	30.5
	15:20:00	15:20:00		31	31.8	
	15:21:00	15:21:00		31.3	32.2	
	15:21:00 15:22:00 15:23:00	15:21:00 15:22:00 15:23:00		31.3 30.9 30.7	32.2 32.2 31.7	30.2 30 29.8

	Baseline SPL
3:00:53 AM	933.2543008
3:01:53 AM	912.0108394
3:02:53 AM	933.2543008
3:03:53 AM 3:04:53 AM	1071.519305 1148.153621
3:05:53 AM	1258.925412
3:06:53 AM	1288.249552
3:07:53 AM 3:08:53 AM	1479.108388
3:09:53 AM	1174.897555 1148.153621
3:10:53 AM	933.2543008
3:11:53 AM	891.2509381
3:12:53 AM 3:13:53 AM	1096.478196 954.992586
3:14:53 AM	933.2543008
3:15:53 AM	1122.018454
3:16:53 AM	977.237221
3:17:53 AM 3:18:53 AM	851.1380382 794.3282347
3:19:53 AM	794.3282347
3:20:53 AM	691.8309709
3:21:53 AM	660.693448
3:22:53 AM 3:23:53 AM	676.0829754 676.0829754
3:24:53 AM	812.8305162
3:25:53 AM	912.0108394
3:26:53 AM	954.992586 724.4359601
3:27:53 AM 3:28:53 AM	794.3282347
3:29:53 AM	758.577575
3:30:53 AM	741.3102413
3:31:53 AM	758.577575
3:32:53 AM 3:33:53 AM	724.4359601 741.3102413
3:34:53 AM	741.3102413
3:35:53 AM	758.577575
3:36:53 AM 3:37:53 AM	794.3282347 776.2471166
3:38:53 AM	66069.3448
3:39:53 AM	1380.384265
3:40:53 AM 3:41:53 AM	1000 977.237221
3:42:53 AM	1023.292992
3:43:53 AM	1122.018454
3:44:53 AM 3:45:53 AM	1258.925412 1479.108388
3:46:53 AM	1445.439771
3:47:53 AM	1202.264435
3:48:53 AM 3:49:53 AM	1412.537545 1288.249552
3:50:53 AM	1584.893192
3:51:53 AM	2041.737945
3:52:53 AM 3:53:53 AM	2187.761624 2454.708916
3:54:53 AM	2137.96209
3:55:53 AM	1819.700859
3:56:53 AM 3:57:53 AM	1698.243652 3630.780548
3:58:53 AM	1659.586907
3:59:53 AM	1380.384265
4:00:53 AM	1148.153621 977.237221
4:01:53 AM 4:02:53 AM	1047.128548
4:03:53 AM	912.0108394
4:04:53 AM	1122.018454
4:05:53 AM 4:06:53 AM	1096.478196 1698.243652
4:07:53 AM	1318.256739
4:08:53 AM	1202.264435
4:09:53 AM 4:10:53 AM	1000 1000
4:10:53 AM 4:11:53 AM	933.2543008
4:12:53 AM	891.2509381
4:13:53 AM	933.2543008
4:14:53 AM 4:15:53 AM	1096.478196 1318.256739
4:16:53 AM	1737.800829
4:17:53 AM	1621.810097
4:18:53 AM 4:19:53 AM	1584.893192 1348.962883
4:19:53 AM 4:20:53 AM	1258.925412
4:21:53 AM	1348.962883
4:22:53 AM 4:23:53 AM	1230.268771 1174.897555
	1105,555

Study	Study	Session	OL	L _{avg}	L _{max}	L _{min}
Study	Time	Time	Status	Meter1	Meter1	Meter1
	15:24:00	15:24:00		30.5	31.5	29.8
	15:25:00	15:25:00		30.8	32.4	29.8
	15:26:00	15:26:00		30.6	32.2	29.8
	15:27:00	15:27:00		30.8	31.9	30
	15:28:00	15:28:00		30.7	31.5	30
	15:29:00	15:29:00		31.3	32.5	30.6
	15:30:00	15:30:00		31.3	32.1	30.8
	15:31:00	15:31:00		31.5 31.7	32.4	30.8
	15:32:00 15:33:00	15:32:00 15:33:00		31.7	32.7 32.3	30.9 30.5
	15:34:00	15:34:00		31.1	32.3	30.3
	15:35:00	15:35:00		32.4	33.2	31.6
	15:36:00	15:36:00		32.7	33.8	31.9
	15:37:00	15:37:00		33	34.5	32.2
	15:38:00	15:38:00		33.1	34.2	32.4
	15:39:00	15:39:00		31.8	33.2	31.1
	15:40:00	15:40:00		32.9	35.9	31.7
	15:41:00	15:41:00		32.9	34.5	31.7
	15:42:00	15:42:00		32	33.2	30.8
	15:43:00	15:43:00		31.6	32.8	30.6
	15:44:00	15:44:00		32.7	35.5	31.1
	15:45:00	15:45:00		31.7	33.4	30.7
	15:46:00	15:46:00		31.9	32.7	31.2
	15:47:00	15:47:00		32.4	33.3	31.4
	15:48:00	15:48:00		32.9	34.4	31.9
	15:49:00	15:49:00		32.4	34.2	31.6
	15:50:00	15:50:00		33.9	36.4	32.6
	15:51:00	15:51:00		32.4	33.1	31.7
	15:52:00 15:53:00	15:52:00 15:53:00		31.8	32.5	31.1 31.3
				32.8 33	34.4 34.1	
	15:54:00 15:55:00	15:54:00 15:55:00		35.7	39.6	32.2 32.4
	15:56:00	15:56:00		33.7	35.2	33
	15:57:00	15:57:00		33.2	34.1	31.8
	15:58:00	15:58:00		32.5	34.3	31.3
	15:59:00	15:59:00		32.3	33.5	31.3
	16:00:00	16:00:00		33.2	34.5	32
	16:01:00	16:01:00		32.8	34.3	31.6
	16:02:00	16:02:00		33.3	34.7	32.1
	16:03:00	16:03:00		32.6	33.9	31.5
	16:04:00	16:04:00		32.4	33.3	31.7
	16:05:00	16:05:00		32.5	33.5	31.5
	16:06:00	16:06:00		32.6	34.2	31.2
	16:07:00	16:07:00		33.2	34.5	32.1
	16:08:00	16:08:00		33.6	36.3	32.7
	16:09:00	16:09:00		33.3	35.1	31.9
	16:10:00	16:10:00		32.5	33.2	31.6
	16:11:00	16:11:00		32.5	34.7	31.5
	16:12:00	16:12:00		32.7	36.8	31.4
	16:13:00	16:13:00		32.8	39.1	30.8
	16:14:00	16:14:00		32.3 32.5	33.4	31.3
	16:15:00 16:16:00	16:15:00 16:16:00		33.1	33.4 35.9	31.6 31.9
	16:17:00	16:17:00		32.5	35.9	31.3
	16:18:00	16:18:00		32.2		31.3
	16:19:00	16:19:00		33.4		32.1
	16:20:00	16:20:00		32.6		31.7
	16:21:00	16:21:00		34.8		33
	16:22:00	16:22:00		33.8		32.3
	16:23:00	16:23:00		33.9		32.4
	16:24:00	16:24:00		36.3		34.2
	16:25:00	16:25:00		37.4	40.5	35.1
	16:26:00	16:26:00		34.5	37.4	32.8
	16:27:00	16:27:00		34.8	36.2	33.3
	16:28:00	16:28:00		45	47.3	34.9
	16:29:00	16:29:00		44.6	45.8	43.6
	16:30:00	16:30:00		44.4	45.6	43.3
	16:31:00	16:31:00		44.7	45.8	43.5
	16:32:00	16:32:00		45.6		44.5
	16:33:00	16:33:00		46.5	50.6	44.5
	16:34:00	16:34:00		46.5	48.4	44.7
	16:35:00	16:35:00		46.2		44.5
	16:36:00	16:36:00		45.4		44.2
	16:37:00	16:37:00		44.6		43.7
	16:38:00	16:38:00		44.2		43.5
	16:39:00	16:39:00		44		42.6
	16:40:00	16:40:00		44.6		43.5
	16:41:00	16:41:00		44.3	46.4	43.3
	16:42:00	16:42:00		44.8		44
	16:43:00	16:43:00		44.3		43.2
	16:44:00	16:44:00		45.3		43.3
	16:45:00 16:46:00	16:45:00		47 46 6	50.9 48.2	44.7 45.1
	16:45:00	16:46:00 16:47:00		46.6 46.6	48.2 50	45.1
	10.47.00	10.47.00		40.0	30	43

	Baseline SPL
4:24:53 AM	1122.018454
4:25:53 AM	1202.264435
4:26:53 AM 4:27:53 AM	1148.153621 1202.264435
4:28:53 AM	1174.897555
4:29:53 AM	1348.962883
4:30:53 AM 4:31:53 AM	1348.962883 1412.537545
4:32:53 AM	1479.108388
4:33:53 AM 4:34:53 AM	1288.249552 1318.256739
4:35:53 AM	1737.800829
4:36:53 AM	1862.087137
4:37:53 AM 4:38:53 AM	1995.262315 2041.737945
4:39:53 AM	1513.561248
4:40:53 AM 4:41:53 AM	1949.8446 1949.8446
4:42:53 AM	1584.893192
4:43:53 AM	1445.439771
4:44:53 AM 4:45:53 AM	1862.087137 1479.108388
4:46:53 AM	1548.816619
4:47:53 AM	1737.800829
4:48:53 AM 4:49:53 AM	1949.8446 1737.800829
4:50:53 AM	2454.708916
4:51:53 AM 4:52:53 AM	1737.800829 1513.561248
4:52:53 AM 4:53:53 AM	1905.460718
4:54:53 AM	1995.262315
4:55:53 AM 4:56:53 AM	3715.352291 2344.228815
4:57:53 AM	2089.296131
4:58:53 AM	1778.27941
4:59:53 AM 5:00:53 AM	1698.243652 2089.296131
5:01:53 AM	1905.460718
5:02:53 AM 5:03:53 AM	2137.96209 1819.700859
5:04:53 AM	1737.800829
5:05:53 AM	1778.27941
5:06:53 AM 5:07:53 AM	1819.700859 2089.296131
5:08:53 AM	2290.867653
5:09:53 AM 5:10:53 AM	2137.96209 1778.27941
5:10:53 AM	1778.27941
5:12:53 AM	1862.087137
5:13:53 AM 5:14:53 AM	1905.460718 1698.243652
5:15:53 AM	1778.27941
5:16:53 AM 5:17:53 AM	2041.737945 1778.27941
5:17:53 AM	1659.586907
5:19:53 AM	2187.761624
5:20:53 AM 5:21:53 AM	1819.700859 3019.95172
5:22:53 AM	2398.832919
5:23:53 AM	2454.708916 4265.795188
5:24:53 AM 5:25:53 AM	5495.408739
5:26:53 AM	2818.382931
5:27:53 AM 5:28:53 AM	3019.95172 31622.7766
5:29:53 AM	28840.31503
5:30:53 AM	27542.28703 29512.09227
5:31:53 AM 5:32:53 AM	36307.80548
5:33:53 AM	44668.35922
5:34:53 AM 5:35:53 AM	44668.35922 41686.93835
5:36:53 AM	34673.68505
5:37:53 AM	28840.31503
5:38:53 AM 5:39:53 AM	26302.67992 25118.86432
5:40:53 AM	28840.31503
5:41:53 AM 5:42:53 AM	26915.34804 30199.5172
5:42:53 AM 5:43:53 AM	26915.34804
5:44:53 AM	33884.41561
5:45:53 AM 5:46:53 AM	50118.72336 45708.81896
5:47:53 AM	45708.81896

Study	Study	Session	OL	Lavg	L _{max}	L _{min}
Study	Time	Time	Status	Meter1	Meter1	Meter1
	16:48:00	16:48:00		45.6	47.4	44.7
	16:49:00 16:50:00	16:49:00 16:50:00		45 45.1	46.6 46.6	43.6 43
	16:51:00	16:51:00		46.6	51.3	44.5
	16:52:00	16:52:00		46.5	49.4	44.7
	16:53:00	16:53:00		46.7	51.6	44.6
	16:54:00	16:54:00		45.7	47.2	44.8
	16:55:00 16:56:00	16:55:00 16:56:00		44.7 44.3	45.7 45.1	43.9 43.5
	16:57:00	16:57:00		44.4	45.4	43.6
	16:58:00	16:58:00		44.7	46.6	43.9
	16:59:00	16:59:00		45	48	43.5
	17:00:00 17:01:00	17:00:00 17:01:00		45.1 45.6	47.1 48	43.9 44.3
	17:02:00	17:02:00		47.2	51	44.7
	17:03:00	17:03:00		48.6	53.2	45.1
	17:04:00	17:04:00		48.7	53.8	45.1
	17:05:00	17:05:00		49.6	54.1	46.5
	17:06:00 17:07:00	17:06:00 17:07:00		51.2 52.7	54.8 57.1	48 49.5
	17:08:00	17:08:00		52.8	56.9	47.5
	17:09:00	17:09:00		52.6	60.1	48.5
	17:10:00	17:10:00		50.1	55.2	47.6
	17:11:00 17:12:00	17:11:00 17:12:00		54.4 54.4	63.1 59.5	48.7 48.9
	17:12:00	17:12:00		51.6	55.9	48.3
	17:14:00	17:14:00		54.6	59.9	49
	17:15:00	17:15:00		55.1	59.1	50.5
	17:16:00	17:16:00		53.2	58.9	48.8
	17:17:00 17:18:00	17:17:00 17:18:00		51.6 47.9	55.5 52.1	48.2 45.6
	17:19:00	17:19:00		46.8	50.8	45.3
	17:20:00	17:20:00		46.8	49.5	45.5
	17:21:00	17:21:00		45.8	47.4	44.4
	17:22:00 17:23:00	17:22:00 17:23:00		46.5 45.9	51.1 46.9	44.8 44.9
	17:24:00	17:24:00		46.4	50.9	44.6
	17:25:00	17:25:00		46.2	48.4	44.7
	17:26:00	17:26:00		45.6	46.9	44.8
	17:27:00 17:28:00	17:27:00 17:28:00		45.1 44.7	45.8 45.4	44.4 44
	17:29:00	17:29:00		44.7	47.3	43.5
	17:30:00	17:30:00		45.1	47.3	43.8
	17:31:00	17:31:00		44.7	46.1	43.6
	17:32:00 17:33:00	17:32:00 17:33:00		45.1 44.5	45.9 45.5	44.3 43.3
	17:33:00	17:34:00		44.5	45.5 44.9	43.9
	17:35:00	17:35:00		44.4	44.8	44
	17:36:00	17:36:00		44.4	44.9	43.6
	17:37:00	17:37:00		44.3	44.9	43.6
	17:38:00 17:39:00	17:38:00 17:39:00		44.2 44.2	45 44.6	43.5 43.8
	17:40:00	17:40:00		44.1	44.6	43.7
	17:41:00	17:41:00		44.4	49	43.7
	17:42:00	17:42:00		44.2	45.8	43.7
	17:43:00 17:44:00	17:43:00 17:44:00		44.4 44.3	45.3 44.9	43.6 43.7
	17:44:00	17:45:00		44.3	44.8	43.7
	17:46:00	17:46:00		44.1	44.5	43.8
	17:47:00	17:47:00		44.1	44.6	43.7
	17:48:00 17:49:00	17:48:00 17:49:00		44.3 44.1	44.7 44.7	43.8 43.7
	17:50:00	17:50:00		44.1	44.7	43.4
	17:51:00	17:51:00		44.7	46.1	43.7
	17:52:00	17:52:00		46.3	48.2	44.5
	17:53:00	17:53:00		44.5	46.1	43.7
	17:54:00 17:55:00	17:54:00 17:55:00		44 44.2	44.6 45.3	43.1 43.4
	17:56:00	17:56:00		43.9	44.6	43.4
	17:57:00	17:57:00		43.9	44.4	43.2
	17:58:00	17:58:00		44.2	44.8	43.3
	17:59:00	17:59:00		44.3	44.9	43.9
	18:00:00 18:01:00	18:00:00 18:01:00		44.3 44.1	44.6 45.3	43.9 43.6
	18:02:00	18:02:00		44.1	45.1	43.6
	18:03:00	18:03:00		44.6	47.3	43.8
	18:04:00	18:04:00		44.1	45	43.4
	18:05:00	18:05:00		44.1	44.6	43.6
	18:06:00 18:07:00	18:06:00 18:07:00		44.2 44.4	45.4 45.9	43.3 43.6
	18:08:00	18:08:00		44.4	45.9	43.0
	18:09:00	18:09:00		44.5	45.7	44
	18:10:00	18:10:00		44.4	44.9	43.8
	18:11:00	18:11:00		44.4	44.8	43.9

	Baseline SPL
5:48:53 AM	36307.80548
5:49:53 AM	31622.7766
5:50:53 AM 5:51:53 AM	32359.36569 45708.81896
5:52:53 AM	44668.35922
5:53:53 AM	46773.51413
5:54:53 AM 5:55:53 AM	37153.52291 29512.09227
5:56:53 AM	26915.34804
5:57:53 AM	27542.28703
5:58:53 AM 5:59:53 AM	29512.09227 31622.7766
6:00:53 AM	32359.36569
6:01:53 AM	36307.80548
6:02:53 AM 6:03:53 AM	52480.74602 72443.59601
6:04:53 AM	74131.02413
6:05:53 AM	91201.08394
6:06:53 AM 6:07:53 AM	131825.6739 186208.7137
6:08:53 AM	190546.0718
6:09:53 AM	181970.0859
6:10:53 AM 6:11:53 AM	102329.2992 275422.8703
6:12:53 AM	275422.8703
6:13:53 AM	144543.9771
6:14:53 AM 6:15:53 AM	288403.1503 323593.6569
6:16:53 AM	208929.6131
6:17:53 AM	144543.9771
6:18:53 AM 6:19:53 AM	61659.50019 47863.00923
6:20:53 AM	47863.00923
6:21:53 AM	38018.93963
6:22:53 AM 6:23:53 AM	44668.35922 38904.5145
6:24:53 AM	43651.58322
6:25:53 AM	41686.93835
6:26:53 AM 6:27:53 AM	36307.80548 32359.36569
6:28:53 AM	29512.09227
6:29:53 AM	30199.5172
6:30:53 AM 6:31:53 AM	32359.36569 29512.09227
6:32:53 AM	32359.36569
6:33:53 AM 6:34:53 AM	28183.82931 27542.28703
6:35:53 AM	27542.28703
6:36:53 AM	27542.28703
6:37:53 AM 6:38:53 AM	26915.34804 26302.67992
6:39:53 AM	26302.67992
6:40:53 AM	25703.95783
6:41:53 AM 6:42:53 AM	27542.28703 26302.67992
6:43:53 AM	27542.28703
6:44:53 AM	26915.34804
6:45:53 AM 6:46:53 AM	26302.67992 25703.95783
6:47:53 AM	25703.95783
6:48:53 AM	26915.34804
6:49:53 AM 6:50:53 AM	25703.95783 25118.86432
6:51:53 AM	29512.09227
6:52:53 AM 6:53:53 AM	42657.95188 28183.82931
6:54:53 AM	25118.86432
6:55:53 AM	26302.67992
6:56:53 AM 6:57:53 AM	24547.08916 24547.08916
6:58:53 AM	26302.67992
6:59:53 AM	26915.34804
7:00:53 AM 7:01:53 AM	26915.34804 25703.95783
7:02:53 AM	26302.67992
7:03:53 AM	28840.31503
7:04:53 AM 7:05:53 AM	25703.95783 25703.95783
7:05:53 AM	26302.67992
7:07:53 AM	27542.28703
7:08:53 AM 7:09:53 AM	27542.28703 28183.82931
7:10:53 AM	27542.28703
7:11:53 AM	27542.28703

tudy	Study	Session	OL	Lavg	L _{max}	L _{min}
ituay	Time	Time	Status	Meter1	Meter1	Meter1
	18:12:00	18:12:00		44.3	44.8	43.8
	18:13:00	18:13:00		44.7	45.7	44.1
	18:14:00	18:14:00		45.4	49.8	43.6
	18:15:00	18:15:00		48.6	61.6	44.3
	18:16:00 18:17:00	18:16:00 18:17:00		44.7 44.9	45.7	43.9 43.8
	18:18:00	18:17:00		45.3	46.5 47.4	44.3
	18:19:00	18:19:00		44.7	45.9	43.8
	18:20:00	18:20:00		44.7	45.7	43.5
	18:21:00	18:21:00		45.1	47.3	44.1
	18:22:00	18:22:00		46	51	44.5
	18:23:00	18:23:00		44.5	45.2	44
	18:24:00	18:24:00		44.9	46.3	43.5
	18:25:00	18:25:00		44.9	46.1	43.9
	18:26:00	18:26:00		44.7	46	43.4
	18:27:00	18:27:00		44.8	45.7	44.3
	18:28:00	18:28:00		45	47.6	43.9
	18:29:00	18:29:00		45.1	46.8	44.3
	18:30:00	18:30:00		45.7	53.9	43.6
	18:31:00	18:31:00		45.1	48.1	43.9
	18:32:00	18:32:00		45.2	47.9	43.8
	18:33:00	18:33:00		44.7	45.6	43.9
	18:34:00	18:34:00		45.4	50	44.3
	18:35:00	18:35:00		45.3	49.3	43.8
	18:36:00	18:36:00		44.2	44.8	43.5
	18:37:00	18:37:00		44.5	45.1	43.9
	18:38:00	18:38:00		44.7	45.8	44
	18:39:00	18:39:00		44.8	46.3	43.6
	18:40:00	18:40:00		44.5	44.9	43.
	18:41:00	18:41:00		44.7	47.3	43.7
	18:42:00	18:42:00		44.9	46.8	43.9
	18:43:00	18:43:00		45	46.9	44
	18:44:00	18:44:00		44.3	44.8	43.
	18:45:00	18:45:00		44.6	45.3	43.
	18:46:00 18:47:00	18:46:00 18:47:00		45.2	46.9 48.7	44.3
	18:48:00	18:48:00		45.1 48.7	53.5	45.4
	18:49:00	18:49:00		47.4	53.6	44.4
	18:50:00	18:50:00		46.9	50.3	44
	18:51:00	18:51:00		45	47.7	43.9
	18:52:00	18:52:00		47.9	52	45
	18:53:00	18:53:00		47.5	50.8	44.8
	18:54:00	18:54:00		50.2	59.6	45.
	18:55:00	18:55:00		47.9	52.2	45
	18:56:00	18:56:00		50.6	56.6	44.9
	18:57:00	18:57:00		47.9	53.5	45.4
	18:58:00	18:58:00		45.3	48.2	44.:
	18:59:00	18:59:00		45.3	48	44.
	19:00:00	19:00:00		44.9	46.5	4
	19:01:00	19:01:00		44.7	45.3	44.
	19:02:00	19:02:00		45.2	46.2	44.
	19:03:00	19:03:00		45.7	46.6	44.
	19:04:00	19:04:00		44.9	46	44.
	19:05:00	19:05:00		44.6	45.1	44.
	19:06:00	19:06:00		44.4	44.8	4
	19:07:00	19:07:00		44.7	45.2	4
	19:08:00	19:08:00		44.7	45.4	44
	19:09:00	19:09:00		44.7	45.5	44.4
	19:10:00	19:10:00 19:11:00		44.7	45	44.
	19:11:00			44.8 44.8	45.1	44.
	19:12:00 19:13:00	19:12:00 19:13:00		44.8	45.2 45.5	44.! 44.!
	19:14:00	19:14:00		45.2	50.2	44.
	19:15:00	19:15:00		44.6	45.8	43.
	19:16:00	19:16:00		44.6	45.2	4
	19:17:00	19:17:00		44.5	44.8	44.
	19:18:00	19:18:00		44.6	44.9	44.:
	19:19:00	19:19:00		44.6	44.9	44.
	19:20:00	19:20:00		44.5	44.9	44.
	19:21:00	19:21:00		44.6	45.1	44.:
	19:22:00	19:22:00		44.6	45.9	4
	19:23:00	19:23:00		44.6	45	44.
	19:24:00	19:24:00		44.9	45.4	44.
	19:25:00	19:25:00		44.9	45.8	44.
	19:26:00	19:26:00		45	45.4	44.
	19:27:00	19:27:00		44.9	45.2	44.
	19:28:00	19:28:00		44.8	45.2	44.
	19:29:00	19:29:00		44.8	45.2	44.
	19:30:00	19:30:00		44.7	45	44.
	19:31:00	19:31:00		44.8	45.2	44.
	19:32:00	19:32:00		44.9	45.9	44.
	19:33:00	19:33:00		44.7	45	44.
	19:34:00	19:34:00		44.8	45.1	44.

	Baseline SPL
7:12:53 AM	26915.34804
7:13:53 AM	29512.09227
7:14:53 AM	34673.68505
7:15:53 AM 7:16:53 AM	72443.59601 29512.09227
7:17:53 AM	30902.95433
7:18:53 AM	33884.41561
7:19:53 AM	29512.09227
7:20:53 AM	28183.82931
7:21:53 AM 7:22:53 AM	32359.36569 39810.71706
7:23:53 AM	28183.82931
7:24:53 AM	30902.95433
7:25:53 AM	30902.95433
7:26:53 AM	29512.09227
7:27:53 AM 7:28:53 AM	30199.5172 31622.7766
7:29:53 AM	32359.36569
7:30:53 AM	37153.52291
7:31:53 AM	32359.36569
7:32:53 AM	33113.11215
7:33:53 AM 7:34:53 AM	29512.09227 34673.68505
7:35:53 AM	33884.41561
7:36:53 AM	26302.67992
7:37:53 AM	28183.82931
7:38:53 AM	29512.09227
7:39:53 AM 7:40:53 AM	30199.5172 28183.82931
7:41:53 AM	29512.09227
7:42:53 AM	30902.95433
7:43:53 AM	31622.7766
7:44:53 AM 7:45:53 AM	26915.34804 28840.31503
7:46:53 AM	33113.11215
7:47:53 AM	32359.36569
7:48:53 AM	74131.02413
7:49:53 AM	54954.08739
7:50:53 AM 7:51:53 AM	48977.88194 31622.7766
7:52:53 AM	61659.50019
7:53:53 AM	56234.13252
7:54:53 AM	104712.8548
7:55:53 AM 7:56:53 AM	61659.50019 114815.3621
7:55:53 AM 7:57:53 AM	61659.50019
7:58:53 AM	33884.41561
7:59:53 AM	33884.41561
8:00:53 AM	30902.95433
8:01:53 AM 8:02:53 AM	29512.09227 33113.11215
8:03:53 AM	37153.52291
8:04:53 AM	30902.95433
8:05:53 AM	28840.31503
8:06:53 AM 8:07:53 AM	27542.28703 29512.09227
8:08:53 AM	29512.09227
8:09:53 AM	29512.09227
8:10:53 AM	29512.09227
8:11:53 AM 8:12:53 AM	30199.5172 30199.5172
8:13:53 AM	31622.7766
8:14:53 AM	33113.11215
8:15:53 AM	28840.31503
8:16:53 AM	28840.31503
8:17:53 AM 8:18:53 AM	28183.82931 28840.31503
8:19:53 AM	28840.31503
8:20:53 AM	28183.82931
8:21:53 AM	28840.31503
8:22:53 AM	28840.31503
8:23:53 AM 8:24:53 AM	28840.31503 30902.95433
8:25:53 AM	30902.95433
8:26:53 AM	31622.7766
8:27:53 AM	30902.95433
8:28:53 AM	30199.5172
8:29:53 AM 8:30:53 AM	30199.5172 29512.09227
8:31:53 AM	30199.5172
8:32:53 AM	30902.95433
8:33:53 AM	29512.09227
8:34:53 AM 8:35:53 AM	30199.5172 30199.5172
_100.00 AIVI	55155.5112

Study	Study	Session	OL	L _{avg}	L _{max}	L _{min}
Study	Time	Time	Status	Meter1	Meter1	Meter1
	19:36:00	19:36:00		44.7	45	44.4
	19:37:00	19:37:00		44.8	45.3	44.1
	19:38:00	19:38:00		44.8	45.3	44.2
	19:39:00	19:39:00		44.8	45.2	44.5
	19:40:00	19:40:00		44.8	45.3	44.4
	19:41:00	19:41:00		44.6	45	44.3
	19:42:00	19:42:00		44.8	45	44.5
	19:43:00	19:43:00		45	45.6	44.6 44.7
	19:44:00 19:45:00	19:44:00 19:45:00		45 45	45.4 45.6	44.7
	19:46:00	19:46:00		45.3	45.7	44.7
	19:47:00	19:47:00		45.2	45.8	44.7
	19:48:00	19:48:00		45.1	45.6	44.6
	19:49:00	19:49:00		45	45.4	44.5
	19:50:00	19:50:00		45.2	45.6	44.7
	19:51:00	19:51:00		45.4	45.7	45
	19:52:00	19:52:00		45.2	45.6	44.8
	19:53:00	19:53:00		45.1	45.6	44.6
	19:54:00	19:54:00		45.2	45.9	44.5
	19:55:00	19:55:00		45.2	45.8	44.6
	19:56:00	19:56:00		45.3	46.3	44.7
	19:57:00	19:57:00		45.4	46.6	44.3
	19:58:00	19:58:00		45.2	45.7	44.6
	19:59:00	19:59:00		45.4	47	44.8
	20:00:00	20:00:00		46.1	48.6	44.8
	20:01:00	20:01:00		49.9	53.7	45.4
	20:02:00	20:02:00		46	47.6	45
	20:03:00	20:03:00		45.5	46.8	44.6
	20:04:00	20:04:00		45.6	46.7	44.8
	20:05:00	20:05:00		45.4	46.5	44.4
	20:06:00	20:06:00		46	48.5	44.6
	20:07:00	20:07:00		45	45.5	44.3
	20:08:00	20:08:00		44.9	45.4	44.5
	20:09:00	20:09:00		45.1	46.5	44.3
	20:10:00	20:10:00		45	45.6	44
	20:11:00	20:11:00		45	45.9	44.4
	20:12:00	20:12:00		44.9	46.2	44.3
	20:13:00 20:14:00	20:13:00 20:14:00		44.8 44.6	45.4 45	44.3 44.1
	20:15:00	20:15:00		44.7	45.4	43.9
	20:16:00	20:16:00		45.3	45.9	44.7
	20:17:00	20:17:00		45.7	46.3	45.2
	20:18:00	20:18:00		46.3	47.5	45.5
	20:19:00	20:19:00		46.3	47.1	45.1
	20:20:00	20:20:00		46	48.4	44.9
	20:21:00	20:21:00		45.6	46.6	44.6
	20:22:00	20:22:00		46.7	50.3	45
	20:23:00	20:23:00		49.6	54.4	45.3
	20:24:00	20:24:00		46.6	47.9	45.3
	20:25:00	20:25:00		45	47.2	44.2
	20:26:00	20:26:00		45.1	46.3	44.3
	20:27:00	20:27:00		45.2	46	44.2
	20:28:00	20:28:00		45.8	47.3	44.9
	20:29:00	20:29:00		46.8	49.7	
	20:30:00	20:30:00		46.6	48.8	
	20:31:00	20:31:00		46.4	49	
	20:32:00	20:32:00		46.3	47.5	
	20:33:00	20:33:00		45.7	47.8	
	20:34:00	20:34:00		45	45.9	
	20:35:00	20:35:00		45.4	46.9	
	20:36:00	20:36:00		45.1	46.9	
	20:37:00 20:38:00	20:37:00		45.2 45.1	46 45.8	
	20:38:00	20:38:00 20:39:00		45.1 45.3	45.8 46.4	
	20:40:00	20:39:00		45.8	40.4	
	20:41:00	20:41:00		45.7	47.6	
	20:42:00	20:42:00		45.7	46.6	
	20:43:00	20:43:00		46	49.2	
	20:44:00	20:44:00		50.1	57.1	
	20:45:00	20:45:00		48.3	52.7	45.3
	20:46:00	20:46:00		46.1	48.6	
	20:47:00	20:47:00		45.8	47.1	
	20:48:00	20:48:00		45.4	46.3	
		20:49:00		45.2	46	
	20:49:00	20.45.00				
	20:49:00 20:50:00	20:50:00		45.2	46.6	44.4
				45.2 45.1	46.6 47.3	44.4
	20:50:00	20:50:00				44.3
	20:50:00 20:51:00	20:50:00 20:51:00		45.1	47.3	44.3 44.5
	20:50:00 20:51:00 20:52:00	20:50:00 20:51:00 20:52:00		45.1 45.2	47.3 47.1	44.3 44.5 44.3
	20:50:00 20:51:00 20:52:00 20:53:00	20:50:00 20:51:00 20:52:00 20:53:00		45.1 45.2 44.9	47.3 47.1 45.9 47.4	44.3 44.5 44.3 44.7
	20:50:00 20:51:00 20:52:00 20:53:00 20:54:00	20:50:00 20:51:00 20:52:00 20:53:00 20:54:00		45.1 45.2 44.9 45.9	47.3 47.1 45.9 47.4	44.3 44.5 44.3 44.7 44.3
	20:50:00 20:51:00 20:52:00 20:53:00 20:54:00 20:55:00	20:50:00 20:51:00 20:52:00 20:53:00 20:54:00 20:55:00		45.1 45.2 44.9 45.9 44.8	47.3 47.1 45.9 47.4 45.2	44.3 44.5 44.3 44.7 44.3 44.5
	20:50:00 20:51:00 20:52:00 20:53:00 20:54:00 20:55:00 20:56:00	20:50:00 20:51:00 20:52:00 20:53:00 20:54:00 20:55:00 20:56:00		45.1 45.2 44.9 45.9 44.8 45.1	47.3 47.1 45.9 47.4 45.2 46.3	44.3 44.5 44.7 44.3 44.5 44.1

	Baseline SPL
8:36:53 AM	29512.09227
8:37:53 AM	30199.5172
8:38:53 AM 8:39:53 AM	30199.5172 30199.5172
8:40:53 AM	30199.5172
8:41:53 AM	28840.31503
8:42:53 AM 8:43:53 AM	30199.5172
8:43:53 AM 8:44:53 AM	31622.7766 31622.7766
8:45:53 AM	31622.7766
8:46:53 AM	33884.41561 33113.11215
8:47:53 AM 8:48:53 AM	32359.36569
8:49:53 AM	31622.7766
8:50:53 AM 8:51:53 AM	33113.11215 34673.68505
8:52:53 AM	33113.11215
8:53:53 AM	32359.36569
8:54:53 AM 8:55:53 AM	33113.11215 33113.11215
8:55:53 AM 8:56:53 AM	33884.41561
8:57:53 AM	34673.68505
8:58:53 AM	33113.11215
8:59:53 AM 9:00:53 AM	34673.68505 40738.02778
9:01:53 AM	97723.7221
9:02:53 AM	39810.71706
9:03:53 AM 9:04:53 AM	35481.33892 36307.80548
9:05:53 AM	34673.68505
9:06:53 AM	39810.71706
9:07:53 AM 9:08:53 AM	31622.7766 30902.95433
9:09:53 AM	32359.36569
9:10:53 AM 9:11:53 AM	31622.7766 31622.7766
9:12:53 AM	30902.95433
9:13:53 AM	30199.5172
9:14:53 AM 9:15:53 AM	28840.31503 29512.09227
9:16:53 AM	33884.41561
9:17:53 AM	37153.52291
9:18:53 AM 9:19:53 AM	42657.95188 42657.95188
9:20:53 AM	39810.71706
9:21:53 AM	36307.80548
9:22:53 AM 9:23:53 AM	46773.51413 91201.08394
9:24:53 AM	45708.81896
9:25:53 AM 9:26:53 AM	31622.7766 32359.36569
9:27:53 AM	33113.11215
9:28:53 AM	38018.93963
9:29:53 AM 9:30:53 AM	47863.00923 45708.81896
9:31:53 AM	43651.58322
9:32:53 AM	42657.95188
9:33:53 AM 9:34:53 AM	37153.52291 31622.7766
9:35:53 AM	34673.68505
9:36:53 AM	32359.36569
9:37:53 AM 9:38:53 AM	33113.11215 32359.36569
9:39:53 AM	33884.41561
9:40:53 AM 9:41:53 AM	38018.93963 37153.52291
9:42:53 AM	37153.52291
9:43:53 AM	39810.71706
9:44:53 AM 9:45:53 AM	102329.2992 67608.29754
9:46:53 AM	40738.02778
9:47:53 AM	38018.93963
9:48:53 AM 9:49:53 AM	34673.68505 33113.11215
9:50:53 AM	33113.11215
9:51:53 AM	32359.36569
9:52:53 AM 9:53:53 AM	33113.11215 30902.95433
9:54:53 AM	38904.5145
9:55:53 AM	30199.5172
9:56:53 AM 9:57:53 AM	32359.36569 31622.7766
9:58:53 AM	34673.68505
9:59:53 AM	30199.5172

Study	Study	Session	OL	L _{avg}	L _{max}	L _{min}
Study	Time	Time	Status	Meter1	Meter1	Meter1
	21:00:00	21:00:00		44.7	45.1	43.9
	21:01:00	21:01:00		44.6	45.4	44.1
	21:02:00	21:02:00		44.7	45.1	44.1
	21:03:00	21:03:00		44.7	45.2	44.1
	21:04:00	21:04:00		44.7	45.2	44.3
	21:05:00	21:05:00		44.7	45.5	43.9
	21:06:00	21:06:00		44.6	45.5	43.7
	21:07:00	21:07:00		44.6 44.7	45.4	43.9
	21:08:00 21:09:00	21:08:00 21:09:00		44.7	45.3 45.5	44 44
	21:10:00	21:10:00		44.7	45.4	44.1
	21:11:00	21:11:00		44.9	45.4	44.5
	21:12:00	21:12:00		45.4	48.5	44.5
	21:13:00	21:13:00		45.2	47.1	44.5
	21:14:00	21:14:00		44.8	45.2	44.4
	21:15:00	21:15:00		44.5	44.9	44.1
	21:16:00	21:16:00		44.6	46.2	44
	21:17:00	21:17:00		45.1	47.3	44.1
	21:18:00	21:18:00		48.5	53.7	44.8
	21:19:00	21:19:00		45	46.2	44
	21:20:00	21:20:00		45	46.1	44.3
	21:21:00	21:21:00		45	45.6	44.5
	21:22:00	21:22:00		45.1	45.7	44.6
	21:23:00	21:23:00		44.9	45.5	44.5
	21:24:00	21:24:00		44.6	45.1	44.2
	21:25:00	21:25:00		44.5	45.3	44.1
	21:26:00	21:26:00		47	49.2	44.5
	21:27:00	21:27:00		45.4	48.1	44.3
	21:28:00	21:28:00		44.4	44.8	44.1
	21:29:00	21:29:00		44.2	44.8	43.5
	21:30:00	21:30:00		44.1 44	44.7	43.5
	21:31:00 21:32:00	21:31:00 21:32:00		44.2	44.6 44.5	43.3 43.7
	21:32:00	21:33:00		44.9	51	43.7
	21:34:00	21:34:00		44.8	48.5	43.8
	21:35:00	21:35:00		47.2	50.7	44.5
	21:36:00	21:36:00		46.9	54.8	44.2
	21:37:00	21:37:00		45.6	53.9	43.8
	21:38:00	21:38:00		44.3	44.7	44
	21:39:00	21:39:00		44.4	45.1	43.9
	21:40:00	21:40:00		45	46.6	43.8
	21:41:00	21:41:00		47.9	50.7	45.4
	21:42:00	21:42:00		44.8	45.5	44.1
	21:43:00	21:43:00		44.4	44.7	44.1
	21:44:00	21:44:00		44.5	45.6	44.2
	21:45:00	21:45:00		44.6	45.6	43.9
	21:46:00	21:46:00		44.6	46.4	43.4
	21:47:00	21:47:00		45.7	49.1	44.5
	21:48:00	21:48:00		45.1	47.8	44
	21:49:00	21:49:00		54.9	62.1	45.5
	21:50:00 21:51:00	21:50:00 21:51:00		46.1	54.9 45.6	44.5
	21:51:00	21:51:00		44.7 44.5	45.6 45.4	44.1 44.1
	21:53:00	21:53:00		44.5	45.4	44.1
	21:54:00			44.5	45.9	
	21:55:00	21:55:00		44.6	45.1	
	21:56:00			44.5	45.1	43.9
		21:57:00		44.7	46.4	
	21:58:00	21:58:00		45.5	48.7	44.3
		21:59:00		44.7	45.5	44.1
	22:00:00	22:00:00		44.5	45	43.8
	22:01:00	22:01:00		44.5	46.2	44
	22:02:00	22:02:00		44.5	45	43.9
	22:03:00	22:03:00		44.4	44.7	44.1
	22:04:00	22:04:00		44.5	45.1	44
	22:05:00	22:05:00		44.4	44.8	44
		22:06:00		44.7	45.5	44.2
	22:07:00	22:07:00		44.8	45.5	44
	22:08:00	22:08:00		44.4	44.9	43.9
	22:09:00	22:09:00		44.5	44.9	44
	22:10:00	22:10:00		44.4	45	43.8
	22:11:00	22:11:00		44.3	44.9	
		22:12:00		44.4	46	43.8
	22:13:00	22:13:00		44.5	44.9	
	22:14:00	22:14:00		44.3	44.7	43.8
		22:15:00		44.5	45	44.1
	22:16:00	22:16:00		44.8	45.8	
	22:17:00	22:17:00		45.1	46.8	44.3
		22:18:00		44.5	44.9	44.1
		22:19:00		44.6	45 45	43.9
	22:20:00	22:20:00		44.4	45	44
		22:21:00		44.2 44.3	44.7	43.8
	22:22:00	22:22:00 22:23:00		44.3 45	44.9 46.2	43.8 44.2
	22.23.00	22.23.00		43	+0.2	44.2

	Danalina CDI
10:00:53 AM	29512.09227
10:00:53 AM	28840.31503
10:02:53 AM	29512.09227
10:03:53 AM	29512.09227
10:04:53 AM	29512.09227
10:05:53 AM	29512.09227
10:06:53 AM 10:07:53 AM	28840.31503 28840.31503
10:08:53 AM	29512.09227
10:09:53 AM	29512.09227
10:10:53 AM	30199.5172
10:11:53 AM	30902.95433
10:12:53 AM 10:13:53 AM	34673.68505 33113.11215
10:14:53 AM	30199.5172
10:15:53 AM	28183.82931
10:16:53 AM	28840.31503
10:17:53 AM	32359.36569
10:18:53 AM 10:19:53 AM	70794.57844 31622.7766
10:19:53 AM	31622.7766
10:21:53 AM	31622.7766
10:22:53 AM	32359.36569
10:23:53 AM	30902.95433
10:24:53 AM	28840.31503
10:25:53 AM 10:26:53 AM	28183.82931 50118.72336
10:27:53 AM	34673.68505
10:28:53 AM	27542.28703
10:29:53 AM	26302.67992
10:30:53 AM	25703.95783
10:31:53 AM 10:32:53 AM	25118.86432 26302.67992
10:32:53 AM	30902.95433
10:34:53 AM	30199.5172
10:35:53 AM	52480.74602
10:36:53 AM	48977.88194
10:37:53 AM 10:38:53 AM	36307.80548 26915.34804
10:39:53 AM	27542.28703
10:40:53 AM	31622.7766
10:41:53 AM	61659.50019
10:42:53 AM 10:43:53 AM	30199.5172 27542.28703
10:43:53 AM	28183.82931
10:45:53 AM	28840.31503
10:46:53 AM	28840.31503
10:47:53 AM	37153.52291
10:48:53 AM 10:49:53 AM	32359.36569 309029.5433
10:50:53 AM	40738.02778
10:51:53 AM	29512.09227
10:52:53 AM	28183.82931
10:53:53 AM 10:54:53 AM	28183.82931 31622.7766
10:55:53 AM	28840.31503
10:56:53 AM	28183.82931
10:57:53 AM	29512.09227
10:58:53 AM	35481.33892 29512.09227
10:59:53 AM 11:00:53 AM	28183.82931
11:01:53 AM	28183.82931
11:02:53 AM	28183.82931
11:03:53 AM	27542.28703
11:04:53 AM 11:05:53 AM	28183.82931 27542.28703
11:06:53 AM	29512.09227
11:07:53 AM	30199.5172
11:08:53 AM	27542.28703
11:09:53 AM	28183.82931
11:10:53 AM 11:11:53 AM	27542.28703 26915.34804
11:11:53 AM	27542.28703
11:13:53 AM	28183.82931
11:14:53 AM	26915.34804
11:15:53 AM	28183.82931
11:16:53 AM 11:17:53 AM	30199.5172 32359.36569
11:17:53 AM	28183.82931
11:19:53 AM	28840.31503
11:20:53 AM	27542.28703
11:21:53 AM 11:22:53 AM	26302.67992 26915.34804
11:22:53 AM 11:23:53 AM	31622.7766

Study	Study	Session	OL	Lavg	L _{max}	L _{min}
,	Time	Time	Status	Meter1	Meter1	Meter1
	22:24:00 22:25:00	22:24:00 22:25:00		47.7 44.5	50.8 45.6	44.8 43.9
	22:26:00	22:26:00		44.3	44.8	43.6
	22:27:00	22:27:00		54.6	62.6	44.1
	22:28:00	22:28:00		50	59.9	44.2
	22:29:00	22:29:00		44.8	46.1	44.1
	22:30:00	22:30:00		44.4	45.5	43.9
	22:31:00	22:31:00		44.1	44.7	43.6
	22:32:00	22:32:00		45.8	48.3	43.9
	22:33:00	22:33:00		46.7	53.4	44.2
	22:34:00	22:34:00		50	55.8	44
	22:35:00	22:35:00		44.3	44.9	43.5
	22:36:00	22:36:00		44.3	45	43.3
	22:37:00	22:37:00		44.3	44.8	43.6
	22:38:00	22:38:00		44.5	45	43.9
	22:39:00 22:40:00	22:39:00 22:40:00		44.5 44.9	45 46.4	44.1 44.2
	22:40:00	22:40:00		44.9	44.6	44.2
	22:42:00	22:42:00		44.4	44.8	43.7
	22:43:00	22:43:00		44.3	45.1	43.7
	22:44:00	22:44:00		44.3	44.8	43.6
	22:45:00	22:45:00		45.1	52.5	43.6
	22:46:00	22:46:00		45	47.3	43.8
	22:47:00	22:47:00		44.6	45.4	43.7
	22:48:00	22:48:00		44.6	45.4	43.9
	22:49:00	22:49:00		44.4	44.8	43.7
	22:50:00	22:50:00		44.4	44.9	43.7
	22:51:00	22:51:00		44.8	46.1	44.1
	22:52:00	22:52:00		44.9	47.3	43.8
	22:53:00	22:53:00		44.4	45	43.8
	22:54:00	22:54:00		44.6	47.1	43.9
	22:55:00	22:55:00		44.6	45.4	43.9
	22:56:00	22:56:00		44.5	45.1	44
	22:57:00	22:57:00		45.9	49.1	43.8
	22:58:00	22:58:00		44.3	44.7	43.7
	22:59:00 23:00:00	22:59:00 23:00:00		44.7 44.5	45.9 45.5	43.8 43.8
	23:01:00	23:01:00		44.4	47.2	43.8
	23:02:00	23:02:00		44.4	45.1	43.6
	23:03:00	23:03:00		44.7	46.2	43.6
	23:04:00	23:04:00		45.3	47.8	44
	23:05:00	23:05:00		45.4	48.2	43.6
	23:06:00	23:06:00		44.3	45.1	43.6
	23:07:00	23:07:00		44.5	45.5	43.9
	23:08:00	23:08:00		44.5	45.6	43.5
	23:09:00	23:09:00		44.5	45.3	43.6
	23:10:00	23:10:00		44.3	45.1	43.4
	23:11:00	23:11:00		44.2	45.4	43.5
	23:12:00	23:12:00		44.2	44.7	43.5
	23:13:00	23:13:00		44.4	45.1	43.7
	23:14:00	23:14:00		58.6	67.3	44.3
	23:15:00 23:16:00	23:15:00 23:16:00		47.2 44.3	54.5 45	43.9 43.5
	23:17:00	23:17:00		44.6	46	
	23:18:00	23:18:00		45.3	46.6	
	23:19:00	23:19:00		44.4	44.9	
	23:20:00	23:20:00		46.2	49.8	
	23:21:00	23:21:00		44.4	46.1	
	23:22:00	23:22:00		44.5	47.1	43.8
	23:23:00	23:23:00		45.6	56.2	43.6
	23:24:00	23:24:00		47.7	50	45.3
	23:25:00	23:25:00		45.8	48.1	44.5
	23:26:00	23:26:00		45.1	47.3	44.1
	23:27:00	23:27:00		44.6	45.2	44
	23:28:00	23:28:00		38.6	44.7	32.8
	23:29:00	23:29:00		34.7	39.4	33
	23:30:00	23:30:00		36.1	39.6	
	23:31:00	23:31:00		37.1	40.4	
	23:32:00	23:32:00		40.3	47.7	33.7
	23:33:00	23:33:00		39.9	44.9	
	23:34:00	23:34:00		34.5	36.6	
	23:35:00	23:35:00		33.3	34.6	
	23:36:00	23:36:00		33.5	34.9	
	23:37:00	23:37:00		35.2	41.5	31.9
	23:38:00 23:39:00	23:38:00 23:39:00		35.7 33.4	41.4 34.9	
	23:39:00	23:39:00		33.4 40.2	34.9 46.2	
	23:40:00	23:40:00		38.2	46.2	
	23:41:00	23:42:00		40.2	44.5	36.9
	23:42:00	23:42:00		41.3	45.9	
	23:44:00	23:44:00		36.4	40.4	
	23:45:00	23:45:00		37.4	40.4	
		23:46:00		38	42.4	
	23:46:00	23.40.00				

	Baseline SPL
11:24:53 AM	58884.36554
11:25:53 AM	28183.82931
11:26:53 AM	26915.34804
11:27:53 AM 11:28:53 AM	288403.1503 100000
11:29:53 AM	30199.5172
11:30:53 AM	27542.28703
11:31:53 AM 11:32:53 AM	25703.95783 38018.93963
11:32:53 AM	46773.51413
11:34:53 AM	100000
11:35:53 AM	26915.34804
11:36:53 AM 11:37:53 AM	26915.34804 26915.34804
11:38:53 AM	28183.82931
11:39:53 AM	28183.82931
11:40:53 AM	30902.95433 25118.86432
11:41:53 AM 11:42:53 AM	27542.28703
11:43:53 AM	26915.34804
11:44:53 AM	26915.34804
11:45:53 AM 11:46:53 AM	32359.36569 31622.7766
11:47:53 AM	28840.31503
11:48:53 AM	28840.31503
11:49:53 AM	27542.28703
11:50:53 AM 11:51:53 AM	27542.28703 30199.5172
11:52:53 AM	30902.95433
11:53:53 AM	27542.28703
11:54:53 AM	28840.31503
11:55:53 AM 11:56:53 AM	28840.31503 28183.82931
11:57:53 AM	38904.5145
11:58:53 AM	26915.34804
11:59:53 AM 12:00:53 PM	29512.09227 28183.82931
12:01:53 PM	27542.28703
12:02:53 PM	27542.28703
12:03:53 PM 12:04:53 PM	29512.09227 33884.41561
12:05:53 PM	34673.68505
12:06:53 PM	26915.34804
12:07:53 PM 12:08:53 PM	28183.82931 28183.82931
12:09:53 PM	28183.82931
12:10:53 PM	26915.34804
12:11:53 PM 12:12:53 PM	26302.67992 26302.67992
12:13:53 PM	27542.28703
12:14:53 PM	724435.9601
12:15:53 PM 12:16:53 PM	52480.74602 26915.34804
12:17:53 PM	28840.31503
12:18:53 PM	33884.41561
12:19:53 PM 12:20:53 PM	27542.28703 41686.93835
12:21:53 PM	27542.28703
12:22:53 PM	28183.82931
12:23:53 PM 12:24:53 PM	36307.80548 58884.36554
12:24:53 PM	38018.93963
12:26:53 PM	32359.36569
12:27:53 PM 12:28:53 PM	28840.31503
12:28:53 PM 12:29:53 PM	7244.359601 2951.209227
12:30:53 PM	4073.802778
12:31:53 PM	5128.61384
12:32:53 PM 12:33:53 PM	10715.19305 9772.37221
12:34:53 PM	2818.382931
12:35:53 PM	2137.96209
12:36:53 PM 12:37:53 PM	2238.721139 3311.311215
12:38:53 PM	3715.352291
12:39:53 PM	2187.761624
12:40:53 PM 12:41:53 PM	10471.28548 6606.93448
12:42:53 PM	10471.28548
12:43:53 PM	13489.62883
12:44:53 PM 12:45:53 PM	4365.158322 5495.408739
12:46:53 PM	6309.573445
12:47:53 PM	8128.305162

Study	Study	Session	OL	Lavg	L _{max}	L _{min}
Study	Time	Time	Status	Meter1	Meter1	Meter1
	23:48:00	23:48:00		38.1	41.3	35.4
	23:49:00	23:49:00		36.2	39.7	34.1
	23:50:00	23:50:00		37.5	40.8	34.7
	23:51:00	23:51:00		38.7	41.4	35.5
	23:52:00	23:52:00		45.8	55.5	35
	23:53:00	23:53:00		45.6	53.8	34.7
	23:54:00	23:54:00		37.5	41.4	34
	23:55:00	23:55:00		38.7	42	36.1
	23:56:00	23:56:00		38.9	43.8	35.1
	23:57:00	23:57:00		40.9	44.8	36
	23:58:00	23:58:00		37.9	42.5	34.2
	23:59:00	23:59:00		38.5	41.1	34.9
	24:00:00	24:00:00		39.7	42.2	36.9

	Baseline SPL
12:48:53 PM	6456.54229
12:49:53 PM	4168.693835
12:50:53 PM	5623.413252
12:51:53 PM	7413.102413
12:52:53 PM	38018.93963
12:53:53 PM	36307.80548
12:54:53 PM	5623.413252
12:55:53 PM	7413.102413
12:56:53 PM	7762.471166
12:57:53 PM	12302.68771
12:58:53 PM	6165.950019
12:59:53 PM	7079.457844
1:00:53 PM	9332.543008

Study #2 - Facility **Short-Duration (15-Minute)**

	Study	Session	OL	l	I	L	[
Study	Time	Time	Status	L _{avg} Meter1	L _{max} Meter1	L _{min} Meter1		Baseline SPL
	IIIIIC	11	Status	IVICTOI I	IVICTOI I	ivictoi 1	Time	(10 ^(Leq/10))
Study #2	0:00:10	1:00:10		46.9	49.5	45.7	3:37:31 PM	48977.8819
Receptor 1	0:00:20	1:00:20		48.6			3:37:41 PM	72443.596
	0:00:30	1:00:30		46	53.6		3:37:51 PM	39810.7171
	0:00:40	1:00:40		44	45.6	43	3:38:01 PM	25118.8643
	0:00:50	1:00:50		44.3	45.1	43.3	3:38:11 PM	26915.348
	0:01:00	1:01:00		43.8	44.8		3:38:21 PM	23988.3292
	0:01:10	1:01:10		44.4	46.2	42.9	3:38:31 PM	27542.287
	0:01:20	1:01:20		44.4			3:38:41 PM	27542.287
	0:01:30	1:01:30		43.7			3:38:51 PM	23442.2882
	0:01:40	1:01:40		44.4			3:39:01 PM	27542.287
	0:01:50 0:02:00	1:01:50 1:02:00		44.5 44.1	45.4 45.4		3:39:11 PM	28183.8293 25703.9578
	0:02:00	1:02:00		44.1			3:39:21 PM 3:39:31 PM	28840.315
	0:02:10	1:02:10		45.3			3:39:41 PM	33884.4156
	0:02:30	1:02:30		44			3:39:51 PM	25118.8643
	0:02:40	1:02:40		43.2			3:40:01 PM	20892.9613
	0:02:50	1:02:50		44	45		3:40:11 PM	25118.8643
	0:03:00	1:03:00		44.4	46.4	42.9	3:40:21 PM	27542.287
	0:03:10	1:03:10		45.1	45.9	43.8	3:40:31 PM	32359.3657
	0:03:20	1:03:20		45.6	47	43.9	3:40:41 PM	36307.8055
	0:03:30	1:03:30		45.7			3:40:51 PM	37153.5229
	0:03:40	1:03:40		48.8			3:41:01 PM	75857.7575
	0:03:50	1:03:50		50.2			3:41:11 PM	104712.855
	0:04:00	1:04:00		46.9			3:41:21 PM	48977.8819
	0:04:10 0:04:20	1:04:10		46.2			3:41:31 PM 3:41:41 PM	41686.9383
	0:04:20	1:04:20 1:04:30		46.6 46.6			3:41:51 PM	45708.819 45708.819
	0:04:40	1:04:40		49.2			3:42:01 PM	83176.3771
	0:04:50	1:04:50		48			3:42:11 PM	63095.7344
	0:05:00	1:05:00		47.2			3:42:21 PM	52480.746
	0:05:10	1:05:10		48	48.8		3:42:31 PM	63095.7344
	0:05:20	1:05:20		48	49.1	46.8	3:42:41 PM	63095.7344
	0:05:30	1:05:30		47.5	48.2	46	3:42:51 PM	56234.1325
	0:05:40	1:05:40		47.1			3:43:01 PM	51286.1384
	0:05:50	1:05:50		48.2			3:43:11 PM	66069.3448
	0:06:00	1:06:00		47.1			3:43:21 PM	51286.1384
	0:06:10	1:06:10		48.8	49.7		3:43:31 PM	75857.7575
	0:06:20 0:06:30	1:06:20 1:06:30		50.4 51	51.8 52.3		3:43:41 PM 3:43:51 PM	109647.82 125892.541
	0:06:40	1:06:40		50.5	51.8		3:44:01 PM	112201.845
	0:06:50	1:06:50		50.3	52		3:44:11 PM	107151.931
	0:07:00	1:07:00		52.3			3:44:21 PM	169824.365
	0:07:10	1:07:10		52.3	53.6	49.4	3:44:31 PM	169824.365
	0:07:20	1:07:20		49.1	50.1	48.5	3:44:41 PM	81283.0516
	0:07:30	1:07:30		48.2				66069.3448
	0:07:40	1:07:40		47.4				54954.0874
	0:07:50	1:07:50		47				50118.7234
	0:08:00 0:08:10	1:08:00 1:08:10		45.1 44.3				32359.3657
	0:08:10	1:08:10		44.4				26915.348 27542.287
	0:08:30	1:08:30		43.9				24547.0892
	0:08:40	1:08:40		42.9				19498.446
	0:08:50	1:08:50		43.4				21877.6162
	0:09:00	1:09:00		43.7	45.2	42.5	3:46:21 PM	23442.2882
	0:09:10	1:09:10		42.9	44.4	41.8	3:46:31 PM	19498.446
	0:09:20	1:09:20		42.4	43.6	41.1	3:46:41 PM	17378.0083
	0:09:30	1:09:30		42.3				16982.4365
	0:09:40	1:09:40		41.6				14454.3977
	0:09:50	1:09:50		41.8				15135.6125
	0:10:00	1:10:00		41.8				15135.6125
	0:10:10 0:10:20	1:10:10		40.9 41.9				12302.6877 15488 1662
	0:10:20	1:10:20 1:10:30		41.9				15488.1662 15488.1662
	0:10:30	1:10:30		42.4				17378.0083
	0:10:50	1:10:50		43.3				21379.6209
	0:11:00	1:11:00		42.1				16218.101

Start: 3:37:21 PM End: 3:52:21 PM Measured: 12/20/2018

Baseline Noise Level (15-Min.)

Study #2 - Facility Short-Duration (15-Minute)

0:11:10	1:11:10	42.8	44	41.9	3:48:31 PM	19054.6072
0:11:20	1:11:20	42.8	43.8	41.3	3:48:41 PM	19054.6072
0:11:30	1:11:30	43	44.9	41.3	3:48:51 PM	19952.6231
0:11:40	1:11:40	42.4	43.5	41.6	3:49:01 PM	17378.0083
0:11:50	1:11:50	42.9	44.2	42.3	3:49:11 PM	19498.446
0:12:00	1:12:00	45	46.1	43.8	3:49:21 PM	31622.7766
0:12:10	1:12:10	44	45.4	42.9	3:49:31 PM	25118.8643
0:12:20	1:12:20	44.9	46	43	3:49:41 PM	30902.9543
0:12:30	1:12:30	44.1	45.7	42.9	3:49:51 PM	25703.9578
0:12:40	1:12:40	43.7	45	42.4	3:50:01 PM	23442.2882
0:12:50	1:12:50	44.7	46.6	42.4	3:50:11 PM	29512.0923
0:13:00	1:13:00	45.4	48	43.6	3:50:21 PM	34673.685
0:13:10	1:13:10	49.8	51	47.9	3:50:31 PM	95499.2586
0:13:20	1:13:20	47.4	49.1	44.7	3:50:41 PM	54954.0874
0:13:30	1:13:30	46.7	48.2	44.7	3:50:51 PM	46773.5141
0:13:40	1:13:40	46.3	47.4	45.3	3:51:01 PM	42657.9519
0:13:50	1:13:50	47.5	50.3	45.7	3:51:11 PM	56234.1325
0:14:00	1:14:00	46.7	48.4	45.3	3:51:21 PM	46773.5141
0:14:10	1:14:10	50.4	51.6	47.4	3:51:31 PM	109647.82
0:14:20	1:14:20	51.4	52.4	50.2	3:51:41 PM	138038.426
0:14:30	1:14:30	50.4	51.7	49.2	3:51:51 PM	109647.82
0:14:40	1:14:40	50.6	51.7	49.2	3:52:01 PM	114815.362
0:14:50	1:14:50	49.9	50.7	49.3	3:52:11 PM	97723.7221
0:15:00	1:15:00	50	50.8	49.4	3:52:21 PM	100000

Study #3 - Haul Route Long-Duration (24-Hours)

Study	Study	Session	OL	Lavg	L _{max}	L _{min}					
	Time	Time	Status	Meter1	Meter1	Meter1	Time Date			Time	Baseline SPL (10 ^(Leq/10))
Study #3	0:01:00	0:01:00		54.8	8 61.6	40.4	Start: 7:43:29 PM 1/23/2019			7:44:29 PM	301995.172
Receptor 4	0:02:00 0:03:00	0:02:00 0:03:00		52.1 61.5			End: 7:43:29 PM 1/24/2019			7:45:29 PM 7:46:29 PM	162181.0097
	0:03:00	0:03:00		54.9						7:40:29 PM	1412537.545 309029.5433
	0:05:00	0:05:00		41.7			Baseline Noise Level (24-Hour) Baseline Noise Level - CNEL			7:48:29 PM	14791.08388
	0:06:00 0:07:00	0:06:00 0:07:00		45.4			24-Hour L _{eq} : 57.5 R4: 58.9 R5: 62.2			7:49:29 PM 7:50:29 PM	25118.86432 34673.68505
	0:08:00	0:08:00		44.5			Baseline Noise Level (L _{eq} 1H) @ R4			7:51:29 PM	28183.82931
	0:09:00	0:09:00		47			Daytime: 59.8			7:52:29 PM	50118.72336
	0:10:00 0:11:00	0:10:00 0:11:00		44.1 53.9			Evening: 50.7 Nighttime: 47.9			7:53:29 PM 7:54:29 PM	25703.95783 245470.8916
	0:12:00	0:12:00		52.5	5 57.9	44.2	- Ingression - Ing			7:55:29 PM	177827.941
	0:13:00 0:14:00	0:13:00 0:14:00		56.6 51.1			15-Min L _{ea} 24-Hour L _{ea} Difference			7:56:29 PM	457088.1896 128824.9552
	0:14:00	0:14:00		59.5			Study #4A 56.3 55.1 1.3	R5-A	Study #4A	7:57:29 PM 7:58:29 PM	891250.9381
	0:16:00	0:16:00		58.6			Study #5 66.3 46.4 19.8	R5-A	Study #4A	7:59:29 PM	724435.9601
	0:17:00 0:18:00	0:17:00 0:18:00		53.5 44.3			Study #4B 62.4 52.7 9.7 Study #4C 67.7 50.8 16.9	R5-A R5-A		8:00:29 PM 8:01:29 PM	223872.1139 26915.34804
	0:19:00	0:19:00		50.6			Study #6 69.9 52.3 17.5	R5-A		8:02:29 PM	114815.3621
	0:20:00 0:21:00	0:20:00 0:21:00		50.7 50.7			Composite Correction Factor @ R4: 13.0	R5-A R5-A		8:03:29 PM 8:04:29 PM	117489.7555 117489.7555
	0:22:00	0:21:00		45.9			Composite Correction Factor & N4. 13.0	R5-A		8:05:29 PM	38904.5145
	0:23:00	0:23:00		54.1			Daytime L _{eq} Evening L _{eq} Nighttime L _{eq}	R5-A		8:06:29 PM	257039.5783
	0:24:00 0:25:00	0:24:00 0:25:00		51.2 50.9			Study #4A 61.1 52.0 49.2 Study #5 79.7 70.6 67.7	R5-A R5-A		8:07:29 PM 8:08:29 PM	131825.6739 123026.8771
	0:26:00	0:26:00		50.3			Study #4B 69.5 60.4 57.6	R5-A		8:09:29 PM	107151.9305
	0:27:00 0:28:00	0:27:00 0:28:00		59.2 55.7			Study #4C 76.7 67.6 64.8 Study #6 77.4 68.3 65.4	R5-A R5-A		8:10:29 PM 8:11:29 PM	831763.7711 371535.2291
	0:29:00	0:29:00		52.3			Study #6 77.4 68.3 65.4	R5-A		8:12:29 PM	169824.3652
	0:30:00	0:30:00		59.4				R5-A		8:13:29 PM	870963.59
	0:31:00 0:32:00	0:31:00 0:32:00		55.6 59.2			Daytime L _{eq} Evening L _{eq} Nighttime L _{eq} Study #4 77.6 69.0 65.6			8:14:29 PM 8:15:29 PM	363078.0548 831763.7711
	0:33:00	0:33:00		57.3			Study #5 79.7 66.3 67.7			8:16:29 PM	537031.7964
	0:34:00 0:35:00	0:34:00		40 39			Study #6 77.4 69.9 65.4			8:17:29 PM	10000 7943.282347
	0:35:00	0:35:00 0:36:00		50.1						8:18:29 PM 8:19:29 PM	102329.2992
	0:37:00	0:37:00		55.7						8:20:29 PM	371535.2291
	0:38:00 0:39:00	0:38:00 0:39:00		39.9 39.6						8:21:29 PM 8:22:29 PM	9772.37221 9120.108394
	0:40:00	0:40:00		42	2 44.2	39.6				8:23:29 PM	15848.93192
	0:41:00 0:42:00	0:41:00 0:42:00		49.8 52.9						8:24:29 PM 8:25:29 PM	95499.2586 194984.46
	0:42:00	0:42:00		55.6						8:26:29 PM	363078.0548
	0:44:00	0:44:00		40.9						8:27:29 PM	12302.68771
	0:45:00 0:46:00	0:45:00 0:46:00		39.3 39.9						8:28:29 PM 8:29:29 PM	8511.380382 9772.37221
	0:47:00	0:47:00		39.7	7 40.8	38.7				8:30:29 PM	9332.543008
	0:48:00 0:49:00	0:48:00 0:49:00		39.2 39.2				R5-B	Study #5	8:31:29 PM 8:32:29 PM	8317.637711 8317.637711
	0:50:00	0:50:00		39.1				R5-B		8:33:29 PM	8128.305162
	0:51:00 0:52:00	0:51:00 0:52:00		39.7 39				R5-B R5-B		8:34:29 PM 8:35:29 PM	9332.543008 7943.282347
	0:53:00	0:52:00		38.7				R5-B		8:36:29 PM	7413.102413
	0:54:00	0:54:00		42.1				R5-B		8:37:29 PM	16218.10097
	0:55:00 0:56:00	0:55:00 0:56:00		42.5 40.5				R5-B R5-B		8:38:29 PM 8:39:29 PM	17782.7941 11220.18454
	0:57:00	0:57:00		42.4	4 47.8	38		R5-B	Study #5	8:40:29 PM	17378.00829
	0:58:00	0:58:00		57.1 40.6		41.8		R5-B R5-B		8:41:29 PM 8:42:29 PM	512861.384 11481.53621
	1:00:00	1:00:00		40.8	3 43.6	39.6		R5-B	Study #5	8:43:29 PM	12022.64435
	1:01:00 1:02:00	1:01:00 1:02:00		45.4 40.2				R5-B R5-B		8:44:29 PM 8:45:29 PM	34673.68505 10471.28548
	1:03:00	1:03:00		40.1	1 42.4	38.3		R5-B	Study #5	8:46:29 PM	10232.92992
	1:04:00 1:05:00	1:04:00 1:05:00		39.9 41.3			-	R5-B R5-A		8:47:29 PM 8:48:29 PM	9772.37221 13489.62883
	1:05:00	1:05:00		41.3				R5-A		8:48:29 PM 8:49:29 PM	12882.49552
	1:07:00	1:07:00		4:				R5-A	,	8:50:29 PM	12589.25412
	1:08:00 1:09:00	1:08:00 1:09:00		41.8 41.3				R5-A R5-A		8:51:29 PM 8:52:29 PM	15135.61248 13489.62883
	1:10:00	1:10:00		40.4	42.9	38.6		R5-A	Study #4B	8:53:29 PM	10964.78196
	1:11:00 1:12:00	1:11:00 1:12:00		43 40.6				R5-A R5-A		8:54:29 PM 8:55:29 PM	19952.62315 11481.53621
	1:13:00	1:13:00		51.6	5 56.2	42		R5-A	Study #4B	8:56:29 PM	144543.9771
	1:14:00 1:15:00	1:14:00 1:15:00		54.3 61.7				R5-A R5-A		8:57:29 PM 8:58:29 PM	269153.4804 1479108.388
	1:16:00	1:16:00		51.7	7 57.6	41		R5-A	Study #4B	8:59:29 PM	147910.8388
	1:17:00	1:17:00		54.1				R5-A		9:00:29 PM	257039.5783
	1:18:00 1:19:00	1:18:00 1:19:00		45.4 48.3				R5-A R5-A		9:01:29 PM 9:02:29 PM	34673.68505 67608.29754
	1:20:00	1:20:00		56.8	8 63	42.1		R5-A		9:03:29 PM	478630.0923
	1:21:00 1:22:00	1:21:00 1:22:00		40.5 38.8						9:04:29 PM 9:05:29 PM	11220.18454 7585.77575
	1:23:00	1:23:00		38.3	39.5	37.4				9:06:29 PM	6760.829754
	1:24:00 1:25:00	1:24:00 1:25:00		38.5 42.6						9:07:29 PM 9:08:29 PM	7079.457844 18197.00859
	1:25:00	1:25:00		40.9						9:08:29 PM 9:09:29 PM	12302.68771
	1:27:00	1:27:00		39.4						9:10:29 PM	8709.6359
	1:28:00 1:29:00	1:28:00 1:29:00		38.4 39.5						9:11:29 PM 9:12:29 PM	6918.309709 8912.509381
	1:30:00	1:30:00		38.6	5 40.6	37.5				9:13:29 PM	7244.359601
	1:31:00	1:31:00		44	4 52.1	37.9				9:14:29 PM	25118.86432

anty, cr				
1:32:00	1:32:00	52.6	57.8	42.2
1:33:00	1:33:00	39.7	43	37.8
1:34:00	1:34:00	39	41.9	37.4
1:35:00 1:36:00	1:35:00 1:36:00	38.2 39.1	39.9 40.5	37.1 38.3
1:37:00	1:37:00	41.8	45.1	38.8
1:38:00	1:38:00	39.1	41.1	37.8
1:39:00	1:39:00	40.3	41.6	38.5
1:40:00	1:40:00	40.6	44.7	38.6
1:41:00	1:41:00	42.1	46	39.3
1:42:00 1:43:00	1:42:00 1:43:00	39.8 40.4	42.1 41.7	38.7 39.2
1:43:00	1:44:00	39.7	41.7	39.2
1:45:00	1:45:00	39.3	41.9	38
1:46:00	1:46:00	42	47.9	38.9
1:47:00	1:47:00	41.9	46.6	39.4
1:48:00	1:48:00	40.2	43.2	38.8
1:49:00 1:50:00	1:49:00 1:50:00	39.4 39.5	41.3 41.3	38.4 38.2
1:51:00	1:51:00	39.1	41.4	37.4
1:52:00	1:52:00	38	39.2	37.1
1:53:00	1:53:00	41.1	43.5	37.8
1:54:00	1:54:00	38.9	40.4	37.3
1:55:00	1:55:00	38.2	40.6	37
1:56:00 1:57:00	1:56:00 1:57:00	39.3 39.1	40.4 40	37.8 37.6
1:58:00	1:58:00	38.3	40.5	37.4
1:59:00	1:59:00	40.8	50.1	37.3
2:00:00	2:00:00	40.7	43.1	39
2:01:00	2:01:00	40.6	42.5	38.6
2:02:00	2:02:00	41.2	43.3	39
2:03:00	2:03:00	39.6	41.9 39.8	38.5
2:04:00 2:05:00	2:04:00 2:05:00	38.4 40.8	39.8 42.5	37 37.7
2:06:00	2:06:00	38.8	40.4	37.7
2:07:00	2:07:00	39	40.9	37
2:08:00	2:08:00	38	39	37
2:09:00	2:09:00	37.9	40.6	36.7
2:10:00	2:10:00	37.1	38.4	36.2
2:11:00 2:12:00	2:11:00 2:12:00	37.4 37.8	38.9 39.4	36.6 36.5
2:12:00	2:12:00	37.8	40.1	37.6
2:14:00	2:14:00	39.4	42.4	37.3
2:15:00	2:15:00	52.4	58.2	41.6
2:16:00	2:16:00	50.5	55.9	42.1
2:17:00	2:17:00	41.2	43.3	39.5
2:18:00	2:18:00	40.1	41.8	39
2:19:00 2:20:00	2:19:00 2:20:00	40.8 39.6	42.2 41.1	39 38.1
2:21:00	2:21:00	39.9	41.1	38.9
2:22:00	2:22:00	39.8	41.3	38.6
2:23:00	2:23:00	39.4	40.9	38.4
2:24:00	2:24:00	38.7	41.7	37.3
2:25:00 2:26:00	2:25:00	38.1 38.5	38.9 39.6	37.1 37.5
2:27:00	2:27:00	38.9	39.7	37.8
2:28:00	2:28:00	38.9	40.4	37.7
2:29:00	2:29:00	40.3	43	38.6
2:30:00	2:30:00	52.8	59.2	40.3
2:31:00	2:31:00	51.2	55.3	44.8
2:32:00 2:33:00	2:32:00 2:33:00	43.8 42.8	45.9 46	42.3 40.4
2:34:00	2:34:00	46.7	53.7	39.8
2:35:00	2:35:00	54.3	60.3	40.6
2:36:00	2:36:00	41.6	44.9	39.1
2:37:00	2:37:00	42.2	44.3	40.5
2:38:00	2:38:00	41.6	44.6	40.1
2:39:00 2:40:00	2:39:00 2:40:00	41.7 42.5	43.7 45.5	40.5 40.6
2:41:00	2:41:00	43.6	45.7	41.4
2:42:00	2:42:00	43.5	51.5	39.9
2:43:00	2:43:00	42.1	49.6	40.4
2:44:00	2:44:00	41.6	43.7	40
2:45:00	2:45:00	43.3	46.1	40.3
2:46:00	2:46:00	59.3	68.5	45.7
2:47:00 2:48:00	2:47:00 2:48:00	54.9 43.7	64.6 48.5	48.4 40.1
2:49:00	2:49:00	40.3	42.5	38.9
2:50:00	2:50:00	42.4	45.6	40.4
2:51:00	2:51:00	42.7	46	41.1
2:52:00	2:52:00	42.2	44.1	40.6
2:53:00	2:53:00	43.5	45.3	41.4
2:54:00 2:55:00	2:54:00 2:55:00	44 44.5	45.5 46.2	41.8 42.6
2:55:00	2:55:00	44.5	46.2 51.1	42.6 41.2
2:57:00	2:57:00	41.8	43.1	40.7
2:58:00	2:58:00	42.4	44.5	41.2
2:59:00	2:59:00	42	43.4	41.3
3:00:00	3:00:00	41.7	44.3	40.4
3:01:00	3:01:00	41.1	41.8	40.1
3:02:00 3:03:00	3:02:00 3:03:00	42.1 42.3	43 44.1	41.5 41.1
3:03:00	3:03:00	42.4	44.1	40.8
3:05:00	3:05:00	43.4	46.7	42.2
3:06:00	3:06:00	45.3	50.3	42.8

9:15:29 PM	181970.0859
9:16:29 PM	9332.543008
9:17:29 PM	7943.282347
9:18:29 PM	6606.93448
9:19:29 PM 9:20:29 PM	8128.305162 15135.61248
9:20:29 PM	8128.305162
9:22:29 PM	10715.19305
9:23:29 PM	11481.53621
9:24:29 PM	16218.10097
9:25:29 PM	9549.92586
9:26:29 PM	10964.78196
9:27:29 PM	9332.543008
9:28:29 PM	8511.380382
9:29:29 PM	15848.93192
9:30:29 PM	15488.16619
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L:51:29 AM L:52:29 AM	9332.543008 239883.2919
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9:38:00	9:38:00	4	14.6	46.5	43.5
9:39:00	9:39:00	5	52.5	62.3	44.7
9:40:00	9:40:00	4	16.6	50.1	44.5
9:41:00	9:41:00		44	45.3	43
9:42:00	9:42:00		15.8	49.9	43.3
9:43:00	9:43:00		14.7	47.2	42.8
9:44:00	9:44:00		14.1	45.8	42.8
9:45:00	9:45:00		17.2	50.6	43.4
9:46:00 9:47:00	9:46:00 9:47:00		57.2 15.7	64.1 48.7	44 43
9:48:00	9:48:00		+3.7 14.4	40.7	42.4
9:49:00	9:49:00		13.4	45.4	42.2
9:50:00	9:50:00		14.1	45.3	42.5
9:51:00	9:51:00		14.7	46	43.8
9:52:00	9:52:00	4	14.1	47.3	42.4
9:53:00	9:53:00	5	57.5	68.2	42.8
9:54:00	9:54:00		47	55.3	43.9
9:55:00	9:55:00		13.6	45.5	42.5
9:56:00	9:56:00		14.2	46	42.9
9:57:00	9:57:00		13.9	44.8	42.3
9:58:00	9:58:00		14.1	45.2	43
9:59:00	9:59:00		52.7	61.4	43.7
10:00:00 10:01:00	10:00:00 10:01:00		14.7 15.3	45.9 47.5	43.8 43.5
10:01:00	10:01:00		+5.5 55.9	64.3	45.5
10:02:00	10:02:00	-	60	68.8	48.7
10:03:00	10:03:00		53.6	59.8	45.2
10:05:00	10:05:00		50.4	57.9	44.7
10:06:00	10:06:00		47.1	53.3	43
10:07:00	10:07:00		19.4	57.3	43.5
10:08:00	10:08:00	5	55.7	63.4	44.3
10:09:00	10:09:00	4	45.7	50	42.6
10:10:00	10:10:00	5	55.7	63	43.6
10:11:00	10:11:00		56.4	65.8	43.7
10:12:00	10:12:00		55.6	61.6	47.9
10:13:00	10:13:00		50.1	69.2	50
10:14:00	10:14:00		51.3	68.5	57.8
10:15:00 10:16:00	10:15:00 10:16:00		59.2 56.1	63.3 63	52 46.3
10:17:00	10:17:00		57.7	64.5	46.7
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10:37:00			58.6	65.7	50.6
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10:40:00	10:40:00		50.7 59.1	66	51.1
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	10:47:00		50.6	64.6	56.3
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10:49:00	10:49:00	5	55.3	59.5	45.1
10:50:00	10:50:00		60 59.7	66.6 71	52.4
10:51:00	10:51:00 10:52:00		59.7 51.2	71 68.4	53 53.2
10:52:00	10:52:00		58	63.4	49.5
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10:55:00	10:55:00		52.8	59.9	45.9
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	10:57:00		57.3	67.7	48.4
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	10:59:00		53.5	62.2	45.6
11:00:00	11:00:00		55.8	64.7	48.3
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13:44:00	13:44:00	58.1	65	49.2
13:45:00	13:45:00	58.8	63.6	51.9
13:46:00	13:46:00	56.9	64	47.7
13:47:00	13:47:00	56.4	62.9	49.8
13:48:00	13:48:00	50.9	56.2	47.1
13:49:00	13:49:00	52.2	57.3	48.2
13:50:00	13:50:00	51.4	57.4	48.3
13:51:00	13:51:00	60.5	71.1	47.8
13:52:00	13:52:00	52.7	55.9	50.3
13:53:00	13:53:00	56.2	61	49.4
13:54:00	13:54:00	54.5	61.7	50.3
13:55:00	13:55:00	50.6	60.6	43.7
13:56:00	13:56:00	53.7	59.9	44.5
13:57:00	13:57:00	64.5	75.8	52.4
13:58:00	13:58:00	59	68.3	50.6
13:59:00	13:59:00	54.1	61.1	47.6
14:00:00	14:00:00	57.5	64.4	49
14:01:00	14:01:00	55	64	48.6
14:02:00	14:02:00	59.2	64.3	50.4
14:03:00	14:03:00	56.3	66.7	48.6
14:04:00	14:04:00	54.3	63.3	48.5
14:05:00	14:05:00	55.3	60.1	48.6
14:06:00	14:06:00	61.3	69.2	52
14:07:00	14:07:00	55.9	61	51
14:08:00	14:08:00	57	63.5	48.3
14:09:00	14:09:00	55.8	61.2	50.8
14:10:00	14:10:00	55.8	61.9	51.2
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16:41:00	16:41:00	58.2	68.1	49.3
16:42:00	16:42:00	59.2	71.7	47.3
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16:43:00	16:43:00	66.4	77.5	50.4
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				50.5
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18:34:00	18:34:00	59.8	64.6	50.5
18:35:00	18:35:00	59	64.9	51.2
18:36:00	18:36:00	61.4	66.6	53.5
18:37:00	18:37:00 18:38:00	60.9	71.7	54.4 56.7
18:38:00		63.2	75.1	56.7
18:39:00 18:40:00	18:39:00 18:40:00	63.7 55.3	76.8 64.2	49.4 48
18:40:00	18:40:00	63.8	71.2	48 56.6
18:42:00	18:42:00	58.6	62.1	54.1
18:43:00	18:43:00	56.3	62.1	48.5
18:44:00	18:44:00	57.1	63.2	49.9
18:45:00	18:45:00	58.7	66.6	47.4
18:46:00	18:46:00	51	54.6	46.6
18:47:00	18:47:00	59.6	66	51.9
18:48:00	18:48:00	57.8	61.8	52.8
18:49:00	18:49:00	54.9	62.7	48.4
18:50:00	18:50:00	58.2	65.3	50.4
18:51:00	18:51:00	55.2	61.3	51.2
18:52:00	18:52:00	60.6	68.1	52.3
18:53:00	18:53:00	55.5	62.3	50.6
18:54:00	18:54:00	58	65.9	52.1
18:55:00	18:55:00	52	53.6	50.1
18:56:00	18:56:00	56.9	68.6	50.6

L:05:29 PM L:06:29 PM	549540.8739 194984.46
L:07:29 PM L:08:29 PM	162181.0097 100000
L:09:29 PM	229086.7653
L:10:29 PM L:11:29 PM	389045.145 1230268.771
L:12:29 PM	954992.586
L:13:29 PM	707945.7844
L:14:29 PM L:15:29 PM	1023292.992 1621810.097
L:16:29 PM	295120.9227
L:17:29 PM L:18:29 PM	426579.5188 707945.7844
L:19:29 PM	3162277.66
L:20:29 PM L:21:29 PM	275422.8703 331131.1215
L:22:29 PM	588843.6554
L:23:29 PM	1174897.555
L:24:29 PM L:25:29 PM	1698243.652 1819700.859
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L:32:29 PM	436515.8322
L:33:29 PM L:34:29 PM	1288249.552 1621810.097
L:35:29 PM	1047128.548
L:36:29 PM	1148153.621
L:37:29 PM L:38:29 PM	229086.7653 309029.5433
L:39:29 PM	1621810.097
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L:52:29 PM	1380384.265
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L:59:29 PM 2:00:29 PM	32359365.69 562341.3252
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2:02:29 PM 2:03:29 PM	2570395.783 2398832.919
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2:37:29 PM 2:38:29 PM	630957.3445 158489.3192
2:39:29 PM 2:39:29 PM	489778.8194

18:57:00	18:57:00	54.1	59.1	49.1
18:58:00	18:58:00	55.5	60.8	46.1
18:59:00	18:59:00	53.3	60.7	45.2
19:00:00	19:00:00	58.9	66.1	52.4
19:01:00	19:01:00	56.9	61.3	49
19:02:00	19:02:00	55.1	59.7	50.2
19:03:00	19:03:00	51.8	57.3	48.3
19:04:00	19:04:00	62	71.4	48.8
19:05:00	19:05:00	58.3	65.1	51.6
19:06:00	19:06:00	56.2	62.5	50
19:07:00	19:07:00	56.9	64.9	49.7
19:08:00	19:08:00	53.8	62	48.4
19:09:00	19:09:00	54.9	62.5	44.8
19:10:00	19:10:00	56.7	64	47.1
19:11:00	19:11:00	59.8	64.6	51.4
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19:13:00	19:13:00	56.5	66.5	46.5
19:14:00	19:14:00	56.4	64	50.6
19:15:00	19:15:00	55.1	61.3	50
19:16:00	19:16:00	60.6	70.2	51.7
19:17:00	19:17:00	62.8	74.7	50.4
19:18:00	19:18:00	59.6	66.8	56.6
19:19:00	19:19:00	60.7	65.6	52.6
19:20:00	19:20:00	58	62.3	52.9
19:21:00	19:21:00	59.1	67	53
19:22:00	19:22:00	61.6	71.3	51.4
19:23:00	19:23:00	59.9	64.6	52.8
19:24:00	19:24:00	56.9	61.7	50.1
19:25:00	19:25:00	56	61.5	50.6
19:26:00	19:26:00	59.2	68.5	49.5
19:27:00	19:27:00	54.1	57.9	48.9
19:28:00	19:28:00	54.5	59	50.4
19:29:00	19:29:00	51.8	57	48.4
19:30:00	19:30:00	57.4	64.9	48.7
19:31:00	19:31:00	55.7	63.3	48.5
19:32:00	19:32:00	54.8	59.8	49
19:33:00	19:33:00	63	75.7	57
19:34:00	19:34:00	55.5	58.3	51.4
19:35:00	19:35:00	58.2	67.2	48.7
19:36:00	19:36:00	63	67.7	57.2
19:37:00	19:37:00	61.8	70.7	54.6
19:38:00	19:38:00	58	62.2	53.7
19:39:00	19:39:00	59.2	68.1	52.8
19:40:00	19:40:00	61.1	69.2	55.1
19:41:00	19:41:00	59.3	66.1	51.9
19:42:00	19:42:00	52.5	59.6	48.9
19:43:00	19:43:00	60.7	67.5	47.1
19:44:00	19:44:00	58.6	68.2	47.8
19:45:00	19:45:00	58.8	67.1	51.1
19:46:00	19:46:00	57.3	63.2	50.9
19:47:00	19:47:00	55.6	59.3	50.5
19:48:00	19:48:00	56.9	60.2	52.3
19:49:00	19:49:00	60.6	63.8	58.1
19:50:00	19:50:00	54.6	61	48.5
19:51:00	19:51:00	56.8	62.1	49
19:52:00	19:52:00	57.6	65	50
19:53:00	19:53:00	54.6	60.8	48.7
19:54:00	19:54:00	54.3	59.4	45
19:55:00	19:55:00	56.7	64.6	45.1
19:56:00	19:56:00	55.5	62.9	49.6
19:57:00	19:57:00	63.5	71	54.5
19:58:00	19:58:00	58.9	64.5	52.4
19:59:00	19:59:00	59	68.5	49.3
20:00:00	20:00:00	59	64.4	51.6
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20:12:00	20:12:00	60.4	67.7	52.7
20:13:00	20:13:00	57.2	63.5	49.8
20:14:00	20:14:00	64.2	74.8	51.7
20:15:00	20:15:00	62.9	69.9	56.6
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20:22:00	20:22:00	53	58.2	46.2
20:23:00	20:23:00	51.1	56	47
20:24:00	20:24:00	66.9		52.4
20:25:00	20:25:00	59.4	65	54.6
20:26:00	20:26:00	58.3	64.4	49.2
20:27:00	20:27:00	54	58.5	49.8
20:28:00	20:28:00	50.2	55.2	47
20:29:00	20:29:00	55.3	64.4	48.6
20:30:00	20:30:00	54.5	58.8	50.3
20:31:00	20:31:00	50	56.9	46.2

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2:43:29 PM	776247.1166
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2:47:29 PM	1584893.192
2:48:29 PM	676082.9754
2:49:29 PM	416869.3835
2:50:29 PM 2:51:29 PM	489778.8194 239883.2919
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2:57:29 PM	436515.8322
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1:13:29 PM 1:14:29 PM	281838.2931 100000
	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,

20:32:00	20:32:00	47.8	51.8	44.7
20:33:00	20:33:00	57.2	65.4	46.3
20:34:00	20:34:00	57.5	65.4	43.2
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20:42:00	20:42:00	55.6	61.4	48.8
20:43:00	20:43:00	58.3	62.1	54
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20:45:00	20:45:00	55.7	60.1	45.7
20:46:00	20:46:00	52.7	59.6	45.2
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20:52:00	20:52:00	57.3	63.3	45.8
20:53:00	20:53:00	56.5	61.6	49.9
20:54:00	20:54:00	54.7	61.1	48.7
20:55:00	20:55:00	49.5	57.5	41.9
20:56:00	20:56:00	51.8	59.2	43.6
20:57:00	20:57:00	49.7	55.3	44.1
20:58:00	20:58:00	54.1	59.2	43.8
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21:00:00	21:00:00	56	60.7	46.7
21:01:00	21:01:00	55.2	60.1	45.3
21:02:00	21:02:00	53.5	59.3	47.9
21:03:00	21:03:00 21:04:00	53.7	57.9	46.8 43.4
21:04:00		50.8	56.8	
21:05:00	21:05:00	62.3	71	45.3
21:06:00	21:06:00 21:07:00	62.9	70.6	46.6
21:07:00		57.4	62.9	51.8
21:08:00	21:08:00	51.3	54.6 64.8	47.8
21:09:00	21:09:00	61		53.6
21:10:00 21:11:00	21:10:00 21:11:00	55.9	62.2	45.2
21:11:00	21:11:00	57.8 60.3	64.2	45.3 52
21:12:00	21:12:00	59.2	68.6 64.8	47.7
21:14:00	21:14:00	56.3	63	49.5
21:14:00	21:14:00	57.5	72.8	49.5
21:16:00	21:16:00	57.7	63.4	47.2
21:17:00	21:17:00	56.2	62.7	47.1
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21:27:00	21:27:00	47.2	53.6	43.4
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21:32:00	21:32:00	57.8	64.3	47.9
21:33:00	21:33:00	54.7	61.2	45.1
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21:36:00	21:36:00	55.2	65	46.2
	21:37:00	57.2	62	51.1
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21:40:00	21:40:00	44.6	51.3	41.6
21:41:00	21:41:00	43.6	47.7	41
21:42:00	21:42:00	56	64.6	47.7
21:43:00 21:44:00	21:43:00 21:44:00	64.5 49.3	76.8 59.2	48.1
	21:44:00	49.3 56.8		42.7 43.1
21:45:00	21:45:00	53.5	66.5 61.1	43.1 44
21:46:00		51.9	57.6	44.2
	21:47:00	49.4	57.6	44.2
21:49:00	21:49:00	46.5	53.9	42.7
21:50:00	21:50:00	48.8	56.4	42.8
21:51:00	21:51:00	52.2	57.8	44.5
	21:52:00	45.7	49.9	43.5
21:53:00	21:53:00	48.1	54.4	42.8
21:54:00	21:54:00	55.5	58	48.6
21:55:00	21:55:00	60.1	69.1	48.7
21:56:00	21:56:00	56.4	62.8	47.7
	21:57:00	51.4	57.7	42.3
21:58:00	21:58:00	43.1	44.9	41.6
21:59:00	21:59:00	47.8	53.7	42.7
22:00:00	22:00:00	47.1	55.4	41.6
22:01:00	22:01:00	41.9	44.5	40.6
	22:02:00	42.5	44.7	40.7
22:03:00	22:03:00	52.3	61	42.3
	22:04:00	54.3	62	42.7
22:05:00	22:05:00	44.4	48.6	41.7
22:06:00	22:06:00	45	50.5	42.2

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4:16:29 PM	524807.4602
4:17:29 PM	562341.3252
4:18:29 PM	123026.8771
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4:24:29 PM	1348962.883
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4:33:29 PM	208929.6131
4:34:29 PM	1584893.192
4:35:29 PM	537031.7964
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4:37:29 PM	295120.9227
4:38:29 PM	89125.09381
4:39:29 PM	151356.1248
4:40:29 PM	93325.43008
4:41:29 PM	257039.5783
4:42:29 PM	177827.941
4:43:29 PM	398107.1706
4:44:29 PM	331131.1215
4:45:29 PM	223872.1139
4:46:29 PM	234422.8815
4:47:29 PM	120226.4435
4:48:29 PM	1698243.652
4:49:29 PM	1949844.6
4:50:29 PM	549540.8739
	134896.2883
4:52:29 PM	1258925.412
4:53:29 PM	389045.145
4:54:29 PM	602559.5861
4:55:29 PM	1071519.305
4:56:29 PM	831763.7711
4:57:29 PM	426579.5188
4:58:29 PM	562341.3252
4:59:29 PM	588843.6554
5:00:29 PM	416869.3835
5:01:29 PM	812830.5162
5:02:29 PM	4265795.188
5:03:29 PM	162181.0097
5:04:29 PM	1122018.454
5:05:29 PM	69183.09709
5:06:29 PM	134896.2883
5:07:29 PM	128824.9552
5:08:29 PM	134896.2883
5:09:29 PM	87096.359
5:10:29 PM	
	52480.74602
5:11:29 PM	194984.46
5:12:29 PM	48977.88194
5:13:29 PM	257039.5783
5:14:29 PM	213796.209
5:15:29 PM	602559.5861
5:16:29 PM	295120.9227
5:17:29 PM	204173.7945
5:18:29 PM	295120.9227
5:19:29 PM	331131.1215
5:20:29 PM	524807.4602
5:21:29 PM	123026.8771
5:22:29 PM	204173.7945
5:23:29 PM	28840.31503
5:24:29 PM	22908.67653
5:25:29 PM	398107.1706
5:26:29 PM	2818382.931
5:27:29 PM	85113.80382
5:28:29 PM	478630.0923
5:29:29 PM	223872.1139
5:30:29 PM	154881.6619
5:31:29 PM	87096.359
5:32:29 PM	44668.35922
5:33:29 PM	75857.7575
5:34:29 PM	165958.6907
5:35:29 PM	37153.52291
5:36:29 PM	64565.4229
5:37:29 PM	354813.3892
5:38:29 PM	1023292.992
5:39:29 PM	436515.8322
5:40:29 PM	138038.4265
5:41:29 PM	20417.37945
5:42:29 PM	60255.95861
5:43:29 PM	51286.1384
5:44:29 PM	15488.16619
5:45:29 PM	17782.7941
5:46:29 PM	169824.3652
5:47:29 PM	269153.4804
5:48:29 PM	27542.28703
	0,00
5:49:29 PM	31622.7766
5:49:29 PM	31622.7766

					Long-Duration (24-nours)		
22:07:00	22:07:00	51.4	61.4	42.4		5:50:29 PM	138038.4265
22:08:00	22:08:00	46.2	50.6	42.2		5:51:29 PM	41686.93835
22:09:00	22:09:00	44.9	47.7	43.1		5:52:29 PM	30902.95433
22:10:00	22:10:00	50.9	58.7	42.4		5:53:29 PM	123026.8771
22:11:00 22:12:00	22:11:00	49.8	55.6	44.2		5:54:29 PM	95499.2586
22:12:00	22:12:00	56.2 44	64.4 47.7	43.8 41.6		5:55:29 PM 5:56:29 PM	416869.3835 25118.86432
22:14:00		44.4	49.5	40.8		5:57:29 PM	27542.28703
22:15:00	22:15:00	55.4	65.1	43.5		5:58:29 PM	346736.8505
22:16:00	22:16:00	49.5	56.1	43.1		5:59:29 PM	89125.09381
22:17:00	22:17:00	43.4	45.6	41.8		6:00:29 PM	21877.61624
22:18:00		57.5	68.7	41.8		6:01:29 PM	562341.3252
22:19:00 22:20:00	22:19:00 22:20:00	42.8 45.4	45.9 48.1	41.3 42.1		6:02:29 PM 6:03:29 PM	19054.60718 34673.68505
	22:21:00	44.5	49.3	41.5		6:04:29 PM	28183.82931
22:22:00	22:22:00	42.4	44	40.3		6:05:29 PM	17378.00829
22:23:00	22:23:00	44.3	51.4	41.4		6:06:29 PM	26915.34804
22:24:00	22:24:00	49.6	57.8	42.3		6:07:29 PM	91201.08394
22:25:00	22:25:00	52.7	63.7	41.5		6:08:29 PM	186208.7137
22:26:00 22:27:00	22:26:00 22:27:00	43.9	49.3 78.1	41.3 44.2		6:09:29 PM	24547.08916
22:28:00	22:28:00	65.1 46.9	52.8	42.2		6:10:29 PM 6:11:29 PM	3235936.569 48977.88194
22:29:00	22:29:00	42.7	44.8	41.6		6:12:29 PM	18620.87137
22:30:00	22:30:00	54.9	65.7	41.2		6:13:29 PM	309029.5433
22:31:00	22:31:00	50.4	58.1	41.9		6:14:29 PM	109647.8196
22:32:00		47.9	53.7	44.3		6:15:29 PM	61659.50019
22:33:00	22:33:00	48.4	53	44.7		6:16:29 PM	69183.09709
22:34:00	22:34:00	45.9 48.4	49.8	44		6:17:29 PM	38904.5145
22:35:00 22:36:00	22:35:00 22:36:00	46.1	53.6 49.3	43.6 43.6		6:18:29 PM 6:19:29 PM	69183.09709 40738.02778
22:37:00	22:37:00	43.3	44.9	41.6		6:20:29 PM	21379.6209
22:38:00	22:38:00	43.3	46.3	41.3		6:21:29 PM	21379.6209
22:39:00	22:39:00	43	45.9	41		6:22:29 PM	19952.62315
22:40:00	22:40:00	41.2	42.8	40.2		6:23:29 PM	13182.56739
22:41:00	22:41:00	41.4	44.3	39.9		6:24:29 PM	13803.84265
	22:42:00	46.3	53.6	40.9		6:25:29 PM	42657.95188 2951209.227
22:43:00 22:44:00	22:43:00 22:44:00	64.7 46.3	71 52.9	52.5 41.9		6:26:29 PM 6:27:29 PM	42657.95188
22:45:00	22:45:00	55.5	65.8	43.7		6:28:29 PM	354813.3892
22:46:00	22:46:00	49.3	53.7	43.7		6:29:29 PM	85113.80382
22:47:00	22:47:00	55.7	65.1	45.4		6:30:29 PM	371535.2291
22:48:00	22:48:00	49	55.9	43.2		6:31:29 PM	79432.82347
22:49:00	22:49:00	55.3	67	45.3		6:32:29 PM	338844.1561
22:50:00 22:51:00	22:50:00 22:51:00	47.4	53.4 47.6	42.4		6:33:29 PM	54954.08739
22:52:00	22:52:00	47.2 54.6	63.1	46.9 43.9		6:34:29 PM 6:35:29 PM	52480.74602 288403.1503
22:53:00	22:53:00	41.7	43.9	40.1		6:36:29 PM	14791.08388
22:54:00	22:54:00	41.8	43	40.6		6:37:29 PM	15135.61248
22:55:00	22:55:00	41.2	43.1	39.4		6:38:29 PM	13182.56739
22:56:00	22:56:00	41.3	44.2	39.4		6:39:29 PM	13489.62883
22:57:00	22:57:00	43.5	47.2	40.9		6:40:29 PM	22387.21139
22:58:00 22:59:00	22:58:00	46.9	54.4	43.4		6:41:29 PM	48977.88194
23:00:00	22:59:00 23:00:00	49.3 47.3	55 54.6	42 42.1		6:42:29 PM 6:43:29 PM	85113.80382 53703.17964
	23:01:00	55.8	62.8	48.9		6:44:29 PM	380189.3963
23:02:00	23:02:00	44.2	49.7	40.6		6:45:29 PM	26302.67992
23:03:00	23:03:00	43.3	46.2	41.4		6:46:29 PM	21379.6209
23:04:00	23:04:00	47.8	54.7	40.9		6:47:29 PM	60255.95861
23:05:00	23:05:00	47.6	53.6	42		6:48:29 PM	57543.99373
23:06:00 23:07:00		49 44.7	57.5 50.2	42.3 42.5		6:49:29 PM 6:50:29 PM	79432.82347 29512.09227
23:08:00		44.2	52.3	39.9		6:51:29 PM	26302.67992
23:09:00		51.2	54.8	46.7		6:52:29 PM	131825.6739
23:10:00		48.9	56.5	44.6		6:53:29 PM	77624.71166
23:11:00		45.4	47.8	42.7		6:54:29 PM	34673.68505
23:12:00		43.4	44.6	42.1		6:55:29 PM	21877.61624
23:13:00		41.7	44.8	40.3		6:56:29 PM	14791.08388
23:14:00 23:15:00		41.9 42.6	44.1 44.2	41 41		6:57:29 PM 6:58:29 PM	15488.16619 18197.00859
23:16:00		44.7	47.4	41.7		6:59:29 PM	29512.09227
23:17:00		44.6	47.7	42.5	R5-A Study	#4C 7:00:29 PM	28840.31503
23:18:00	23:18:00	55.9	65.3	42.4	R5-A Study	#4C 7:01:29 PM	389045.145
23:19:00		44.1	48.9	41.4		#4C 7:02:29 PM	25703.95783
23:20:00		42.6	45.5 48.6	40.3		#4C 7:03:29 PM	18197.00859
23:21:00 23:22:00		43.1 59	48.6 68.8	41.7 48.6	R5-A Study: R5-A Study:	#4C 7:04:29 PM #4C 7:05:29 PM	20417.37945 794328.2347
23:23:00		49.5	62.5	41.8		#4C 7:06:29 PM	89125.09381
23:24:00		45.6	51.6	41.3		#4C 7:07:29 PM	36307.80548
23:25:00		45.7	51.6	41.8		#4C 7:08:29 PM	37153.52291
23:26:00	23:26:00	44.2	46.7	41.1		#4C 7:09:29 PM	26302.67992
23:27:00		43.7	47.2	42.1		#4C 7:10:29 PM	23442.28815
23:28:00		42.6 51.6	44.3	41.2		#4C 7:11:29 PM	18197.00859
23:29:00 23:30:00		51.6 44.6	59.2 48.9	41.8 42		#4C 7:12:29 PM #4C 7:13:29 PM	144543.9771 28840.31503
23:30:00		44.6	48.9 58.2	40.4	R5-A Study		95499.2586
23:32:00		51.8	57.9	41.5	R5-A Study		151356.1248
23:33:00		43.6	45.4	41.7		7:16:29 PM	22908.67653
23:34:00		45	50.2	42.1		7:17:29 PM	31622.7766
23:35:00		43.2	44.6	42		7:18:29 PM	20892.96131
23:36:00		45.1	50.9	42.4		7:19:29 PM	32359.36569
23:37:00 23:38:00		44.1 44.6	45.2 47.3	43.1 42.5		7:20:29 PM 7:21:29 PM	25703.95783 28840.31503
23:38:00		44.8	47.3	42.5		7:21:29 PM 7:22:29 PM	30199.5172
23:40:00		46	48	44		7:23:29 PM	39810.71706
23:41:00		50.3	57.7	46		7:24:29 PM	107151.9305

3:42:00	23:42:00	45.9	52	41.9		7:25:29 PM	38904.5
23:43:00	23:43:00	43.9	47	41.5		7:26:29 PM	24547.08
23:44:00	23:44:00	51.3	57.9	43.7	R5-C Study #6	7:27:29 PM	134896.2
23:45:00	23:45:00	51.3	60.6	41.7	R5-C Study #6	7:28:29 PM	134896.2
23:46:00	23:46:00	44.7	46.9	42.4	R5-C Study #6	7:29:29 PM	29512.092
23:47:00	23:47:00	53.2	60.3	42.8	R5-C Study #6	7:30:29 PM	208929.63
23:48:00	23:48:00	54.7	64.8	46.1	R5-C Study #6	7:31:29 PM	295120.92
23:49:00	23:49:00	54.7	63.8	47	R5-C Study #6	7:32:29 PM	295120.92
23:50:00	23:50:00	47.9	51.4	43.8	R5-C Study #6	7:33:29 PM	61659.500
23:51:00	23:51:00	48.7	53.4	45	R5-C Study #6	7:34:29 PM	74131.024
23:52:00	23:52:00	48.5	54.7	44.8	R5-C Study #6	7:35:29 PM	70794.578
23:53:00	23:53:00	45.4	49.1	42.5	R5-C Study #6	7:36:29 PM	34673.685
23:54:00	23:54:00	49.6	57.8	42.7	R5-C Study #6	7:37:29 PM	91201.083
23:55:00	23:55:00	51.8	58.3	46.6	R5-C Study #6	7:38:29 PM	151356.12
23:56:00	23:56:00	52.6	59.5	44.9	R5-C Study #6	7:39:29 PM	181970.08
23:57:00	23:57:00	50.3	59.6	43	R5-C Study #6	7:40:29 PM	107151.93
23:58:00	23:58:00	48	54.1	43.6	R5-C Study #6	7:41:29 PM	63095.734
23:59:00	23:59:00	59.1	74.1	45.9	R5-C Study #6	7:42:29 PM	812830.51
24:00:00	24:00:00	51.2	56.4	43		7:43:29 PM	131825.67

Study #4, #5, #6 - Haul Route Short-Duration (15-Minute)

Study	Study	Session	OL	Lavg	L _{max}	L _{min}				
Study	Time	Time	Status	Meter1	Meter1	Meter1				Baseline SPL
									Time	(10 ^(Leq/10))
Study #4A		0:00:10		61		49.1		7:58:44 PM	7:58:54 PM	1258925.412
R5-A	0:00:20	0:00:20		45.4		42.4		8:13:44 PM	7:59:04 PM	34673.68505
	0:00:30 0:00:40	0:00:30 0:00:40		41.8 41.4		41.1 41.1		1/23/2019	7:59:14 PM	15135.61248 13803.84265
	0:00:40	0:00:40		43.8				Evening	7:59:24 PM 7:59:34 PM	23988.32919
	0:00:50	0:01:00		41.8		41.2			7:59:44 PM	15135.61248
	0:01:10	0:01:10		43		41.3		oise Level	7:59:54 PM	19952.62315
	0:01:20	0:01:20		46		44.4		56.3	8:00:04 PM	39810.71706
	0:01:30	0:01:30		48.4	55.7	44.1		61.3	8:00:14 PM	69183.09709
	0:01:40	0:01:40		44.3	45.6	43.7	<u></u>		8:00:24 PM	26915.34804
	0:01:50	0:01:50		46.2					8:00:34 PM	41686.93835
	0:02:00	0:02:00		49.1					8:00:44 PM	81283.05162
	0:02:10	0:02:10		51.6					8:00:54 PM	144543.9771
	0:02:20 0:02:30	0:02:20 0:02:30		52.5 50.7		50.7 48.4			8:01:04 PM	177827.941 117489.7555
	0:02:40	0:02:30		47.2					8:01:14 PM 8:01:24 PM	52480.74602
	0:02:50	0:02:40		46.9		43.6			8:01:34 PM	48977.88194
	0:03:00	0:03:00		44.7		43.7			8:01:44 PM	29512.09227
	0:03:10	0:03:10		47.6	50	46.5			8:01:54 PM	57543.99373
	0:03:20	0:03:20		50.3		49.4			8:02:04 PM	107151.9305
	0:03:30	0:03:30		53.8		51			8:02:14 PM	239883.2919
	0:03:40	0:03:40		56.2		54.1			8:02:24 PM	416869.3835
	0:03:50	0:03:50		52.3					8:02:34 PM	169824.3652
	0:04:00 0:04:10	0:04:00 0:04:10		53.7 54.3		52.8 53.4			8:02:44 PM	234422.8815 269153.4804
	0:04:10	0:04:10		54.5 51.8					8:02:54 PM 8:03:04 PM	151356.1248
	0:04:30	0:04:30		49.6		47.1			8:03:14 PM	91201.08394
	0:04:40	0:04:40		46.4					8:03:24 PM	43651.58322
	0:04:50	0:04:50		45.5	45.9	45.1			8:03:34 PM	35481.33892
	0:05:00	0:05:00		44.5	46.4	43.7			8:03:44 PM	28183.82931
	0:05:10	0:05:10		45.2		43.4			8:03:54 PM	33113.11215
	0:05:20	0:05:20		45.4		43.9			8:04:04 PM	34673.68505
	0:05:30	0:05:30		45.2		44.7			8:04:14 PM	33113.11215
	0:05:40 0:05:50	0:05:40 0:05:50		47.1 51.6		44.8 48.7			8:04:24 PM 8:04:34 PM	51286.1384 144543.9771
	0:06:00	0:06:00		55.1		52.6			8:04:44 PM	323593.6569
	0:06:10	0:06:10		57.2		54.8			8:04:54 PM	524807.4602
	0:06:20	0:06:20		56.7					8:05:04 PM	467735.1413
	0:06:30	0:06:30		55	56.4	53.5			8:05:14 PM	316227.766
	0:06:40	0:06:40		52.6		51.7			8:05:24 PM	181970.0859
	0:06:50	0:06:50		53		52.1			8:05:34 PM	199526.2315
	0:07:00	0:07:00		53.4					8:05:44 PM	218776.1624
	0:07:10 0:07:20	0:07:10 0:07:20		56.5 52.5					8:05:54 PM 8:06:04 PM	446683.5922 177827.941
	0:07:20	0:07:20		48.5					8:06:14 PM	70794.57844
	0:07:40	0:07:40		45.4		43.6			8:06:24 PM	34673.68505
	0:07:50	0:07:50		43.5					8:06:34 PM	22387.21139
	0:08:00	0:08:00		44.4	45.1				8:06:44 PM	27542.28703
	0:08:10	0:08:10		46					8:06:54 PM	39810.71706
	0:08:20	0:08:20		46.4					8:07:04 PM	43651.58322
	0:08:30	0:08:30		54.5					8:07:14 PM	281838.2931
	0:08:40 0:08:50	0:08:40 0:08:50		58.8 55		44.8 44			8:07:24 PM 8:07:34 PM	758577.575 316227.766
	0:08:50	0:08:50		50.5					8:07:44 PM	112201.8454
	0:09:10	0:09:10		50.3		45.5			8:07:54 PM	102329.2992
	0:09:20	0:09:20		54.8					8:08:04 PM	301995.172
	0:09:30	0:09:30		52.6					8:08:14 PM	181970.0859
	0:09:40	0:09:40		52.1	52.8				8:08:24 PM	162181.0097
	0:09:50	0:09:50		47.7					8:08:34 PM	58884.36554
	0:10:00	0:10:00		44.5					8:08:44 PM	28183.82931
	0:10:10	0:10:10		45.2					8:08:54 PM	33113.11215
	0:10:20 0:10:30	0:10:20		48.3 57.9					8:09:04 PM	67608.29754
	0:10:30	0:10:30 0:10:40		57.9 68					8:09:14 PM 8:09:24 PM	616595.0019 6309573.445
	0.10.40	0.10.40		00	. /2.3	01.0			0.UJ.Z4 FIVI	0000070.440

Ventura Co	ounty, CA			Short	-Duratior	ı (15-Minute)		Pacific Rock Qu
	0.40.50	0.40.50	50 F	62.0	57.0		0.00.24.014	4422040 454
	0:10:50	0:10:50	60.5	63.8	57.9		8:09:34 PM	1122018.454
	0:11:00	0:11:00	59	67	57.2		8:09:44 PM	794328.2347
	0:11:10	0:11:10	58.7	66.5	50.2		8:09:54 PM	741310.2413
	0:11:20	0:11:20	47.7	50.5	45.3		8:10:04 PM	58884.36554
	0:11:30	0:11:30	49.4	53.3	46.5		8:10:14 PM	87096.359
	0:11:40	0:11:40	59.6	61	53.3		8:10:24 PM	912010.8394
	0:11:50	0:11:50	59.6	60.4	58.2		8:10:34 PM	912010.8394
	0:12:00	0:12:00	56.7	58.4	53.8		8:10:44 PM	467735.1413
	0:12:10	0:12:10	52.4	54	50.3		8:10:54 PM	173780.0829
	0:12:20	0:12:20	48.1	51.5	45.5		8:11:04 PM	64565.4229
	0:12:30	0:12:30	45.9	46.6	45.3		8:11:14 PM	38904.5145
	0:12:40	0:12:40	46.4	51.5	45.1		8:11:24 PM	43651.58322
	0:12:50	0:12:50	56.2	60.4	48.8		8:11:34 PM	416869.3835
	0:13:00	0:13:00	58.9	61	56		8:11:44 PM	776247.1166
	0:13:10	0:13:10	58.7	60.5	56.5		8:11:54 PM	741310.2413
	0:13:20	0:13:20	58.8	60.9	56.8		8:12:04 PM	758577.575
	0:13:30	0:13:30	61.1	66.1	55.5		8:12:14 PM	1288249.552
	0:13:40	0:13:40	64.4	67.4	61.6		8:12:24 PM	2754228.703
	0:13:50	0:13:50	57.9	61.6	53.9		8:12:34 PM	616595.0019
	0:14:00	0:14:00	54.6	57.1	52.5		8:12:44 PM	288403.1503
	0:14:10	0:14:10	56.4	58.8	54.1		8:12:54 PM	436515.8322
	0:14:20	0:14:20	57.7	59.7	56.1		8:13:04 PM	588843.6554
	0:14:30	0:14:30	57.6	61.1	52.9		8:13:14 PM	575439.9373
	0:14:40	0:14:40	53.4	55.3	52.3			218776.1624
	0:14:50	0:14:40	58.1	61.6			8:13:24 PM	645654.229
	0:14:30	0:14:30	67.9	71.4	55.3 57		8:13:34 PM	6165950.019
Study #5	0:00:10	0:00:10	61.4	63.7	59.3	Start: 8:32:50 PM	8:13:44 PM	1380384.265
R5-B	0:00:10	0:00:10				End: 8:47:50 PM	8:33:00 PM	
иэ-в	0:00:20		61.8 65.7	63 67.8	59.5 62.3	Measured: 1/23/2019	8:33:10 PM	1513561.248
	0:00:30	0:00:30 0:00:40	62.7		60.5		8:33:20 PM	3715352.291
	0:00:40		63.2	64.2	59	Evening	8:33:30 PM	1862087.137
		0:00:50		65.4			8:33:40 PM	2089296.131
	0:01:00	0:01:00	56.2	59	53.5	Desaline Naise Level	8:33:50 PM	416869.3835
	0:01:10	0:01:10	57.4	58.3	56	Baseline Noise Level	8:34:00 PM	549540.8739
	0:01:20	0:01:20	55.5	58.7	53.4	L _{eq} : 66.3	8:34:10 PM	354813.3892
	0:01:30	0:01:30	60.5	62.3	58.4	CNEL: 71.3	8:34:20 PM	1122018.454
	0:01:40	0:01:40	59.8	61.5	58.2		8:34:30 PM	954992.586
	0:01:50	0:01:50	60.3	61.6	58.9		8:34:40 PM	1071519.305
	0:02:00	0:02:00	60.3	62.4	58		8:34:50 PM	1071519.305
	0:02:10	0:02:10	60.7	63.6	58.5		8:35:00 PM	1174897.555
	0:02:20	0:02:20	63.3	67.5	61.3		8:35:10 PM	2137962.09
	0:02:30	0:02:30	59.8	62.3	55.5		8:35:20 PM	954992.586
	0:02:40	0:02:40	60.7	68.2	54		8:35:30 PM	1174897.555
	0:02:50	0:02:50	69.3	72.4	60.2		8:35:40 PM	8511380.382
	0:03:00	0:03:00	61.6	64.1	57.7		8:35:50 PM	1445439.771
	0:03:10	0:03:10	60	64	56.1		8:36:00 PM	1000000
	0:03:20	0:03:20	60.5	63.4	56.2		8:36:10 PM	1122018.454
	0:03:30	0:03:30	62.5	65	58.4		8:36:20 PM	1778279.41
	0:03:40	0:03:40	64.7	67.6	61.1		8:36:30 PM	2951209.227
	0:03:50	0:03:50	62.8	64.8	60		8:36:40 PM	1905460.718
	0:04:00	0:04:00	62.1	62.9	60		8:36:50 PM	1621810.097
	0:04:10	0:04:10	63.4	64.2	62.6		8:37:00 PM	2187761.624
	0:04:20	0:04:20	64.1	66	60.1		8:37:10 PM	2570395.783
	0:04:30	0:04:30	63.7	65.4	62.4		8:37:20 PM	2344228.815
	0:04:40	0:04:40	63.9	66.1	61.5		8:37:30 PM	2454708.916
	0:04:50	0:04:50	70.7	74.1	64.3		8:37:40 PM	11748975.55
	0:05:00	0:05:00	66.5	68.6	63.6		8:37:50 PM	4466835.922
	0:05:10	0:05:10	65.5	67.8	64.4		8:38:00 PM	3548133.892
	0:05:20	0:05:20	65.3	67.2	63.8		8:38:10 PM	3388441.561
	0:05:30	0:05:30	61.6	63.8	58.2		8:38:20 PM	1445439.771
	0:05:40	0:05:40	68.2	76.7	58		8:38:30 PM	6606934.48
	0:05:50	0:05:50	70.1	76.2	64.8		8:38:40 PM	10232929.92
	0:06:00	0:06:00	61	64.7	54.9		8:38:50 PM	1258925.412
	0:06:10	0:06:10	57.6	59.2	55.8		8:39:00 PM	575439.9373
	0:06:20	0:06:20	55.9	58.8	53.2		8:39:10 PM	389045.145
	0:06:30	0:06:30	58.7	60.4	56.2		8:39:20 PM	741310.2413
	0:06:40	0:06:40	58.7	59.9	56.6		8:39:30 PM	741310.2413
	0:06:50	0:06:50	58.8	60.7	54.1		8:39:40 PM	758577.575

	**			Short-D	uration (15-r	viinute)		
	0:07:00	0:07:00	58.8	C1 1	54		8:30:E0 DM	750577 575
	0:07:00	0:07:00	58.8 59.9	61.1 62.4	54 57.1		8:39:50 PM 8:40:00 PM	758577.575 977237.221
	0:07:10	0:07:20	62.7	65.3	56.9		8:40:10 PM	1862087.137
	0:07:20	0:07:30	62.4	65.4	60.1		8:40:20 PM	1737800.829
	0:07:40	0:07:40	62	64.4	57.5		8:40:30 PM	1584893.192
	0:07:50	0:07:50	60.1	61.8	57.5		8:40:40 PM	1023292.992
	0:08:00	0:08:00	60.7	65.3	57.6		8:40:50 PM	1174897.555
	0:08:10	0:08:10	72.6	74.9	65.3		8:41:00 PM	18197008.59
	0:08:20	0:08:20	75.6	79.1	67.1		8:41:10 PM	36307805.48
	0:08:30	0:08:30	79.5	83.1	70.2		8:41:20 PM	89125093.81
	0:08:40	0:08:40	65.5	70.2	62.4		8:41:30 PM	3548133.892
	0:08:50	0:08:50	64.7	66.9	63.2		8:41:40 PM	2951209.227
	0:09:00	0:09:00	65.5	66.5	64.5		8:41:50 PM	3548133.892
	0:09:10	0:09:10	62.1	65.9	58.4		8:42:00 PM	1621810.097
	0:09:20	0:09:20	65.3	67.8	62.7		8:42:10 PM	3388441.561
	0:09:30	0:09:30	62.6	64.8	59.4		8:42:20 PM	1819700.859
	0:09:40	0:09:40	66.5	69	61.3		8:42:30 PM	4466835.922
	0:09:50 0:10:00	0:09:50 0:10:00	64.4 67.9	66.8 69.5	63 65.5		8:42:40 PM	2754228.703
	0:10:00	0:10:00	64.9	69.4	61.5		8:42:50 PM 8:43:00 PM	6165950.019 3090295.433
	0:10:10	0:10:10	60.9	63.5	56		8:43:10 PM	1230268.771
	0:10:20	0:10:30	63.1	65.6	58.7		8:43:20 PM	2041737.945
	0:10:30	0:10:40	61.6	66.1	57.8		8:43:30 PM	1445439.771
	0:10:50	0:10:50	63.1	66.4	59.1		8:43:40 PM	2041737.945
	0:11:00	0:11:00	64.4	66.2	62.2		8:43:50 PM	2754228.703
	0:11:10	0:11:10	63.2	66.4	61.6		8:44:00 PM	2089296.131
	0:11:20	0:11:20	63.8	70.1	60.5		8:44:10 PM	2398832.919
	0:11:30	0:11:30	66.9	71.2	59.3		8:44:20 PM	4897788.194
	0:11:40	0:11:40	62.4	66.6	55.5		8:44:30 PM	1737800.829
	0:11:50	0:11:50	53.6	55.6	52		8:44:40 PM	229086.7653
	0:12:00	0:12:00	53.4	54.5	51.7		8:44:50 PM	218776.1624
	0:12:10	0:12:10	58	59.7	54		8:45:00 PM	630957.3445
	0:12:20	0:12:20	63.4	67.8	59.7		8:45:10 PM	2187761.624
	0:12:30	0:12:30	65.9	69.2	62.5		8:45:20 PM	3890451.45
	0:12:40 0:12:50	0:12:40 0:12:50	62.1 58.8	63.1 62.7	61.4 53.1		8:45:30 PM	1621810.097 758577.575
	0:12:30	0:13:00	52.9	55	51.9		8:45:40 PM 8:45:50 PM	194984.46
	0:13:10	0:13:10	59.2	62.3	55		8:46:00 PM	831763.7711
	0:13:20	0:13:20	61.2	63.7	56.2		8:46:10 PM	1318256.739
	0:13:30	0:13:30	63.9	65.3	60.6		8:46:20 PM	2454708.916
	0:13:40	0:13:40	60.1	63.2	57.5		8:46:30 PM	1023292.992
	0:13:50	0:13:50	58.7	60.6	55.6		8:46:40 PM	741310.2413
	0:14:00	0:14:00	55.9	57.5	53.9		8:46:50 PM	389045.145
	0:14:10	0:14:10	76.4	82.1	56		8:47:00 PM	43651583.22
	0:14:20	0:14:20	61.7	66.1	57.9		8:47:10 PM	1479108.388
	0:14:30	0:14:30	59.4	61.3	55.8		8:47:20 PM	870963.59
	0:14:40	0:14:40	58.1	60.8	53.7		8:47:30 PM	645654.229
	0:14:50	0:14:50	62.3	65.4	58.2		8:47:40 PM	1698243.652
Study #4B	0:15:00	0:15:00 0:00:10	70.6 62.8	73.5 66.6	65.2	Start: 8:48:59 PM	8:47:50 PM 8:49:09 PM	11481536.21 1905460.718
•	0:00:10	0:00:10	63.8	66.6	61.1	End: 9:03:59 PM	8:49:19 PM	2398832.919
	0:00:20	0:00:30	67.2	70.1	61.4	Measured: 1/23/2019	8:49:29 PM	5248074.602
	0:00:40	0:00:40	64.2	68.9	59	Evening	8:49:39 PM	2630267.992
	0:00:50	0:00:50	58.1	60.1	55.4	• 0	8:49:49 PM	645654.229
	0:01:00	0:01:00	60	66.9	53.2		8:49:59 PM	1000000
	0:01:10	0:01:10	62.1	67	51.4	Baseline Noise Level	8:50:09 PM	1621810.097
	0:01:20	0:01:20	50.6	51.8	49.6	L _{eq} : 62.4	8:50:19 PM	114815.3621
	0:01:30	0:01:30	50.2	50.6	49.7	CNEL: 67.4	8:50:29 PM	104712.8548
	0:01:40	0:01:40	52.5	55.6	50		8:50:39 PM	177827.941
	0:01:50	0:01:50	57.5	59.3	55.6		8:50:49 PM	562341.3252
	0:02:00	0:02:00	62.7	64.5	59		8:50:59 PM	1862087.137
	0:02:10	0:02:10	60.2	62.3	57.8		8:51:09 PM	1047128.548
	0:02:20	0:02:20	55.9	58	52.9		8:51:19 PM	389045.145
	0:02:30	0:02:30	51.9	54.4	50.5		8:51:29 PM	154881.6619
	0:02:40	0:02:40	57.2	61.4	52.7		8:51:39 PM	524807.4602
	0:02:50 0:03:00	0:02:50	63.8 57.8	70.5	57.2 51.8		8:51:49 PM	2398832.919
	0.03.00	0:03:00	57.8	66.4	51.8		8:51:59 PM	602559.5861

0:03:10	0:03:10	56.4	59.9	52.7	8:52:09 PM	436515.8322
0:03:20	0:03:20	61.1	64.3	54	8:52:19 PM	1288249.552
0:03:30	0:03:30	60.6	62.6	55.8	8:52:29 PM	1148153.621
0:03:40	0:03:40	66.7	71.5	60.4	8:52:39 PM	4677351.413
0:03:50	0:03:50	60.5	61.7	59.5	8:52:49 PM	1122018.454
0:04:00	0:04:00	64.8	66.4	61.8	8:52:59 PM	3019951.72
0:04:10	0:04:10	60	63.4	55.1	8:53:09 PM	1000000
0:04:10	0:04:10	58.3	59.6	55.8	8:53:19 PM	676082.9754
0:04:20	0:04:30	51.9	55.8	50.3		154881.6619
			59.2		8:53:29 PM	
0:04:40	0:04:40	54.9		50.2	8:53:39 PM	309029.5433
0:04:50	0:04:50	58.1	62.5	53	8:53:49 PM	645654.229
0:05:00	0:05:00	58.8	65.2	52.8	8:53:59 PM	758577.575
0:05:10	0:05:10	62.2	65.7	57.5	8:54:09 PM	1659586.907
0:05:20	0:05:20	62.4	64	59.7	8:54:19 PM	1737800.829
0:05:30	0:05:30	57.1	60.6	53.2	8:54:29 PM	512861.384
0:05:40	0:05:40	58.9	62.4	54.1	8:54:39 PM	776247.1166
0:05:50	0:05:50	64.4	67.3	58.2	8:54:49 PM	2754228.703
0:06:00	0:06:00	63	66.1	57.1	8:54:59 PM	1995262.315
0:06:10	0:06:10	53.1	57.1	50.2	8:55:09 PM	204173.7945
0:06:20	0:06:20	51.8	54.4	50.6	8:55:19 PM	151356.1248
0:06:30	0:06:30	57.5	60.9	51.6	8:55:29 PM	562341.3252
0:06:40	0:06:40	60.8	64.3	51.4	8:55:39 PM	1202264.435
0:06:50	0:06:50	57.7	63	50.7	8:55:49 PM	588843.6554
0:07:00	0:07:00	52.9	53.8	51.1	8:55:59 PM	194984.46
0:07:10	0:07:10	49.6	51.1	48.6	8:56:09 PM	91201.08394
0:07:20	0:07:20	63.5	70.7	50.4	8:56:19 PM	2238721.139
0:07:30	0:07:30	58.2	62.8	55.4	8:56:29 PM	660693.448
0:07:40	0:07:40	57.7	62.6	50.7	8:56:39 PM	588843.6554
0:07:50	0:07:50	55.9	58.8	50.9	8:56:49 PM	389045.145
0:08:00	0:08:00	54.6	58.9	50.1	8:56:59 PM	288403.1503
0:08:10	0:08:10	58.2	59.4	56.2	8:57:09 PM	660693.448
0:08:20	0:08:20	54.4	56.9	53	8:57:19 PM	275422.8703
0:08:30	0:08:30	68.4	76.6	53.7	8:57:29 PM	6918309.709
0:08:40	0:08:40	71	76.8	64.2	8:57:39 PM	12589254.12
0:08:50	0:08:50	73	77.4	65.9	8:57:49 PM	19952623.15
0:09:00	0:09:00	62.6	66	58.4	8:57:59 PM	1819700.859
0:09:10	0:09:10	60.7	62.6	58.3	8:58:09 PM	1174897.555
0:09:20	0:09:20	73.4	77.5	60.7	8:58:19 PM	21877616.24
0:09:30	0:09:30	63.9	77.3	52	8:58:29 PM	2454708.916
		56.7	59.9	52	8:58:39 PM	
0:09:40	0:09:40 0:09:50	56.7 57.7	62.8			467735.1413
0:09:50				53.6	8:58:49 PM	588843.6554
0:10:00	0:10:00	62	64	59.3	8:58:59 PM	1584893.192
0:10:10	0:10:10	61.1	62.6	57	8:59:09 PM	1288249.552
0:10:20	0:10:20	61.1	64.3	55.1	8:59:19 PM	1288249.552
0:10:30	0:10:30	53.9	58.4	51.1	8:59:29 PM	245470.8916
0:10:40	0:10:40	60.7	67.4	55	8:59:39 PM	1174897.555
0:10:50	0:10:50	67.7	70.7	58.3	8:59:49 PM	5888436.554
0:11:00	0:11:00	58.5	61	53.4	8:59:59 PM	707945.7844
0:11:10	0:11:10	58.4	61.6	51.8	9:00:09 PM	691830.9709
0:11:20	0:11:20	51.6	54.9	49.9	9:00:19 PM	144543.9771
0:11:30	0:11:30	51.4	52.7	51	9:00:29 PM	138038.4265
0:11:40	0:11:40	51.5	53.2	49.3	9:00:39 PM	141253.7545
0:11:50	0:11:50	59.9	64.9	51.2	9:00:49 PM	977237.221
0:12:00	0:12:00	59.8	64.9	57.3	9:00:59 PM	954992.586
0:12:10	0:12:10	63.9	65.8	59.2	9:01:09 PM	2454708.916
0:12:20	0:12:20	65.9	67.8	62.7	9:01:19 PM	3890451.45
0:12:30	0:12:30	59.9	64.1	53.9	9:01:29 PM	977237.221
0:12:40	0:12:40	53	54.6	51.9	9:01:39 PM	199526.2315
0:12:50	0:12:50	57.2	61.8	52.4	9:01:49 PM	524807.4602
0:13:00	0:13:00	61.5	65.7	57.1	9:01:59 PM	1412537.545
0:13:10	0:13:10	58.8	64.1	55.6	9:02:09 PM	758577.575
0:13:20	0:13:20	53	55.6	50.4	9:02:19 PM	199526.2315
0:13:30	0:13:30	55.2	58.2	51.2	9:02:29 PM	331131.1215
0:13:40	0:13:40	58.2	61.3	53.9	9:02:39 PM	660693.448
0:13:50	0:13:50	58.8	60.8	57	9:02:49 PM	758577.575
0:14:00	0:14:00	60.7	63.1	58	9:02:59 PM	1174897.555
0:14:10	0:14:10	60.9	65.4	56.5	9:03:09 PM	1230268.771
					2.22.00	

ventura count	ιγ, οι τ			Short-Du	ıration (15-N	/linute)		r deme nock Qu
0:1	14:20	0:14:20	54.3	57.6	49.9		9:03:19 PM	269153.4804
	14:30	0:14:30	50.2	50.9	49.5		9:03:29 PM	104712.8548
0:1	14:40	0:14:40	50.1	51.6	49		9:03:39 PM	102329.2992
	14:50	0:14:50	56.1	62.5	50.9		9:03:49 PM	407380.2778
	15:00	0:15:00	56.4	62.4	50.5	Chart. 7:00:42 DM	9:03:59 PM	436515.8322
Study #4C 0:0 R5-A 0:0	00:10 00:20	0:00:10 0:00:20	60.2 65.7	61.1 68	59.4 60	Start: 7:00:42 PM End: 7:15:42 PM	7:00:52 PM	1047128.548 3715352.291
	00:30	0:00:30	67.5	73.8	61.3	Measured: 1/24/2019	7:01:02 PM 7:01:12 PM	5623413.252
	00:40	0:00:40	72.1	75.6	65.2	Evening	7:01:12 PM	16218100.97
	00:50	0:00:50	63.2	65.2	62	Evening	7:01:32 PM	2089296.131
	01:00	0:01:00	63.6	64.5	62.7		7:01:42 PM	2290867.653
0:0	01:10	0:01:10	64.4	64.9	64	Baseline Noise Level	7:01:52 PM	2754228.703
0:0	01:20	0:01:20	64.9	66	63.4	L _{eq} : 67.7	7:02:02 PM	3090295.433
0:0	01:30	0:01:30	64.3	66.1	62.8	CNEL: 72.7	7:02:12 PM	2691534.804
0:0	01:40	0:01:40	64.4	65.7	63.1		7:02:22 PM	2754228.703
	01:50	0:01:50	62.9	63.9	61.6		7:02:32 PM	1949844.6
	02:00	0:02:00	63.1	64.2	61.4		7:02:42 PM	2041737.945
	02:10	0:02:10	63.7	67.3	61.2		7:02:52 PM	2344228.815
	02:20	0:02:20	68.7	73.6	64.1		7:03:02 PM	7413102.413
	02:30	0:02:30	64	65.7	63.1		7:03:12 PM	2511886.432
	02:40	0:02:40	64.5	65.8	63		7:03:22 PM	2818382.931
	02:50 03:00	0:02:50 0:03:00	64.5 64	65.8 66.2	62.8 61.4		7:03:32 PM 7:03:42 PM	2818382.931 2511886.432
	03:10	0:03:10	66.4	67.2	65.8		7:03:42 PM	4365158.322
	03:20	0:03:20	71.4	75.2	64.4		7:04:02 PM	13803842.65
	03:30	0:03:30	64.2	65.9	62.5		7:04:12 PM	2630267.992
	03:40	0:03:40	61.7	63.8	59.8		7:04:22 PM	1479108.388
0:0	03:50	0:03:50	62.8	63.5	61.8		7:04:32 PM	1905460.718
0:0	04:00	0:04:00	61.9	63.1	60.2		7:04:42 PM	1548816.619
	04:10	0:04:10	62	62.7	61.2		7:04:52 PM	1584893.192
	04:20	0:04:20	61.2	62.4	60.2		7:05:02 PM	1318256.739
	04:30	0:04:30	62.3	65.1	60.8		7:05:12 PM	1698243.652
	04:40	0:04:40	65.3	67.9	64.2		7:05:22 PM	3388441.561
	04:50	0:04:50	65.7	68.5	62.2		7:05:32 PM	3715352.291
	05:00 05:10	0:05:00 0:05:10	63.1 67.7	66.9 68.4	61.4 66.6		7:05:42 PM 7:05:52 PM	2041737.945 5888436.554
	05:20	0:05:20	66.7	68	64.8		7:06:02 PM	4677351.413
	05:30	0:05:30	81.5	88.1	64.1		7:06:12 PM	141253754.5
	05:40	0:05:40	65.4	68.6	64		7:06:22 PM	3467368.505
0:0	05:50	0:05:50	64.5	67.6	62.2		7:06:32 PM	2818382.931
0:0	06:00	0:06:00	67.4	68	66.7		7:06:42 PM	5495408.739
0:0	06:10	0:06:10	66.3	67.8	63.8		7:06:52 PM	4265795.188
	06:20	0:06:20	63.9	65.2	62.3		7:07:02 PM	2454708.916
	06:30	0:06:30	64.6	66.2	62.5		7:07:12 PM	2884031.503
	06:40	0:06:40	61.2	62.6	59.7		7:07:22 PM	1318256.739
	06:50 07:00	0:06:50 0:07:00	59.5 62.7	61.5 68	57.7 60.6		7:07:32 PM	891250.9381 1862087.137
	07:10	0:07:10	66.3	68.3	63.5		7:07:42 PM 7:07:52 PM	4265795.188
	07:20	0:07:20	63.7	64.4	63		7:08:02 PM	2344228.815
	07:30	0:07:30	62.6	63.6	61.7		7:08:12 PM	1819700.859
	07:40	0:07:40	75.5	82.8	56.8		7:08:22 PM	35481338.92
0:0	07:50	0:07:50	67	74.5	55.7		7:08:32 PM	5011872.336
0:0	08:00	0:08:00	56	59.2	53.5		7:08:42 PM	398107.1706
	08:10	0:08:10	59.5	63.8	55.5		7:08:52 PM	891250.9381
	08:20	0:08:20	64	65.9	62		7:09:02 PM	2511886.432
	08:30	0:08:30	63.8	65.8	61.5		7:09:12 PM	2398832.919
	08:40 08:50	0:08:40 0:08:50	62.9	66 66.3	58 61.3		7:09:22 PM	1949844.6
	09:00	0:09:00	64 62.4	65.4	57.2		7:09:32 PM	2511886.432 1737800.829
	09:00	0:09:10	62.4	62.9	61.3		7:09:42 PM 7:09:52 PM	1659586.907
	09:20	0:09:20	59.9	61.6	58.2		7:10:02 PM	977237.221
	09:30	0:09:30	58.5	61.4	55.7		7:10:02 PM	707945.7844
	09:40	0:09:40	61.1	66.2	56.5		7:10:22 PM	1288249.552
	09:50	0:09:50	64.4	66.4	62.3		7:10:32 PM	2754228.703
	10:00	0:10:00	64.6	68.2	61.3		7:10:42 PM	2884031.503
0:1	10:10	0:10:10	65.5	67.8	61.5		7:10:52 PM	3548133.892
0:1	10:20	0:10:20	61.9	65.6	58.8		7:11:02 PM	1548816.619

Ventura Co	ounty, CA			Short	-Duration	(15-Minute)		Pacific Rock Qu
	0.10.20	0.10.20	(2.2	CE 4	60		7.11.12 DNA	2000206 121
	0:10:30	0:10:30	63.2	65.4	60		7:11:12 PM	2089296.131
	0:10:40	0:10:40	63.5	64.4	61.9		7:11:22 PM	2238721.139
	0:10:50	0:10:50	61.7	64.2	57.7		7:11:32 PM	1479108.388
	0:11:00	0:11:00	57.3	60	54		7:11:42 PM	537031.7964
	0:11:10	0:11:10	59.8	61.1	55.2		7:11:52 PM	954992.586
	0:11:20	0:11:20	63.5	66.8	60.1		7:12:02 PM	2238721.139
	0:11:30	0:11:30	63.2	65.6	61		7:12:12 PM	2089296.131
	0:11:40	0:11:40	64.8	67.9	61.1		7:12:22 PM	3019951.72
	0:11:50	0:11:50	61.3	63.5	59.4		7:12:32 PM	1348962.883
	0:12:00	0:12:00	61.2	64.2	57.1		7:12:42 PM	1318256.739
	0:12:10	0:12:10	60.6	61.7	58.6		7:12:52 PM	1148153.621
	0:12:20	0:12:20	63.3	65.4	58.8		7:13:02 PM	2137962.09
	0:12:30	0:12:30	64.6	66.3	62.9		7:13:12 PM	2884031.503
	0:12:40	0:12:40	64.8	67.4	62.6		7:13:22 PM	3019951.72
	0:12:50	0:12:50	67	69.1	64.6		7:13:32 PM	5011872.336
	0:13:00	0:13:00	65.9	66.9	63.9		7:13:42 PM	3890451.45
	0:13:10	0:13:10	80	84.7	63.9		7:13:52 PM	100000000
	0:13:20	0:13:20	64.9	70.2	61.8		7:14:02 PM	3090295.433
	0:13:30	0:13:30	68.3	70	65.8		7:14:12 PM	6760829.754
	0:13:40	0:13:40	64	65.9	62.3		7:14:22 PM	2511886.432
	0:13:50	0:13:50	64.2	65.3	62.8		7:14:32 PM	2630267.992
	0:14:00	0:14:00	67.4	69.4	64.2		7:14:42 PM	5495408.739
	0:14:10	0:14:10	64	66.1	60.9		7:14:52 PM	2511886.432
	0:14:20	0:14:20	61.2	64.4	58.9		7:15:02 PM	1318256.739
	0:14:30	0:14:30	66.5	68.5	64.4		7:15:12 PM	4466835.922
	0:14:40	0:14:40	62.9	65.9	60.7		7:15:22 PM	1949844.6
	0:14:50	0:14:50	58.6	61.3	57.6		7:15:32 PM	724435.9601
	0:15:00	0:15:00	62	65.1	58.2		7:15:42 PM	1584893.192
Study #6	0:00:10	0:00:10	70.4	74.4	65.4	Start: 7:27:34 PM	7:27:44 PM	10964781.96
R5-C	0:00:20	0:00:20	72.6	76.7	66.4	End: 7:42:34 PM	7:27:54 PM	18197008.59
	0:00:30	0:00:30	67.3	70.9	61.7	Measured: 1/24/2019	7:28:04 PM	5370317.964
	0:00:40	0:00:40	69.1	70.6	63.4	Evening	7:28:14 PM	8128305.162
	0:00:50	0:00:50	58.1	63.4	54.4		7:28:24 PM	645654.229
	0:01:00	0:01:00	72.1	76.6	59.7		7:28:34 PM	16218100.97
	0:01:10	0:01:10	71.9	73.3	69.3	Baseline Noise Level	7:28:44 PM	15488166.19
	0:01:20	0:01:20	69.4	71.3	62.5	L _{eq} : 69.9	7:28:54 PM	8709635.9
	0:01:30	0:01:30	68.9	74.6	61.5	CNEL: 74.9	7:29:04 PM	7762471.166
	0:01:40	0:01:40	73.6	78.1	61		7:29:14 PM	22908676.53
	0:01:50	0:01:50	67.4	74.1	57.6		7:29:24 PM	5495408.739
	0:02:00	0:02:00	72.3	75.1	61.4		7:29:34 PM	16982436.52
	0:02:10	0:02:10	66.5	69.4	60.2		7:29:44 PM	4466835.922
	0:02:20	0:02:20	60.8	67.7	51.6		7:29:54 PM	1202264.435
	0:02:30	0:02:30	67.4	72.4	54.9		7:30:04 PM	5495408.739
	0:02:40	0:02:40	67.4	72	54.5		7:30:14 PM	5495408.739
	0:02:50	0:02:50	67.5	73.2	52.9		7:30:24 PM	5623413.252
	0:03:00	0:03:00	73.9	75.9	69.5		7:30:34 PM	24547089.16
	0:03:10	0:03:10	77.3	82.4	66.9		7:30:44 PM	53703179.64
	0:03:20	0:03:20	68.6	71.9	65.7		7:30:54 PM	7244359.601
	0:03:30	0:03:30	68.7	70.2	66.6		7:31:04 PM	7413102.413
	0:03:40	0:03:40	63.6	69	54.8		7:31:14 PM	2290867.653
	0:03:50	0:03:50	63.9	66.6	56.9		7:31:24 PM	2454708.916
	0:04:00	0:04:00	66.3	69.7	58.2		7:31:34 PM	4265795.188
	0:04:10	0:04:10	71.1	75.8	65.3		7:31:44 PM	12882495.52
	0:04:20	0:04:20	69.4	71.3	64.9		7:31:54 PM	8709635.9
	0:04:30	0:04:30	74.3	77.3	71.4		7:32:04 PM	26915348.04
	0:04:40	0:04:40	71.2	75.1	64		7:32:14 PM	13182567.39
	0:04:50	0:04:50	70.5	72.2	63.9		7:32:24 PM	11220184.54
	0:05:00	0:05:00	68.3	71.1	61.4		7:32:34 PM	6760829.754
	0:05:10	0:05:10	71.1	75.8	63.7		7:32:44 PM	12882495.52
	0:05:20	0:05:20	72.9	76.7	68.4		7:32:54 PM	19498446
	0:05:30	0:05:30	70.7	76.7	61.7		7:33:04 PM	11748975.55
	0:05:40	0:05:40	72.1	76	66.2		7:33:14 PM	16218100.97
	0:05:50	0:05:50	73.2	75	71.2		7:33:24 PM	20892961.31
	0:06:00	0:06:00	71.9	76.9	67.8		7:33:34 PM	15488166.19
	0:06:10	0:06:10	70	73.7	65.9		7:33:44 PM	10000000
	0:06:20	0:06:20	70.8	72.7	67.3		7:33:54 PM	12022644.35
	0:06:30	0:06:30	64.7	70.9	53.2		7:34:04 PM	2951209.227

0:06:40	0:06:40	69.1	74.3	53.2	7:34:14 PM	8128305.162
0:06:50	0:06:50	57.9	63.8	53.9	7:34:24 PM	616595.0019
0:07:00	0:07:00	73.9	78.2	60.4	7:34:34 PM	24547089.16
0:07:10	0:07:10	69.9	74.7	61.4	7:34:44 PM	9772372.21
0:07:20	0:07:20	55.5	61.3	49.7	7:34:54 PM	354813.3892
0:07:30	0:07:30	66.2	68.7	58.4	7:35:04 PM	4168693.835
0:07:40	0:07:40	70.3	75.8	59.9	7:35:14 PM	10715193.05
0:07:50	0:07:50	64.5	71.9	50.8	7:35:24 PM	2818382.931
0:08:00	0:08:00	55	62	50	7:35:34 PM	316227.766
0:08:10	0:08:10	63.8	67.1	57.6	7:35:44 PM	2398832.919
0:08:20	0:08:20	68.3	74.5	58.5	7:35:54 PM	6760829.754
0:08:30	0:08:30	73.1	76.9	64.3	7:36:04 PM	20417379.45
0:08:40	0:08:40	72.6	77.1	67.6	7:36:14 PM	18197008.59
0:08:50	0:08:50	70.2	73.6	63.5	7:36:24 PM	10471285.48
0:09:00	0:09:00	57.3	63.5	49.1	7:36:34 PM	537031.7964
0:09:10	0:09:10	55.3	62.7	49	7:36:44 PM	338844.1561
0:09:20	0:09:20	74.9	81.1	62.7	7:36:54 PM	30902954.33
0:09:30	0:09:30	66.5	70.1	64	7:37:04 PM	4466835.922
0:09:40	0:09:40	73.2	76.2	65.2	7:37:14 PM	20892961.31
0:09:50	0:09:50	68.2	71.2	62.7	7:37:24 PM	6606934.48
0:10:00	0:10:00	72.8	78.7	58.9	7:37:34 PM	19054607.18
0:10:10	0:10:10	67.4	69.8	58.8	7:37:44 PM	5495408.739
0:10:20	0:10:20	59	64	54.4	7:37:54 PM	794328.2347
0:10:30	0:10:30	53.8	56.3	52.2	7:38:04 PM	239883.2919
0:10:40	0:10:40	72.2	77.4	56.3	7:38:14 PM	16595869.07
0:10:50	0:10:50	74.2	77.2	67.4	7:38:24 PM	26302679.92
0:11:00	0:11:00	72.8	76.7	63.5	7:38:34 PM	19054607.18
0:11:10	0:11:10	67.6	73.5	61.2	7:38:44 PM	5754399.373
0:11:20	0:11:20	70.6	76	63	7:38:54 PM	11481536.21
0:11:30	0:11:30	67.1	69.8	63.5	7:39:04 PM	5128613.84
0:11:40	0:11:40	69.7	76.1	55.7	7:39:14 PM	9332543.008
0:11:50	0:11:50	62.8	66.8	55.8	7:39:24 PM	1905460.718
0:12:00	0:12:00	68.4	75.1	55.6	7:39:34 PM	6918309.709
0:12:10	0:12:10	68	71.3	59.9	7:39:44 PM	6309573.445
0:12:20	0:12:20	55.3	59.9	52	7:39:54 PM	338844.1561
0:12:30	0:12:30	73.1	78	56.2	7:40:04 PM	20417379.45
0:12:40	0:12:40	68.9	70.5	65.7	7:40:14 PM	7762471.166
0:12:50	0:12:50	58.9	65.7	53.1	7:40:24 PM	776247.1166
0:13:00	0:13:00	67	69.9	56.5	7:40:34 PM	5011872.336
0:13:10	0:13:10	69.4	75	56.9	7:40:44 PM	8709635.9
0:13:20	0:13:20	51.6	56.8	48.4	7:40:54 PM	144543.9771
0:13:30	0:13:30	59.3	66	49.3	7:41:04 PM	851138.0382
0:13:40	0:13:40	70.3	74.2	63.7	7:41:14 PM	10715193.05
0:13:50	0:13:50	69.4	71.8	62.1	7:41:24 PM	8709635.9
0:14:00	0:14:00	56.7	62	52.8	7:41:34 PM	467735.1413
0:14:10	0:14:10	64.5	69.1	54.2	7:41:44 PM	2818382.931
0:14:20	0:14:20	51.9	55.2	49.5	7:41:54 PM	154881.6619
0:14:30	0:14:30	67.3	72.7	55.2	7:42:04 PM	5370317.964
0:14:40	0:14:40	55.5	62.9	49.1	7:42:14 PM	354813.3892
0:14:50	0:14:50	55.9	64.2	49.3	7:42:24 PM	389045.145
0:15:00	0:15:00	73	78.4	64.2	7:42:34 PM	19952623.15

Noise and Groundborne Vibration Impact Assessment	LU10-0003 Modification Application Pacific Rock Quarry
	APPENDIX D
NON-TRANSPORTATION SOURCES – NOI	SE IMPACT DETERMINATION

Insertion Loss Calculations @ R2 + R3

Mountain Range/Topography Barrier Noise Attenuation

LU10-003 CUP Modification Pacific Rock Quarry

Insertion Loss Calculations @ Receptors 2 (R2) & 3 (R3)

Insertion Loss (IL) Equation = $5dB + 20log((\sqrt{2pN})/tanh(\sqrt{2pN}))dB$

Source: Center for Transportation Research's Design Guide for Highway Noise Barriers (2003)

Caltrans Technical Noise Supplement offers the following guidance (Caltrans, 2013):

"Given the same site cross section, distance between source and receiver, and barrier height, a berm allows greater barrier attenuation than the thin screen (wedge), such as a soundwall. In general the actual extra attenuation associated with a berm is somewhere between 1 and 3 dBA."

Because the intervening mountain range is a large earthen mass (similar to an earthen berm), an additional -3 dBA of noise attenuation is assumed.

Fresnel Number (N): $((a + b - 1)f)/c_0$

Note: Fresnel number (N) is a nondimensional measure of how much farther the sound must travel as a result of the barrier.

- 1 The original length of the direct path from source to receiver (ft.)
- a Path length from barrier to source (ft.)
- b Path length from barrier to receiver (ft.)
- f Equipment sound frequency in hertz (Hz)
- c₀ Speed of sound propagation in air (approximately 1,100 ft./sec.)

Receptor & Equipment Source Elevation Data

	940	feet (amsl)	(approximate elevation of the lowest intervening mountain peak between closest excavation area and Receptors 2 (R2) and 3 (R3))
	865	feet (amsl)	(approximate elevation of Receptors 2 (R2) and 3 (R3))
	830	feet	(approximate distance between closest/lowest intervening mountain peak and Receptor 2 (R2) and 3 (R3))
	875	feet (amsl)	(approximate elevation of the excavation area closest to Receptor 2 (R2) and 3 (R3))
	400	feet	(approximate distance between closest/lowest intervening mountain peak and closest excavation area)
1		٦.	

True Distances

ĺ	833.4	feet	(straight line distance between lowest intervening mountain ridge and Receptor 2 (R2) and 3 (R3))
	405.2	feet	(straight line distance between lowest intervening mountain ridge and excavation area closest to Receptor 2 (R2) and 3 (R3))

Project Results

ject nesuits						
1	1,238.63	feet	(total true distance between closest excavation/equipment area(s) and R2/R3)			
a	405.33	feet	(direct distance between the closest/lowest mountain peak and top of excavation equipment)			
b	833.40	feet	(direct distance between the closest/lowest mountain peak and R2/R3)			
f	2,000.00	hertz	(2,000 is appropriate for crushing/screening, conservatively applied to mobile mining equipment)			
Fresnel Number (N	0.16					
Estimated Insertion		land the contract of the contr				
Loss '	10.4	dBA reduction @ R2/R3 due to intervening mountain range				

Footnotes:

Note - Mining equipment (e.g., loaders, excavators, dozers, etc.) height is estimated to be 8-feet above the ground surface. Receiver/receptor height is estimated to be 5-feet above the ground surface.

amsl = above mean sea level (feet). Elevations were estimated using topographic data provided by Pacific Rock.

A - Per Caltrans *Technical Noise Supplement* (2013) guidance referenced above, an additional -3 dB of noise attenuation is assumed due to mountain range being the equivalent of an "earthen berm" as opposed to a hard surfaced soundwall.

LU10-003 CUP Modification Pacific Rock Quarry

Facility/Onsite Noise Impact Calculations

Excavation Equipment (Mobile Sources) N	cavation Equipment (Mobile Sources) Noise Reference Data								
Equipment	L _{max} at 50-feet ^A	Usage Factor (%) B	Adjusted L _{eq} 1H at 50-feet	Source of Data					
Front-End Loader	80	0.33	75.2	Equipment noise data sourced from the Federal Highway Administration's (FHWA's) Roadway Construction Noise Model and Ventura County's Construction Noise Threshold Criteria and Contruction. Plan. Usage factor (UF) is "the percentage of time during the work period that the equipment is operating under full load or near full power". Usage factors are based on the default					
Dozer (Bulldozer)	85	0.33	80.2						
Excavator	85	0.33	80.2	equipment specific usage factors from FHWA's <i>Roadway Construction Noise Model</i> multiplied by an efficiency factor. An efficiency factor of 83% (50 minutes/hour) is utilized to account for					
Rock Drill	85	0.05	72.0	operator inefficiencies and breaks. Rock drill and water truck efficiencies are assumed to be 25% (15 minutes/hour) due to their less frequent and shorter activity cycles.					
Water Truck	94	0.10	84.0						
Mobile Source Noise Levels (L _{eq} 1H):	95.5	dBA	87.1	dBA					

Footnotes

- A L_{max} noise levels for mobile equipment are defaults from the FHWA's Roadway Construction Noise Model . Water truck L_{max} taken from Ventura County's Construction Noise Threshold Criteria and Control Plan.
- B Default usage factors (UF %) taken from the FHWA's *Roadway Construction Noise Model*. Default UF's % are multiplied by an efficiency factor to account for operator inefficiencies and breaks.

 Front-End Loader = 40%, Dozer (Bulldozer) = 40%, Excavator = 40%, Rock Drill = 20%, Water Truck = 40% (utilized "Dump Truck" UF %).

Unmitigated Mobile Source Noise Levels @ Facility Receptors (L _{eq} 1H)									
Receptor ^A	Auchient Deutime Neier	Nearest Mining/Facility Boundary			Nearest Mining Area with Line-of-Sight (LoS) ^F				
	Ambient Daytime Noise Level (dBA) ^C	Distance Nearest Mine Boundary to Receptor (ft.) ^B	Noise Attenuation due to Topography (dBA) ^E	Mobile Sources Noise with Attenuation (dBA) ^{D, E}	Distance Nearest Mining Area with Direct Line-of-Sight (LoS) to Receptor (ft.)	Noise Attenuation due to Topography (dBA)	Mobile Sources Noise with Attenuation (dBA) ^D		
Receptor 1 (R1)	41.6	1,160	0	59.8	1,160	0	59.8		
Receptor 2-A (R2-A)	44.8	1,161	-10	49.8					
Receptor 2-B (R2-B)	44.8	1,194	-10	49.5	1,652	0	56.7		
Receptor 2-C (R2-C)	44.8	943	-10	51.6					
Receptor 3 (R3)	44.8	390	-10	59.2					

- A Please see Figure 2 which shows the location of Receptors R1, R2 and R3. R2 receptors (R2-A, R2-B and R2-C) collectively represent residential properties in the Dos Vientos community in Newbury Park.
- B Distances (feet) between receptors and closest excavation boundaries were estimated using Google Earth (see Figure 2).
- C Ambient measurements were collected at Receptors R1 and R2/R3 on 12/20/2018 and 12/21/2018. Please see Appendix C for more detail.
- D L_{eg}/L_{max} = Total Equipment L_{eg}/L_{max} @ 50-feet 20*log(D/50). D = distance between source and receptor. (Source: Ventura County's Construction Noise Threshold and Control Plan and FHWA's Roadway Construction Noise Model).
- E As shown on Figures 4A, 4B, and 4C, none of the residences that comprise Receptor 2 (R2) or the portion of the Powerline hiking trail represented by Receptor 3 (R3) will have direct line-of-sight to mobile equipment sources operating within the mining areas closest to each receptor.

 Therefore, due to intervening mountain ranges blocking line-of-sight between noise sources (i.e., mobile mining equipment) and receptors, an additional -10 dBA of noise attenuation is assumed at Receptors 2 (R2) and 3 (R3).
 - See the noise barrier insertion loss calculations (Appendix D) for more detail. Based on the intervening topography, -10 dBA of sound attenuation represents a conservative estimate of noise attenuation provided by the mountain ridge.
- F As shown on Figures 4A and 4C, Receptors 2-A (R2-A), 2-C (R2-C) and 3 (R3) do not have a direct line-of-sight to any of the expanded mining boundaries. Therefore, worst case noise impacts will occur when mobile equipment is operating at the nearest mining boundary (see previous calculations) with a -10 dBA attenuation assumed due to the intervening topography. However, for Receptor 2-B (R2-B) there are three (3) areas within the expanded mining boundary where this receptor will have direct line-of-sight to operating mobile equipment (e.g., loaders, excavators, water truck, etc.), and therefore no noise attenuation can be assumed. Please see Figure 4B and Figure 5 which show the three (3) mining areas where Receptor 2-B (R2-B) will have direct line-of-sight to operating mining equipment, the closest of which is approximately 1,652-feet away. Mobile equipment operating in these areas will produce the worst case noise impacts at Receptor 2-B (R2-B), and are therefore analyzed to determine the significance of noise impacts at this receptor.

Unmitigated Noise Impacts with Line-of-Sight/Attenuation

Facility/Onsite Noise Impact Calculations @ Receptor 2-B (R2-B)

Excavation (i.e., mobile) Equipment Noise Refer	cavation (i.e., mobile) Equipment Noise Reference Data								
Equipment	L _{max} at 50-feet ^A	Usage Factor (%) B	Adjusted L _{eq} 1H at 50-feet	Source of Data					
Front-End Loader	80	0.33	75.2	Equipment noise data sourced from the Federal Highway Administration's (FHWA's)Roadway Construction Noise Model and Ventura County's Construction Noise Threshold Criteria and Control					
Dozer (Bulldozer)	85	0.33	80.2	Plan . Usage factor (UF) is "the percentage of time during the work period that the equipment is					
Excavator	85	0.33	80.2	operating under full load or near full power". Usage factors are based on the default equipment specific usage factors from FHWA's Roadway Construction Noise Model multiplied by an efficiency					
Rock Drill	85	0.05	72.0	factor. An efficiency factor of 83% (50 minutes/hour) is utilized to account for operator inefficiencies and breaks. Rock drill and water truck efficiencies are assumed to be 25% (15					
Water Truck	94	0.10	84.0	minutes/hour) due to their less frequent and shorter activity cycles.					
Mobile Source Noise Levels - L _{eq} 1H (dBA):	95.5		87.1						

Aggregate + Recycle Plant Equipment (Stationary Source) Noise Reference Data							
Emiliana	Measured L _{eq} at	Source of Data					
Equipment	50-feet ^c	Source of Data					
Recycle Plant		The existing Aggregate Plant and the proposed Recycle Plant noise levels based on field measurements of rock crushing/recycling activities from a previous Sespe noise study completed in Otay Mesa, California (Sespe, 2020). This reference data is a conservative representation of Pacific Rock's existing and					
Aggregate Plant	84.1	(Sespe, 2020). This reference data is a conservative representation of Pacific Rock's existing and proposed operations. See Appendix B for relevant equipment measurement data and additional explanation from the Sespe's 2020 study.					

necycle, r	ASSI CEUTE I IUITE IVOIS	
Recycle Plant Noise @ 50-feet:	84.1	dBA
Aggregate Plant Noise @ 50-feet:	84.1	dBA
Distance (ft.) from R2-B to Recycle Plant ^B :	2,688	feet
Distance (ft.) from R2-B to Aggregate Plant ^B :	2,781	feet
Assumed LoS Attenuation D:	-10	dba
Recycle Plant Noise Level @ R2-B:	39.5	dBA

Aggregate Plant Noise Level @ R2-B:

Total Stationary Source Noise @ R2-B:

Recycle/Aggregate Plant Noise @ R2-R

39.2

Aı	nbient Noise Level @ R2	-В
Measured Ambient Noise Level (Daytime):	44.8	dBA

Unmitigated Noise Propagation Calculations @ Receptor 2-B (R2-B)

	Receptor 2-A ^E	Receptor 2-B G	Receptor 2-C E	1
Excavation Noise @ 50-feet =		87.1		dBA
Distance to LoS-A = F		1,652		feet
Peak Noise Level (L _{eq} 1H) at LoS-A =		57.1		dBA
Distance to LoS-B = F		2,486		feet
Peak Noise Level (L _{eq} 1H) at LoS-B =		54.0		dBA
Distance to LoS-C = F		3,528		feet
Peak Noise Level (L _{eq} 1H) at LoS-C =		51.8		dBA

dBA

Footnotes:

- A L_{max} noise levels for equipment are defaults from the FHWA's Roadway Construction Noise Model . Water truck L_{max} taken from Ventura County's Construction Noise Threshold Criteria and Control Plan.
- B Default usage factors (UF %) taken from the FHWA's Roadway Construction Noise Model. Default UF's % are multiplied by an efficiency factor to account for operator inefficiencies and breaks. Front-End Loader = 40%, Dozer (Bulldozer) = 40%, Excavator = 40%, Rock Drill = 20%, Water Truck = 40% (utilized "Dump Truck" UF %).
- C Aggregate and Recycle Plant Lea noise levels at 50-feet based on field measurements of a rock crushing/aggregate processing plant from a previous Sespe noise study conducted in Otay Mesa, California (Sespe, 2020). See Appendix B more detail.
- D Due to intervening mountain ranges/excavation pit walls blocking line-of-sight between Receptor 2-B (R2-B) and the Aggregate Plant and Recycle Plant locations, an additional -10 dBA of noise attenuation is assumed.
- E Please see Figure 5 which displays the LoS areas and associated distances in relation to Receptor 2-8 (R2-B). Receptor 2-8 (see Figure 4A) and 2-C (see Figure 4C) do not have line-of-sight to the areas designated as LoS-A, LoS-B or LoS-C, and therefore calculations are not shown (see previous sheet).
- F Distances (feet) between R2-B and closest line-of-sight (LoS) areas estimated using Google Earth (see Figure 5).
- G Since Line-of-Sight Area A (LoS-A) is the visible mining area (i.e. has line-of-sight) nearest to Receptor 2-B, mining in LoS-A will result in the worst case noise impacts to receptor R2-B and is therefore utilized to determine the significance of Facility noise impacts at this receptor.

PA01_Noise Calcs_Nov 2020_v3.xlsx

Sespe Consulting, Inc.

Facility/Onsite (Non-Transportation) Noise Impact Calculations

Unmitigated Noise Impacts

Facility/Onsite Noise Impact Calculations

Excavation Equipment (Mobile Sources) Noise Reference Data								
Fruitament	L _{max} at	Usage Factor	Adjusted L _{eq} 1H at	Source of Data				
Equipment	50-feet ^A	(%) ^B	50-feet	Source of Data				
Front-End Loader	80	0.33	75.2	Entertain the Entertain the Entertain terms of the Market and Market and Control of the Market a				
Dozer (Bulldozer)	85	0.33	80.2	Equipment noise data sourced from the Federal Highway Administration's (FHWA's) Roadway Construction Noise Model and Ventura County's Construction Noise Threshold Criteria and Control Plan. Usage factor (UF) is "the percentage of time during the work period that the equipment is operating under full load or near full power". Usage factors are based on the default equipment specific usage factors from FHWA's Roadway				
Excavator	85	0.33	80.2	Construction Noise Model multiplied by an efficiency factor. An efficiencies are assumed				
Rock Drill	85	0.05	72.0	to be 25% (15 minutes/hour) due to their less frequent and shorter activity cycles.				
Water Truck	94	0.10	84.0					
Mobile Source Noise Levels (L _{eq} 1H):	95.5	dBA	87.1	dBA				

Aggregate + Recycle Plant Equipment (Stationary Source) Noise Reference Data						
Equipment	Measured L _{eq} at	Source of Data				
Equipment	50-feet ^c	Source of Data				
Recycle Plant	84.1	The existing Aggregate Plant and the proposed Recycle Plant noise levels based on field measurements of rock crushing/recycling activities from a previous Sespe noise study completed in Otay Mesa, California (Sespe, 2020).				
Aggregate Plant	0.4.1	This reference data is a conservative representation of Pacific Rock's existing and proposed operations. See Appendix B for relevant equipment measurement data and additional explanation from the Sespe's 2020 study.				

Footnotes

- A L_{max} noise levels for mobile equipment are defaults from the FHWA's Roadway Construction Noise Model . Water truck L_{max} taken from Ventura County's Construction Noise Threshold Criteria and Control Plan.
- B Default usage factors (UF %) taken from the FHWA's Roadway Construction Noise Model . Default UF's % are multiplied by an efficiency factor to account for operator inefficiencies and breaks.
- Front-End Loader = 40%, Dozer (Bulldozer) = 40%, Excavator = 40%, Rock Drill = 20%, Water Truck = 40% (utilized "Dump Truck" UF %).
- C Existing Aggregate Plant and proposed Recycle Plant Lea noise level at 50-feet is based on field measurements of a rock crushing/processing plant from a previous Sespe noise study completed in Otay Mesa, California (Sespe, 2020). See Appendix B for relevant excerpt and source measurement data from Sespe's 2020 study.

Unmitigated Onsite Noise Levels @ Facility Receptors (L _{eq} 1H)										
Receptor ^A Ambient Daytime Noise Level (dBA) ^c	Audian Barina Nain Land	Mobile Sour			Stationary Sou	Project Impacts & Significance Determination				
	Distance Mobile Sources to Receptor (ft.) ^B	Mobile Sources Noise with Attenuation (dBA) D, E, H	Distance to Existing Aggregate Plant to Receptor (ft.) B	Existing Aggregate Plant Noise with Attenuation (dBA) D, E	Distance to Proposed Recycle Plant to Receptor (ft.) B	Proposed Recycle Plant Noise with Attenuation (dBA) D, E	Total Project Noise Level @ Receptor (dBA) F	Significance Threshold (dBA) G	Significant?	
Receptor 1 (R1)	41.6	1,160	59.8	2,474	50.2	1,833	52.8	61.0	55	Yes
Receptor 2-A (R2-A)	44.8	1,161	49.8	2,728	39.4	2,547	40.0	51.6	55	No
Receptor 2-B (R2-B)	44.8	1,652	56.7	2,781	39.2	2,688	39.5	57.1	55	Yes
Receptor 2-C (R2-C)	44.8	943	51.6	2,730	39.4	2,580	39.8	52.8	55	No
Receptor 3 (R3)	44.8	390	59.2	2,201	41.2	1,955	42.3	59.5	55	Yes

- A Please see Figure 2 which shows the location of Receptors R1, R2 and R3. R2 receptors (R2-A, R2-B and R2-C) collectively represent residential properties in the Dos Vientos community in Newbury Park.
- B Distances (feet) between receptors and closest excavation boundaries/line-of-sight areas as well as the stationary Aggregate Plant and potential Recycle Plant locations estimated using Google Earth (see Figure 2 and 5).
- C Ambient measurements were collected at Receptors R1 and R2/R3 on 12/20/2018 and 12/21/2018. Please see Appendix C for more detail.
- D Lon/Lmay = Total Equipment Lon/Lmay @ 50-feet 20*log(D/50). D = distance between source and receptor. (Source: Ventura County's Construction Noise Threshold and Control Plan and FHWA's Roadway Construction Noise Model).
- E Due to intervening mountain ranges blocking line-of-sight between noise sources (i.e., mining equipment, recycle plant) and receptors, an additional -10 dBA of noise attenuation is assumed. Specifically, none of the Facility receptors to the east (R2 and R3) have a direct
- line-of-sight to the existing Aggregate Plant or proposed Recycle Plant due to its proposed location within the bottom of the existing mine pit. Additionally, the intervening mountain range blocks line-of-sight between excavation equipment and Receptors R2-A (Figure 4A) as well as R2-C and R-3 (Figure 4C).

 See the noise barrier insertion loss calculations (Appendix D) for more detail. Based on the intervening topography, -10 dBA of sound attenuation represents a conservative estimate. Conservatively, no attenuation was assumed at Receptor 1 (R1) as portions of this receptor may have an unobstructed view of both the existing Aggregate Plant and proposed Recycle Plant.
- F Total Project noise levels (L_{eq}1H) at each receptor represents the calculated Facility noise level (i.e., operating mobile and stationary equipment) added to the measured ambient noise level. This represents the total unmitigated noise level (L_{eq}1H, dBA) experienced at receptors as a result of the Project. Please note, these Project noise levels take into account applicable line-of-sight attenuation.
- G Because excavation operations will continue to occur during daytime hours only (7:00 a.m. 4:00 p.m.), only the daytime significance thresholds are utilized to determine the significance of noise impacts at Facility receptors.

 Ventura County General Plan Noise Element has a daytime (6:00 a.m. 7:00 p.m.) significance threshold of 55 L₀₀1H dBA.
- H As discussed previously, there are areas within the expanded mining boundary where Receptor 2-B (R2-B) will have a direct line-of-sight to mobile equipment (e.g., loaders, excavators, water truck, etc.) within the expanded mine areas, and therefore no noise attenuation can be assumed.

 Please see Figure 4B and Figure 5 which show the three (3) mining areas where Receptor 2-B (R2-B) will have direct line-of-sight to operating mining equipment, the closest of which is approximately 1,652-feet away.

 Mobile equipment operating in these areas will produce the worst case noise impacts at Receptor 2-B (R2-B), and are therefore analyzed to determine the significance of noise impacts at this receptor.

Mitigation Measure NO-2 - Mitigated Noise Impacts

Mitigated Noise Levels at Impacted Receptors

Equipment	Dominant Noise Components ^A	Unmitigated L _{eq} @ 50-feet (dBA)	Noise Component to Mitigated ^{B, C}	Control Techniques ^{B, C}	Probable Noise Reduction (dBA) ^c	Mitigated L _{eq} 1H @ 50-feet (dBA) ^D	$L_{avg} 10^{(X/10)}$
Front-End Loader	E, C, F, I, H	75.2	Exhaust (E)	Install improved muffler	-10	65.2	3320000.
Dozer (Bulldozer)	E, C, F, I, H	80.2	Exhaust (E)	Install improved muffler	-10	70.2	10498761.
Excavator (Shovel)	E, C, F, I, H, W	80.2	Exhaust (E)	Install improved muffler	-10	70.2	10498761.
Rock Drill	W, E, P	72.0	Exhaust (E)	Install improved muffler	-5	67.0	5000000.
Water Truck	W, E, C, F, I, T	84.0	Exhaust (E)	Install improved muffler	-5	79.0	79432823.
Total Mitigated Excavation Noise Level (L _{eq} 1H):							80.4

<u>Footnotes</u>

- A Ranked noisy components. C = casing, E = exhaust, F = fan, H = hydraulics, I = intake air, P = pneumatic exhaust, T = transmission, W = work tool. These represent the equipment components that can be controlled/altered to reduce reduce the overall noise level generated by the equipment. (Sources: Ventura County's Construction Noise Threshold Criteria and Control Plan, EPA's Noise from Construction Equipment and Operations, Building Equipment, and Home Appliances).
- B Ventura County's Construction Noise Threshold Criteria and Control Plan has unmitigated and mitigated noise levels for the equipment shown at 50-feet. Mitigated noise levels are the "estimated level obtainable by quieter methods or equipment and implementing feasible noise control." These can be achieved by controlling the noisy equipment components (e.g., the exhaust).
- C The EPA's *Noise from Construction Equipment and Operations, Building Equipment, and Home Appliances* notes that installation of an "improved muffler" on each equipment's "exhaust" would result in a "probable noise reduction" of -10 dBA.

 Conservatively, this NVIA assumes this control measure would achieve only a -5 dBA noise reduction for the rock drill and water truck, as the exhaust port is not the dominant noise component. This mitigation is also presented in

 Ventura County's *Construction Noise Threshold Criteria and Control Plan*, which references the EPA's mitigated equipment noise levels. An excerpt from the EPA's guidance document is included in Appendix B.
- D Following installation of an "improved muffler" on each piece of mining equipment, the mitigated noise level (Lea) is expected to be achieved. (Source: EPA's Noise from Construction Equipment and Operations, Building Equipment, and Home Appliances).

Plant Equipment (Stationary Source) Noise Reference Data						
Equipment	Measured L _{eq} at 50-feet ^G	Source of Data				
Recycle Plant	84.1	See previous				
Aggregate Plant	84.1	sheet/Appendix B.				

Mitigated Onsite Noise Levels @ Facility Receptors (L _{eq} 1H)											
		Mitigated Mobile Source Noise Levels			Stationary Source Noise Levels				Mitigated Project Impacts & Significance Determination		
Receptor ^A	A Ambient Daytime Noise Level (dBA) ^C	Distance Mobile Source to Receptor (ft.) A, F	Mobile Source Noise with Attenuation (dBA) B, C	Distance to Existing Aggregate Plant to Receptor (ft.) ^A	Existing Aggregate Plant Noise with Attenuation (dBA) B, C	Distance to Proposed Recycle Plant to Receptor (ft.) ^A	Proposed Recycle Plant Noise with Attenuation (dBA) ^{B, C}	Total Project Noise Level @ Receptor (dBA) ^D	Significance Threshold (dBA) ^E	Significant?	
Receptor 1 (R1)	41.6	1,160	53.1	2,474	50.2	1,833	52.8	57.1	55	Yes	
Receptor 2-A (R2-A)	44.8	1,161	43.0	2,728	39.4	2,547	40.0	48.4	55	No	
Receptor 2-B (R2-B)	44.8	1,652	50.0	2,781	39.2	2,688	39.5	51.7	55	No	
Receptor 2-C (R2-C)	44.8	943	44.9	2,730	39.4	2,580	39.8	49.0	55	No	
Receptor 3 (R3)	44.8	390	52.5	2,201	41.2	1,955	42.3	53.8	55	No	

Note: Prior to mitigation, noise impacts at Residence 2A (R2-A) and Residence 2C (R2-C) were shown to be below the significance threshold due to intervening topography (see previous sheet). However, since the proposed mitigation will apply to all excavation equipment, including equipment operating near R2-A and R2-C, the mitigated noise levels at these receptors are also shown here for informational purposes.

- A Distances estimated using Google Earth (see Figure 2 & Figure 5).
- B Le_o/L_{max} = Total Equipment L_{eo}/L_{max} @ 50-feet 20*log(D/50). D = distance between source and receptor. (Source: Ventura County's Construction Noise Threshold and Control Plan and FHWA's Roadway Construction Noise Model).
- C Due to intervening mountain ranges blocking line-of-sight between noise sources (i.e., mining equipment, recycle plant) and receptors, an additional -10 dBA of noise attenuation is assumed. Specifically, none of the Facility receptors to the east (R2 and R3) have a direct line-of-sight to the existing Aggregate Plant or proposed Recycle Plant due to its proposed location within the bottom of the existing mine pit. Additionally, the intervening mountain range blocks line-of-sight between excavation equipment and Receptors R2-A (Figure 4A) as well as R2-C and R-3 (Figure 4C).
- See the noise barrier insertion loss calculations (Appendix D) for more detail. Based on the intervening topography, -10 dBA of sound attenuation was assumed at Receptor 1 (R1) as portions of this receptor may have an unobstructed view of both the existing Aggregate Plant and proposed Recycle Plant.
- D Total Project noise levels (L_{eq}1H) at each receptor represents the calculated Facility noise level (i.e., operating mobile and stationary equipment) added to the measured ambient noise level. This represents the total noise level (L_{eq}1H, dBA) experienced at receptors as a result of the Project. Please note, these Project noise levels take into account applicable line-of-sight attenuation as well as equipment mitigations (i.e., improved mufflers on mobile equipment) described above.
- E Ventura County 2040 General Plan Health and Safety Element has the daytime (6:00 a.m. 7:00 p.m.) significance threshold of 55 Lea1H dBA.
- F Since Line-of-Sight Area A (LoS-A) is the visible mining area (i.e., has line-of-sight) nearest to Receptor 2-B, mining in LoS-A will result in the worst case noise impacts to receptor R2-B and is therefore utilized to determine the significance of Facility noise impacts. Distances (feet) between R2-B and closest line-of-sight (LoS) areas estimated using Google Earth (see Figure 5).
- G Ambient measurements were collected at Receptors R1 and R2/R3 on 12/20/2018 and 12/21/2018. Please see Appendix C for more detail.

Mitigation Measure NO-4 - Mitigated Noise Impacts

Mitigated Noise Levels at Impacted Receptors

Equipment	Dominant Noise Components ^A	Unmitigated L _{eq} @ 50-feet (dBA)	Noise Component to Mitigated ^{B, C}	Control Techniques ^{B, C}	Probable Noise Reduction (dBA) ^C	Mitigated L _{eq} 1H @ 50-feet (dBA) ^D	$L_{avg} 10^{(X/10)}$
Front-End Loader	E, C, F, I, H	75.2	Exhaust (E)	Install improved muffler	-10	65.2	3320000.
Dozer (Bulldozer)	E, C, F, I, H	80.2	Exhaust (E)	Install improved muffler	-10	70.2	10498761.
Excavator (Shovel)	E, C, F, I, H, W	80.2	Exhaust (E)	Install improved muffler	-10	70.2	10498761.
Rock Drill	W, E, P	72.0	Exhaust (E)	Install improved muffler	-5	67.0	5000000.
Water Truck	W, E, C, F, I, T	84.0	Exhaust (E)	Install improved muffler	-5	79.0	79432823.
Total Mitigated Excavation Noise Level (L _{en} 1H):					80.4		

Footnotes

- A Ranked noisy components. C = casing, E = exhaust, F = fan, H = hydraulics, I = intake air, P = pneumatic exhaust, T = transmission, W = work tool. These represent the equipment components that can be controlled/altered to reduce reduce the overall noise level generated by the equipment. (Sources: Ventura County's Construction Noise Threshold Criteria and Control Plan, EPA's Noise from Construction Equipment and Operations, Building Equipment, and Home Appliances).
- B Ventura Country's Construction Noise Threshold Criteria and Control Plan has unmitigated and mitigated noise levels for the equipment shown at 50-feet. Mitigated noise levels are the "estimated level obtainable by quieter methods or equipment and implementing feasible noise control." These can be achieved by controlling the noisy equipment components (e.g., the exhaust).
- C The EPA's *Noise from Construction Equipment and Operations, Building Equipment, and Home Appliances* notes that installation of an "improved muffler" on each equipment's "exhaust" would result in a "probable noise reduction" of -10 dBA.

 Conservatively, this NVIA assumes this control measure would achieve only a -5 dBA noise reduction for the rock drill and water truck, as the exhaust port is not the dominant noise component. This mitigation is also presented in

 Ventura County's *Construction Noise Threshold Criteria and Control Plan*, which references the EPA's mitigated equipment noise levels. An excerpt from the EPA's guidance document is included in Appendix B.
- D Following installation of an "improved muffler" on each piece of mining equipment, the mitigated noise level (L eq) is expected to be achieved. (Source: EPA's Noise from Construction Equipment and Operations, Building Equipment, and Home Appliances).

Plant Equipment (Stationary Source) Noise Reference Data				
Equipment Measured L _{eq} at 50-feet G Source				
Recycle Plant	84.1	See previous		
Aggregate Plant	84.1	sheet/Appendix B.		

Mitigated Onsite Noise Levels @ Facility Receptors (L _{eq} 1H)										
		Mitigated Mobile Source Noise Levels		Stationary Source Noise Levels				Mitigated Project Impacts & Significance Determination		
Receptor ^A	Ambient Daytime Noise Level (dBA) C	Distance Mobile Source to		Distance to Existing Aggregate			-	Total Project Noise Level	Significance Threshold (dBA) ^E	Significant?
		Receptor (ft.) A, F	Attenuation (dBA) B, C	Plant to Receptor (ft.) A	with Attenuation (dBA) ^H	Plant to Receptor (ft.) A	with Attenuation (dBA) B, C	@ Receptor (dBA) D, H	Threshold (dBA)	
Receptor 1 (R1)	41.6	1,160	53.1	2,474		1,833	52.8	56.1	55	Yes
Receptor 2-A (R2-A)	44.8	1,161	43.0	2,728		2,547	40.0	47.8	55	No
Receptor 2-B (R2-B)	44.8	1,652	50.0	2,781		2,688	39.5	51.4	55	No
Receptor 2-C (R2-C)	44.8	943	44.9	2,730		2,580	39.8	48.5	55	No
Receptor 3 (R3)	44.8	390	52.5	2,201		1,955	42.3	53.5	55	No

Note: Prior to mitigation, noise impacts at Receptor 2 (R2) and Receptor 3 (R3) were shown to be below the significance threshold due to intervening topography (see previous sheet). However, since the proposed mitigation will apply to all excavation equipment, including stationary and mobile equipment operating near R2 and R3, the mitigated noise levels at these receptors are also shown here for informational purposes.

- A Distances estimated using Google Earth (see Figure 2 & Figure 5).
- B L_{eq}/L_{max} = Total Equipment L_{eq}/L_{max} @ 50-feet 20*log(D/50). D = distance between source and receptor. (Source: Ventura County's Construction Noise Threshold and Control Plan and FHWA's Roadway Construction Noise Model).
- C Due to intervening mountain ranges blocking line-of-sight between noise sources (i.e., mining equipment, recycle plant) and receptors, an additional -10 dBA of noise attenuation is assumed. Specifically, none of the Facility receptors to the east (R2 and R3) have a direct line-of-sight between noise sources (i.e., mining equipment, recycle plant) as R2-C and R-3 (Figure 4C).

 | Comparison of the existing Aggregate Plant or proposed Recycle Plant due to its proposed location within the bottom of the existing mine pit. Additionally, the intervening mountain range blocks line-of-sight between excavation equipment and Receptors R2-A (Figure 4A) as well as R2-C and R-3 (Figure 4C).
- See the noise barrier insertion loss calculations (Appendix D) for more detail. Based on the intervening topography, -10 dBA of sound attenuation was assumed at Receptor 1 (R1) as portions of this receptor may have an unobstructed view of both the existing Aggregate Plant and proposed Recycle Plant.
- D Total Project noise levels (Leq1H) at each receptor represents the calculated Facility noise level (i.e., operating mobile and stationary equipment) added to the measured ambient noise level. This represents the total noise level (Leq1H, dBA) experienced at receptors
- as a result of the Project. Please note, these Project noise levels take into account applicable line-of-sight attenuation as well as mobile equipment mitigations (i.e., improved mufflers on mobile equipment) and stationary equipment mitigations (i.e., no simultaneous operation of processing equipment).
- E Ventura County 2040 General Plan Health and Safety Element has the daytime (6:00 a.m. 7:00 p.m.) significance threshold of 55 L_{ea}1H dBA.
- F Since Line-of-Sight Area A (LoS-A) is the visible mining area (i.e., has line-of-sight) nearest to Receptor 2-B, mining in LoS-A will result in the worst case noise impacts to receptor R2-B and is therefore utilized to determine the significance of Facility noise impacts. Distances (feet) between R2-B and closest line-of-sight (LoS) areas estimated using Google Earth (see Figure 5).
- G Ambient measurements were collected at Receptors R1 and R2/R3 on 12/20/2018 and 12/21/2018. Please see Appendix C for more detail.
- H Per recommend Mitigation Measure NO-4, the existing Aggregate Plant and proposed Recycle Plant will not operate simultaneously for any time period. As such, the noise contribution from the existing Aggregate Plant has been removed from the total Project noise impacts determined at Receptors 1 (R1), 2 (R2) and 3 (R3).

 As shown on the previous calculation sheet, the Aggregate Plant is estimated to produce less noise than the Recycle Plant at all Facility receptors. Therefore, assuming the Recycle Plant is operational but the Aggregate Plant does not operate per Mitigation Measure NO-4, produces the conservative worst-case noise impacts at Facility receptors (R1, R2 and R3). With the implementation of Mitigation Measure NO-4, impacts are less than significant at Facility receptors (R1, R2 and R3).

Mitigated Noise Levels at Impacted Receptors

Expected Decrease in Excavation Equipment	n Equipment (Mobile Sources) Noise Levels due to Mi Dominant Noise Components A	Unmitigated L _{eq} @ 50-feet (dBA)	Noise Component to Mitigated ^{B, C}	Control Techniques ^{B, C}	Probable Noise Reduction (dBA) ^C	Mitigated L _{eq} 1H @ 50-feet (dBA) D	L _{avg} 10 ^(X/10)
Front-End Loader	E, C, F, I, H	75.2	Exhaust (E)	Install improved muffler	-10	65.2	3320000.0
Dozer (Bulldozer)	E, C, F, I, H	80.2	Exhaust (E)	Install improved muffler	-10	70.2	10498761.8
Excavator (Shovel)	E, C, F, I, H, W	80.2	Exhaust (E)	Install improved muffler	-10	70.2	10498761.8
Rock Drill	W, E, P	72.0	Exhaust (E)	Install improved muffler	-5	67.0	5000000.0
Water Truck	W, E, C, F, I, T	84.0	Exhaust (E)	Install improved muffler	-5	79.0	79432823.5
					Total Mitigated Excavati	ion Noise Level (L _{eq} 1H):	80.4

Footnotes:

- A Ranked noisy components. C = casing, E = exhaust, F = fan, H = hydraulics, I = intake air, P = pneumatic exhaust, T = transmission, W = work tool. These represent the equipment components that can be controlled/altered to reduce reduce the overall noise level generated by the equipment. (Sources: Ventura County's Construction Noise Threshold Criteria and Control Plan, EPA's Noise from Construction Equipment and Operations, Building Equipment, and Home Appliances).
- B Ventura County's Construction Noise Threshold Criteria and Control Plan has unmitigated and mitigated noise levels for the equipment shown at 50-feet. Mitigated noise levels are the "estimated level obtainable by quieter methods or equipment and implementing feasible noise control." These can be achieved by controlling the noisy equipment components (e.g. the exhaust).
- C The EPA's Noise from Construction Equipment and Operations, Building Equipment, and Home Appliances notes that installation of an "improved muffler" on each equipment's "exhaust" would result in a "probable noise reduction" of -10 dBA. Conservatively, this NVIA assumes this control measure would achieve only a -5 dBA noise reduction for the rock drill and water truck, as the exhaust port is not the dominant noise component. This mitigation is also presented in Ventura County's Construction Noise Threshold Criteria and Control Plan, which references the EPA's mitigated equipment noise levels. An excerpt from the EPA's guidance document is included in Appendix B.
- D Following installation of an "improved muffler" on each piece of mining equipment, the mitigated noise level (L en) is expected to be achieved. (Source: EPA's Noise from Construction Equipment and Operations, Building Equipment, and Home Appliances).

Plant Equipment (Stationary Source) Noise Reference Data @ R1					
F	Measured L _{eq} at	Distance to R1 to	Plant Noise Level @ R1	C	
Equipment	50-feet ^G	Stationary Source (ft.) A	with Attenuation (dBA) ^C	Source of Data	
Recycle Plant	84.1	1,833	52.8	See previous sheet/Appendix B.	
Aggregate Plant	84.1	2,474		See previous sneet/Appendix i	
	Total Stationa	ry Source Noise Level @ R1:	52.8	dBA	

Ambient Noise Levels @ R1					
Receptor	Ambient Daytime Noise Level (dBA) F	Source of Data			
Receptor 1 (R1)	41.6	See Appendix C.			

Receptor 1 (R1) - Distance Propagation Calculations					
Distance Assessed ^A	Noise Level (L _{eq} 1H) @ Receptor without Attenuation (dBA) ^D	Ventura County Significance Threshold (L _{eq}) ^E	Significant		
1,160	56.1	55	Yes		
1,310	55.6	55	Yes		
1,460	55.2	55	Yes		
1,610	54.9	55	No		
1,760	54.7	55	No		
1,910	54.5	55	No		

Note: As shown above, cumulative worst-case noise impacts (i.e., Recycle Plant and mobile excavation equipment) experienced at Receptor 1 (R1) are below the County General Plan threshold when excavation equipment is operating a minimum of 1,600-feet away from R1.

Therefore, per Mitigation Measure NO-5, to ensure noise impacts are less than significant at R1, neither the proposed Recycle Plant nor the existing Aggregate Plant shall operated when excavation is occurring within 1,600-feet of R1 in accordance with recommended Mitigation Measure NO-5.

Please note, with the implementation of Mitigation Measure NO-5, worst-case noise impacts experienced at R1 when excavation is occurring at the closest mining boundary (i.e., 1,160-feet between source and receptor), noise levels experienced at R1 would be 53.4 dBA, which is below the General Plan threshold of 55 dBA. Please see Figure 8 which displays the potential mining areas less than 1,600-feet away from Receptor R1. If excavation is occurring within the area shown on Figure 8, Mitigation Measure NO-5 (i.e., no processing operations) shall be implemented.

- A Distances estimated using Google Earth (see Figure 8).
- B Leg/L_{max} = Total Equipment Leg/L_{max} @ 50-feet 20*log(D/50). D = distance between source and receptor. (Source: Ventura County's Construction Noise Threshold and Control Plan and FHWA's Roadway Construction Noise Model).
- C Per recommend Mitigation Measure NO-4, the existing Aggregate Plant and proposed Recycle Plant will not operate simultaneously for any time period. As such, the noise contribution from the existing Aggregate Plant has been removed from the total Project noise impacts determined at Receptors 1 (R1), 2 (R2) and 3 (R3). As shown on the previous calculation sheet, the Aggregate Plant is estimated to produce less noise than the Recycle Plant at the Facility receptors. Therefore, assuming the Recycle Plant is operational but the Aggregate Plant does not operate per Mitigation Measure NO-4, produces the conservative worst-case noise impacts at Facility receptors (R1, R2 and R3).
- D Total Project noise levels (Lea 1H) at each receptor represents the calculated Facility noise level (i.e. operating mobile and stationary equipment) added to the measured ambient noise level. This represents the total noise level (L eq 1H) at each receptor represents the calculated Facility noise level (i.e. operating mobile and stationary equipment) added to the measured ambient noise level. Please note, these Project noise levels take into account applicable line-of-sight attenuation as well as mobile equipment mitigations (i.e., improved mufflers on mobile equipment), stationary equipment mitigations (i.e., no simultaneous operation of processing equipment), and distance mitigations at Receptor 1 (i.e., no processing operations when excavation occurring within 1,600-feet of R1).
- E Ventura County 2040 General Plan Health and Safety Element has the daytime (6:00 a.m. 7:00 p.m.) significance threshold of 55 L ealth dBA.
- F Ambient measurements were collected at Receptors R1 and R2/R3 on 12/20/2018 and 12/21/2018. Please see Appendix C for more detail.

Noise and Groundborne Vibration Impact Assessment	LU10-0003 Modification Application Pacific Rock Quarry
	APPENDIX E
TRANSPORTATION SOURCES – NOI	SE IMPACT DETERMINATION

Noise Model Input/Setting Summary

LU10-0003 CUP Modification Pacific Rock Quarry

SoundPLAN Essential 4.0

SoundPLAN Essential 4.0 - Model Settings & Data

Noise Standards Utilized	
Noise Source	Noise Standard
Traffic/Road	Traffic Noise Model - FHWA; 1998 (TNM)
Industrial	ISO 9613-2: 1996

Calculation Settings					
Grid Noise Map					
Height above ground:	1.5	meters			
rieigiit above ground.	4.9	feet			
Grid distance:	5.0	meters			
Grid distance.	16.4	feet			
	Limit Lines				
Height above ground:	1.5	meters			
rieigiit above ground.	4.9	feet			

Environmental/Meteorological Settings				
Parameter	Setting	Unit		
Temperature	61.2	F°		
	16.2	C°		
Humidity	79	%		
Air Pressure	1014	mbar (SoundPLAN default)		

Note: Average temperature and humidity data for Oxnard/Camarillo taken from the Western Regional Climate Center (WRCC).

Receiver Settings		
Height above ground for free field receivers:	1.5	meters
rieight above ground for free field receivers.	4.9	feet
Height above ground floor for building receivers:	2	meters
Theight above ground noor for building receivers.	6.6	feet
Floor height:	3.7	meters
Floor fleight.	12.1	feet

Volume Attenuation Areas			
Туре	Description	He	ight
Wall	Soundwall along residences located on	1.8	meters
vvaii	Pleasant Valley Road & Pancho Road	6	feet
Ground Absorbption	Grass/shrubs in front of R4	1.0	Ground factor

Receptor Building Data											
Description Height											
	Facility Receptors										
		2	floors								
R1	Conejo Mountain Funeral Home	3.7	meters								
		12.0	feet								
		2	floors								
R2	Residence(s)	3.7	meters								
		12.0	feet								
	Haul Route Receptors										
		1	floors								
R4	Residence	3.7	meters								
		12.0	feet								
		1	floors								
R5	Residence(s)	3.7	meters								
		12.0	feet								

Daily Truck Trips			
Vehicle Type	Daily Loads	Daily Trips	Source
Aggregate Truck (HHD)	60	120	Condition #38, CUP 3817-3

Note: There are no proposed changes to existing daily CUP truck trip limit (i.e. 60 loads/day, 120 one-way trips/day)

Affected Roadway Attributes & Distribution of Project Trips													
Roadway	Segment Length (km)	Speed Limit (km/h)	Road Width (m)	Road Material	Project Trips/Day	% of Trips							
Howard Road (near facility)	0.77	8 (5 mph)	8	OGAC	120	100%							
Howard Road (near Receptor 3)	0.76	24 (15 mph)	8	DGAC	120	100%							
Pancho Road	1.55	48 (30 mph)	8	Average (of DGAC and PCC)	120	100%							
Pleasant Valley Road (northbound/southbound)	0.76	80 (50 mph)	24	PCC	102	85%							
Pleasant Valley Road (westbound/eastbound)	0.44	80 (50 mph)	24	PCC	18	15%							

Based on information provided by Pacific Rock, it is assumed that 85% of daily truck trips leaving the Facility will head north/south on Pleasant Valley Road toward the 101 Freeway, and the other 15% will head west/east toward the Pacific Coast Highway/Oxnard.

OGAC = open-graded asphaltic concrete

DGAC = dense-graded asphaltic concrete

PCC = Portland cement concrete

PLEASANT VALLEY ROAD (Lewis Road → Pancho Road)

Day Measured: Tuesday
Date Measured: 11/27/2018

EASTBOUND														
Time	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	Total
12:00 AM	0	23	2	0	2	0	0	0	0	0	0	0	0	27
1:00 AM	0	24	5	0	0	0	0	0	0	0	0	0	0	29
2:00 AM	0	14	1	0	1	0	0	0	0	0	0	0	0	16
3:00 AM	0	40	5	0	2	0	0	0	0	0	0	0	0	47
4:00 AM	0	217	32	0	8	0	0	0	0	0	0	0	0	257
5:00 AM	0	470	64	0	18	0	0	0	0	0	0	0	0	552
6:00 AM	1	638	115	0	36	1	0	3	0	0	0	0	0	794
7:00 AM	2	744	128	3	49	1	0	1	1	0	0	0	0	929
8:00 AM	2	527	88	2	37	1	0	2	5	0	0	0	0	664
9:00 AM	1	373	59	2	34	1	0	0	0	0	0	0	0	470
10:00 AM	0	304	69	0	33	0	0	0	2	0	0	0	0	408
11:00 AM	1	340	58	1	25	1	0	4	0	0	0	0	0	430
12:00 PM	1	461	74	2	38	3	0	3	3	0	0	0	0	585
1:00 PM	0	446	81	4	38	0	0	1	1	0	0	0	0	571
2:00 PM	1	491	77	2	40	1	0	2	1	0	0	0	0	615
3:00 PM	2	761	127	1	41	3	1	1	0	0	0	0	0	937
4:00 PM	3	779	98	0	35	0	0	3	0	0	0	0	0	918
5:00 PM	1	640	96	2	32	0	0	2	1	0	0	0	0	774
6:00 PM	3	525	60	0	27	0	0	1	0	0	0	0	0	616
7:00 PM	2	271	34	0	10	0	0	0	0	0	0	0	0	317
8:00 PM	0	235	26	0	7	0	0	0	0	0	0	0	0	268
9:00 PM	0	183	20	0	4	0	0	0	0	0	0	0	0	207
10:00 PM	0	81	7	0	0	0	0	0	0	0	0	0	0	88
11:00 PM	0	50	4	0	1	0	0	0	,	0	0	0	0	55
Totals:	20	8,637	1,330	19	518	12	1	23		0	0	0	0	10,574
% of Totals	0%	82%	13%	0%	5%	0%	0%	0%	0%	0%	0%	0%	0%	100%
	_1	0 = 1		-1	a I	_1	_		_	_		_	_	
AM Volumes	7	3,714	626	8	245	5	0	10		0	0	0	0	4,623
% AM	0%	35%	6%	0%	2%	0%	0%	0%	0%	0%	0%	0%	0%	44%
AM Peak Hour	7:00 AM	7:00 AM		7:00 AM	7:00 AM	6:00 AM		11:00 AM	8:00 AM					7:00 AM
Volume	2	744	128	3	49	1		4	5					936
PM Volumes	13	4,923	704	11	273	7	1	13	6	0	0	0	0	5,951
% PM	0%	47%	7%	0%	3%	0%	0%	0%	0%	0%	0%	0%	0%	56%
PM Peak Hour	4:00 PM	4:00 PM	3:00 PM		3:00 PM		3:00 PM		12:00 PM					3:00 PM
Volume	3	779	127	4	41	3	1	3	3					964

Direc	tional Peak Periods	AM 7:00	a.m 9:0	00 a.m.	Noon 12:	00 p.m 2	::00 p.m.	PM 4:00	p.m 6:0	00 p.m.	Off Peak Volumes		
	All Classes	Volume %			Volume		%	Volume		%	Volume		%
		1,593 ↔ 15%		1,156	\leftrightarrow	11%	1,692	\leftrightarrow	16%	6,133	\leftrightarrow	58%	

Classification Definitions

1 Motorcycles 4 Buses 7 >= 4-Axle Single Units (Med.) 10 >= 6-Axle Single Trailers (Heavy) 13 >= 7-Axle Multi-Trailers (Heavy)

PLEASANT VALLEY ROAD (Lewis Road → Pancho Road)

Day Measured: Tuesday
Date Measured: 11/27/2018

WESTBOUND														
Time	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	Total
12:00 AM	0	79	5	1	1	0	0	0	0	0	0	0	0	86
1:00 AM	0	60	4	0	0	0	0	0	0	0	0	0	0	64
2:00 AM	0	50	4	0	0	0	0	0	0	0	0	0	0	54
3:00 AM	0	13	1	0	0	0	0	0	0	0	0	0	0	14
4:00 AM	0	35	8	0	1	0	0	0	0	0	0	0	0	44
5:00 AM	0	156	20	0	6	0	0	2	0	0	0	0	0	184
6:00 AM	3	386	54	0	12	0	0	0		0	0	0	0	455
7:00 AM	1	627	74	1	21	0	0	0		0	0	0	0	724
8:00 AM	1	599	64	0	18	1	0	0	0	0	0	0	0	683
9:00 AM	1	375	35	0	13	0	0	1	0	0	0	0	0	425
10:00 AM	0	288	46	0	10	0	0	0		0	0	0	0	346
11:00 AM	2	313	48	0	13	0	0	1	0	0	0	0	0	377
12:00 PM	1	357	54	0	13	0	0	0		0	0	0	0	425
1:00 PM	1	389	55	1	15	0	0	0		0	0	0	0	462
2:00 PM	1	516	73	1	21	0	0	1	1	0	0	0	0	614
3:00 PM	4	875	120	2	33	0	0	0		0	0	0	0	1,035
4:00 PM	2	956	143	4	34	1	0	0		0	0	0	0	1,140
5:00 PM	2	986	117	2	26	0	0	0		0	0	0	0	1,133
6:00 PM	0	529	52	1	14	0	0	0		0	0	0	0	596
7:00 PM	0	203	20	0	7	0	0	0		0	0	0	0	230
8:00 PM	0	144	17	0	3	0	0	0		0	0	0	0	164
9:00 PM	0	136	10	0	1	0	0	0		0	0	0	0	147
10:00 PM	0	107	12	0	1	0	0	0		0	0	0	0	120
11:00 PM	0	78	5	0	1	0	0	0	0	0	0	0	0	84
Totals:	19	8,257	1,041	13	264	2	0	5	5	0	0	0	0	9,606
% of Totals	0%	86%	11%	0%	3%	0%	0%	0%	0%	0%	0%	0%	0%	100%
AM Volumes	8	2,981	363	2	95	1	0	4	2	0	0	0	0	3,456
% AM	0%	31%	4%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	36%
AM Peak Hour	6:00 AM	7:00 AM	7:00 AM		7:00 AM	8:00 AM		5:00 AM						7:00 AM
Volume	3.00 AIVI	7.00 AIVI	7.00 AW	7.00 AIVI	7.00 AIVI	6.00 AIVI		3.00 AIVI	10.00 AIVI					7.00 AW
PM Volumes	11	5,276	678	11	169	1	0	1	3	0	0	0		6,150
% PM	0%	55%	7%	0%	2%	0%	0%	0%	0%	0%	0%	0%	0%	64%
PM Peak Hour	3:00 PM	5:00 PM	4:00 PM		4:00 PM	4:00 PM		2:00 PM	1:00 PM					3:00 PM
Volume	3.00 F WI	986	143	4.00 F W	34	4.00 F W		2.00 F 101	1.00 1 101					1,174
voiume	4	300	143	4	34	1		1	1					1,174

Directional Peak Periods	AM 7:00 a	AM 7:00 a.m 9:00 a.m.			00 p.m 2	:00 p.m.	PM 4:00	p.m 6:0	0 p.m.	Off Peak Volumes		
All Classes	Volume %			Volume		%	Volume		%	Volume		%
	1,407	\leftrightarrow	15%	887	\leftrightarrow	9%	2,273	\leftrightarrow	24%	5,039	\leftrightarrow	52%

Classification Definitions

1 Motorcycles 4 Buses 7 >= 4-Axle Single Units (Med.) 10 >= 6-Axle Single Trailers (Heavy) 13 >= 7-Axle Multi-Trailers (Heavy)

PLEASANT VALLEY ROAD (US 101 Freeway → Pancho Road)

Day Measured: Tuesday
Date Measured: 11/27/2018

NORTHBOUND														
Time	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	Total
12:00 AM	0	47	2	0	1	0	0	1	0	0	0	0	0	51
1:00 AM	0	35	2	0	0	0	0	0	0	0	0	0	0	37
2:00 AM	0	53	1	0	2	0	0	0	0	0	0	0	0	56
3:00 AM	0	24	3	0	0	0	0	1	0	0	0	0	0	28
4:00 AM	0	89	14	0	4	0	0			0	0	0	0	107
5:00 AM	0	289	50	1	12	0	0		0	0	0	0	0	352
6:00 AM	0	469	64	0	18	0	0		0	0	0	0	0	554
7:00 AM	0	822	94	1	28	0	0		0	0	0	0	0	945
8:00 AM	0	584	80	0	25	0	0		0	0	0	0	0	690
9:00 AM	0	408	54	1	26	3	0		0	0	0	0	0	493
10:00 AM	0	423	43	3	31	0	0		1	0	0	0	0	504
11:00 AM	0	478	62	1	23	0	0		1	0	0	0	0	565
12:00 PM	0	591	60	0	29	0	0		1	0	0	0	0	684
1:00 PM	0	530	68	0	26	1	0		0	0	0	0	0	627
2:00 PM	0	654	68	0	28	0	0		0	0	0	0	0	751
3:00 PM	1	853	98	0	31	0	0			0	0	0	0	983
4:00 PM	1	987	88	0	27	0	0		1	0	0	0	0	1,104
5:00 PM	1	958	98	0	20	0	0		0	0	0	0	0	1,078
6:00 PM	0	645	43	0	28	0	0		0	0	0	0	0	718
7:00 PM	0	380	27	0	9	0	0		_	0	0	0	0	416
8:00 PM	0	226	10	0	7	0	0			0	0	0	0	243
9:00 PM	0	193	7	0	6	0	0		0	0	0	0	0	206
10:00 PM	0	103	1	0	1	0	0		1	0	0	0	0	106
11:00 PM	0	54	3	0	2	0	0		0	0	0	0	0	59
Totals:	3	-,	1,040	7	384	4	0		5	0	0	0	0	11,357
% of Totals	0%	87%	9%	0%	3%	0%	0%	0%	0%	0%	0%	0%	0%	100%
AM Volumes	0	3,721	469	7	170	3	0	10	2	0	0	0	0	4,382
% AM	0%	33%	4%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	39%
AM Peak Hour		7:00 AM	7:00 AM	#######	10:00 AM	9:00 AM		6:00 AM	10:00 AM					7:00 AM
Volume		822	94	3	31	3		3	1					957
PM Volumes	3	6,174	571	0	214	1	0	9	3	0	0	0	0	6,975
% PM	0%	54%	5%	0%	2%	0%	0%	0%	0%	0%	0%	0%	0%	61%
PM Peak Hour	3:00 PM	4:00 PM	3:00 PM		3:00 PM	1:00 PM		12:00 PM	12:00 PM					3:00 PM
Volume	1	987	98		31	1		3	1					1,122

Directional Peak Perio	ds AM 7:	00 a.m 9	:00 a.m.	Noon 12	:00 p.m 2	2:00 p.m.	PM 4:00) p.m 6:0	00 p.m.	Off Peak Volumes		
All Class	volume	Volume %				%	Volume		%	Volume		%
	1,635	1,635 ↔ 14%		1,311	\leftrightarrow	12%	2,182	\leftrightarrow	19%	6,229	\leftrightarrow	55%

Classification Definitions

1 Motorcycles 4 Buses 7 >= 4-Axle Single Units (Med.) 10 >= 6-Axle Single Trailers (Heavy) 13 >= 7-Axle Multi-Trailers (Heavy)

PLEASANT VALLEY ROAD (US 101 Freeway → Pancho Road)

Day Measured: Tuesday
Date Measured: 11/27/2018

SOUTHBOUND														
Time	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	Total
12:00 AM	0	30	1	0	2	0	0	1	0	0	0	0	0	34
1:00 AM	0	18	4	0	1	0	0	0	0	0	0	0	0	23
2:00 AM	0	16	4	0	4	0	0	0	0	0	0	0	0	24
3:00 AM	0	15	2	0	0	0	0	0	0	0	0	0	0	17
4:00 AM	0	123	18	0	6	0	0	0	0	0	0	0	0	147
5:00 AM	0	276	33	0	18	1	0	1	1	0	0	0	0	330
6:00 AM	1	540	80	0	33	0	0	0	0	0	0	0	0	654
7:00 AM	2	747	113	3	59	1	0	2	0	0	0	0	0	927
8:00 AM	1	706	99	1	39	4	1	2	0	0	0	0	0	853
9:00 AM	1	425	68	1	24	0	1	1	0	0	0	0	0	521
10:00 AM	1	328	49	0	23	2	1	1	0	0	0	0	0	405
11:00 AM	1	367	67	1	21	1	0	0	1	0	0	0	0	459
12:00 PM	1	476	78	2	33	0	0	0	0	0	0	0	0	590
1:00 PM	1	456	82	2	28	2	0	1	1	0	0	0	0	573
2:00 PM	0	496	78	1	40	1	0	2	0	0	0	0	0	618
3:00 PM	1	650	100	1	40	1	0	0	0	0	0	0	0	793
4:00 PM	1	728	107	3	59	0	0	0	0	0	0	0	0	898
5:00 PM	0	723	120	1	37	0	0	1	1	0	0	0	0	883
6:00 PM	0	516	64	0	24	0	0	0	0	0	0	0	0	604
7:00 PM	0	251	26	0	12	0	0	0	0	0	0	0	0	289
8:00 PM	0	178	25	0	8	0	0	0	0	0	0	0	0	211
9:00 PM	0	174	26	0	7	0	0	0	0	0	0	0	0	207
10:00 PM	0	120	18	0	5	0	0	0	0	0	0	0	0	143
11:00 PM	0	73	3	0	3	0	0	0	0	0	0	0	0	79
Totals:	11	8,432	1,265	16	526	13	3	12	4	0	0	0	0	10,282
% of Totals	0%	82%	12%	0%	5%	0%	0%	0%	0%	0%	0%	0%	0%	100%
AM Volumes	7	3,591	538	6	230	9	3	8	2	0	0	0	0	4,394
% AM	0%	35%	5%	0%	2%	0%	0%	0%	0%	0%	0%	0%	0%	43%
AM Peak Hour	7:00 AM	7:00 AM	7:00 AM		7:00 AM	8:00 AM	8:00 AM	7:00 AM	5:00 AM					7:00 AM
Volume	7.007.11	7.00 AW	113	3	7.00 AW	4	0.00 AIVI	7.00 AIVI	1					932
PM Volumes	4	4,841	727	10	296	4	0	4	2	0	0	0	0	5,888
% PM	0%	47%	7%	0%	3%	0%	0%	0%	0%	0%	0%	0%	0%	57%
PM Peak Hour	12:00 PM	4:00 PM	5:00 PM		4:00 PM	1:00 PM		2:00 PM	1:00 PM					3:00 PM
Volume	1	728	120	3	59	2		2	1					916
Volulile	1	, 20	120	3	33	2		2	1					510

Directional Peak Periods	AM 7:00	a.m 9:	00 a.m.	Noon 12:	Noon 12:00 p.m 2:00 p.m.			p.m 6:0	0 p.m.	Off Peak Volumes		
All Classes	Volume	Volume %				%	Volume		%	Volume		%
	1,780	\leftrightarrow	17%	1,163	\leftrightarrow	11%	1,781	\leftrightarrow	17%	5,558	\leftrightarrow	54%

Classification Definitions

1 Motorcycles 4 Buses 7 >= 4-Axle Single Units (Med.) 10 >= 6-Axle Single Trailers (Heavy) 13 >= 7-Axle Multi-Trailers (Heavy)

PANCHO ROAD (Howard Road → Pleasant Valley Road)

Day Measured: Tuesday
Date Measured: 11/27/2018

NORTHBOUND														
Time	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	Total
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:00 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	1
3:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 AM	0	1	1	0	2	0	0	0	0	0	0	0	0	4
6:00 AM	0	5	1	1	3	0	0	0	0	0	0	0	0	10
7:00 AM	0	10	2	0	5	0	0	1	1	0	0	0	0	19
8:00 AM	0	11	1	0	4	0	0	0	1	0	0	0	0	17
9:00 AM	0	8	5	0	6	0	0	2	1	0	0	0	0	22
10:00 AM	0	9	2	0	3	1	0	0	2	0	0	0	0	17
11:00 AM	1	26	4	1	5	0	1	0	2	0	0	0	0	40
12:00 PM	1	32	11	0	2	0	0	0	2	0	0	0	0	48
1:00 PM	0	20	5	0	4	0	0	0	0	0	0	0	0	29
2:00 PM	0	43	10	1	6	2	0	0	0	0	0	0	0	62
3:00 PM	0	55	13	0	10	0	0	0	0	0	0	0	0	78
4:00 PM	0	56	11	1	7	0	0	0	0	0	0	0	0	75
5:00 PM	0	19	6	0	4	0	0	0	0	0	0	0	0	29
6:00 PM	0	4	1	0	2	0	0	0	0	0	0	0	0	7
7:00 PM	0	8	1	0	1	0	0	0	0	0	0	0	0	10
8:00 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	2
9:00 PM	0	0	0	0	2	0	0	0	0	0	0	0	0	2
10:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	2
Totals:	2	311	74	4	66	3	1	4	9	0	0	0	0	474
% of Totals	0%	66%	16%	1%	14%	1%	0%	1%	2%	0%	0%	0%	0%	100%
AM Volumes	1	70	16	2	28	1	1	4	7	0	0	0	0	130
% AM	0%	15%	3%	0%	6%	0%	0%	1%	1%	0%	0%	0%	0%	27%
	11:00 AM		9:00 AM		9:00 AM		11:00 AM	9:00 AM	10:00 AM					11:00 AM
Volume	1	26	5	1	6	1	1	2	2					45
PM Volumes	1	241	58	2	38	2	0	0	2	0	0	0	0	344
% PM	0%	51%	12%	0%	8%	0%	0%	0%	0%	0%	0%	0%	0%	73%
PM Peak Hour	12:00 PM	4:00 PM	3:00 PM	2:00 PM	3:00 PM	2:00 PM			12:00 PM					3:00 PM
Volume	1	56	13	1	10	2			2					85

Directional Peak Periods	AM 7:00 a	a.m 9:0	0 a.m.	Noon 12:	Noon 12:00 p.m 2:00 p.m.			PM 4:00 p.m 6:00 p.m.			Off Peak Volumes		
All Classes	Volume		%	Volume		%	Volume		%	Volume		%	
	36	\leftrightarrow	8%	77	\leftrightarrow	16%	104	\leftrightarrow	22%	257	\leftrightarrow	54%	

Classification Definitions

1 Motorcycles 4 Buses 7 >= 4-Axle Single Units (Med.) 10 >= 6-Axle Single Trailers (Heavy) 13 >= 7-Axle Multi-Trailers (Heavy)

PANCHO ROAD (Howard Road → Pleasant Valley Road)

Day Measured: Tuesday
Date Measured: 11/27/2018

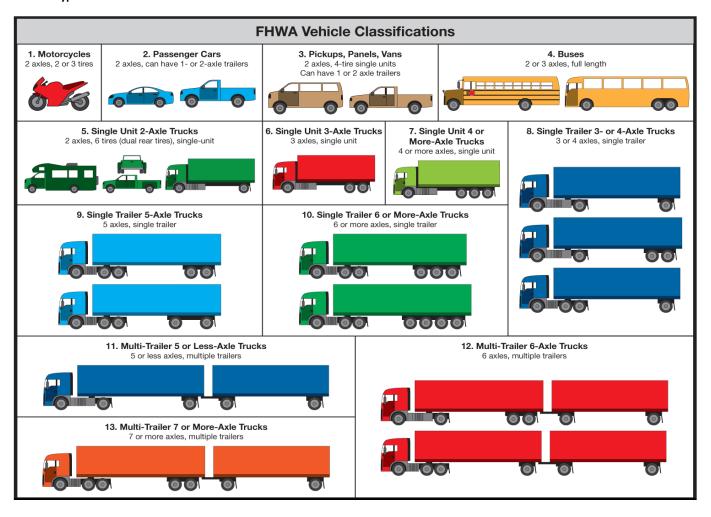
SOUTHBOUND														
Time	#1	#2	#3	#4	#5	#6	#7	#8	#9	#10	#11	#12	#13	Total
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	1
5:00 AM	0	5	2	0	0	0	0	0	0	0	0	0	0	7
6:00 AM	0	89	14	1	8	0	0	0	1	0	0	0	0	113
7:00 AM	0	17	5	0	3	0	0	1	0	0	0	0	0	26
8:00 AM	1	16	5	0	7	1	0	0	0	0	0	0	0	30
9:00 AM	2	13	2	0	5	1	0	0	1	0	0	0	0	24
10:00 AM	0	19	1	0	3	1	0	0	2	0	0	0	0	26
11:00 AM	1	27	5	0	6	0	0	0	1	0	0	0	0	40
12:00 PM	1	32	9	0	3	1	0	0	1	0	0	0	0	47
1:00 PM	0	19	5	0	7	1	0	0	0	0	0	0	0	32
2:00 PM	0	21	3	1	2	0	0	0	0	0	0	0	0	27
3:00 PM	0	26	3	0	3	0	0	0	0	0	0	0	0	32
4:00 PM	0	17	3	0	4	0	0	0	0	0	0	0	0	24
5:00 PM	0	11	4	0	2	0	0	0	0	0	0	0	0	17
6:00 PM	0	11	2	0	0	0	0	0	0	0	0	0	0	13
7:00 PM	0	6	0	0	0	0	0	0	0	0	0	0	0	6
8:00 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	2
9:00 PM	0	2	0	0	0	0	0	1	0	0	0	0	0	3
10:00 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	2
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Totals:	5	335	64	2	53	5	0	2	6	0	0	0	0	472
% of Totals	1%	71%	14%	0%	11%	1%	0%	0%	1%	0%	0%	0%	0%	100%
	-1	407	2.1	-	22				-					267
AM Volumes	4	187	34	1	32	3	0	1	5	0	0	0	0	267
% AM	1%	40%	7%	0%	7%	1%	0%	0%	1%	0%	0%	0%	0%	57%
AM Peak Hour	9:00 AM	6:00 AM	6:00 AM		6:00 AM			7:00 AM	10:00 AM					6:00 AM
Volume	2	89	14	1	8	1		1	2					118
PM Volumes	1	148	30	1	21	2	0	1	1	0	0	0	0	205
% PM	0%	31%	6%	0%	4%	0%	0%	0%	0%	0%	0%	0%	0%	43%
PM Peak Hour	12:00 PM		12:00 PM		1:00 PM			9:00 PM	12:00 PM					12:00 PM
Volume	1	32	9	1	7	1		1	1					53

Directional Peak Periods	AM 7:00 a	a.m 9:0	00 a.m.	Noon 12:	Noon 12:00 p.m 2:00 p.m.			PM 4:00 p.m 6:00 p.m.			Off Peak Volumes		
All Classes	Volume		%	Volume		%	Volume		%	Volume		%	
	56	\leftrightarrow	12%	79	\leftrightarrow	17%	41	\leftrightarrow	9%	296	\leftrightarrow	63%	

Classification Definitions

1 Motorcycles 4 Buses 7 >= 4-Axle Single Units (Med.) 10 >= 6-Axle Single Trailers (Heavy) 13 >= 7-Axle Multi-Trailers (Heavy)

Vehicle Type Visual Guide



Baseline Traffic Summary - Daytime VRPA Technologies Inc.

Existing/Ba	aseline - Daytime	(6:00 a.m 7:	00 p.m.) Traffi						ı				
Dooduusu	Commant	Divertion	Times	Ac	tual Tra	affic Counts by \			Ave	erage Ti	raffic Counts by		
Roadway	Segment	Direction	Time	Motorcycles	Buses	Automobiles	Medium Trucks	Heavy Trucks	Motorcycles	Buses	Automobiles	Medium Trucks	Heavy Trucks
			6:00 a.m.	1	0	753	37	3				HUCKS	TTUCKS
			7:00 a.m.	2	3	872	50	2	İ				
			8:00 a.m.	2	2	615	38	7	1				
			9:00 a.m.	1	2	432	35	0	1				
			10:00 a.m.	0	0	373	33	2					
			11:00 a.m.	1	1	398	26	4			520		
		eastbound	12:00 p.m.	1	2	535	41	6	1	1	628	37	3
			1:00 p.m. 2:00 p.m.	0 1	2	527 568	38 41	3	ł				
			3:00 p.m.	2	1	888	45	1	İ				
			4:00 p.m.	3	0	877	35	3	1				
			5:00 p.m.	1	2	736	32	3					
	Lewis Road →		6:00 p.m.	3	0	585	27	1					
	Pancho Road		6:00 a.m.	3	0	440	12	0	l				
			7:00 a.m. 8:00 a.m.	1	0	701 663	21 19	0	ł				
			9:00 a.m.	1	0	410	13	1	1				
			10:00 a.m.	0	0	334	10	2	1				
			11:00 a.m.	2	0	361	13	1]				
		westbound	12:00 p.m.	1	0	411	13	0	1	1	625	19	1
			1:00 p.m.	1	1	444	15	1					
			2:00 p.m.	1	1	589	21	2					
			3:00 p.m.	4	4	995	33	0					
			4:00 p.m. 5:00 p.m.	2	2	1,099 1,103	35 26	0	1				
Pleasant			6:00 p.m.	0	1	581	14	0	İ				
Valley			6:00 a.m.	0	0	533	18	3					
Road			7:00 a.m.	0	1	916	28	0]				
			8:00 a.m.	0	0	664	25	1					
			9:00 a.m.	0	1	462	29	1	Į.				
			10:00 a.m.	0	3	466	31	4					
		northbound	11:00 a.m. 12:00 p.m.	0	0	540 651	23 29	4	0	0	717	26	2
		Hortinbound	1:00 p.m.	0	0	598	27	2	Ĭ		,1,	20	_
			2:00 p.m.	0	0	722	28	1	1				
			3:00 p.m.	1	0	951	31	0	1				
			4:00 p.m.	1	0	1,075	27	1]				
			5:00 p.m.	1	0	1,056	20	1					
	US 101 →		6:00 p.m.	0	0	688	28	2					
	Pancho Road		6:00 a.m.	1 2	0	620	33	2	ł				
			7:00 a.m. 8:00 a.m.	1	3	860 805	60 44	2	ł				
			9:00 a.m.	1	1	493	25	1	İ				
			10:00 a.m.	1	0	377	26	1	1				
			11:00 a.m.	1	1	434	22	1					
		southbound	12:00 p.m.	1	2	554	33	0	1	1	636	37	1
			1:00 p.m.	1	2	538	30	2	Į.				
			2:00 p.m.	0	1	574 750	41 41	0	ł				
			3:00 p.m. 4:00 p.m.	1	3	835	59	0	ł				
			5:00 p.m.	0	1	843	37	2	1				
			6:00 p.m.	0	0	580	24	0	<u> </u>	L_			<u> </u>
			6:00 a.m.	0	1	6	3	0					
			7:00 a.m.	0	0	12	5	2					
			8:00 a.m.	0	0	12	4	1	1				
			9:00 a.m. 10:00 a.m.	0	0	13 11	6 4	3					
			11:00 a.m.	1	1	30	6	2	1				
		northbound	12:00 p.m.	1	0	43	2	2	0	0	28	5	1
			1:00 p.m.	0	0	25	4	0]				
			2:00 p.m.	0	1	53	8	0					
			3:00 p.m.	0	0	68	10	0					
Don-l			4:00 p.m.	0	1	67	7	0					
Pancho Road /	Pleasant Valley		5:00 p.m. 6:00 p.m.	0	0	25 5	2	0					
Howard	Road → Howard		6:00 p.m. 6:00 a.m.	0	1	103	8	1					
Road	Road		7:00 a.m.	0	0	22	3	1	1				
			8:00 a.m.	1	0	21	8	0	1				
			9:00 a.m.	2	0	15	6	1					
			10:00 a.m.	0	0	20	4	2					
			11:00 a.m.	1	0	32	6	1					
		southbound	12:00 p.m.	1	0	41	4	1	0	0	29	4	1
			1:00 p.m.	0	0	24	8	0	ł				
			2:00 p.m. 3:00 p.m.	0	0	24 29	3	0	l				
1			4:00 p.m.	0	0	29	4	0	1				
			5:00 p.m.	0	0	15	2	0					

Day Measured: Tuesday Date Measured: 11/27/2018

PA01_Noise Calcs_fnl.xlsx April 12, 2019

Existing/B	aseline - Evening I	Hours (7:00 p.n	n 10:00 p.m.) Traffic Data									
				Act	tual Traf	ffic Counts by V	ehicle Type		Avera	age Traf	fic Counts by V	ehicle Type	;
Roadway	Segment	Direction	Time	Motorcycles	Buses	Automobiles	Medium Trucks	Heavy Trucks	Motorcycles	Buses	Automobiles	Medium Trucks	Heavy Trucks
			7:00 p.m.	2	0	305	10	0					
		eastbound	8:00 p.m.	0	0	261	7	0	1	0	256	7	0
	Lewis Road →		9:00 p.m.	0	0	203	4	0	1				
	Pancho Road		7:00 p.m.	0	0	223	7	0					
Pleacant		westbound	8:00 p.m.	0	0	161	3	0	0	0	177	4	0
			9:00 p.m.	0	0	146	1	0					
·	alley oad US 101 →	northbound	7:00 p.m.	0	0	407	9	0					
Noau			8:00 p.m.	0	0	236	7	0	0	0	281	7	0
			9:00 p.m.	0	0	200	6	0					
	Pancho Road		7:00 p.m.	0	0	277	12	0					
		southbound	8:00 p.m.	0	0	203	8	0	0	0	227	9	0
			9:00 p.m.	0	0	200	7	0					
			7:00 p.m.	0	0	9	1	0					
	Pleasant Valley Road → Howard	northbound	8:00 p.m.	0	0	2	0	0	0	0	4	1	0
Pancho			9:00 p.m.	0	0	0	2	0					<u> </u>
Road			7:00 p.m.	0	0	6	0	0					
		southbound	8:00 p.m.	0	0	2	0	0	0	0	3	0	0
			9:00 p.m.	0	0	2	0	1]				1

Day Measured: Tuesday
Date Measured: 11/27/2018

Baseline Traffic Summary - Nighttime VRPA Technologies Inc.

LAISTING/ DO	aseline - Nighttime	(10:00 p.m	6:00 a.m.) Tra	ffic Data									
				А	ctual Traffi	c Counts by Vel	nicle Type		Ave	rage Tra	affic Counts by \	ehicle Type	
Roadway	Segment	Direction	Time	Motorcycles	Buses	Automobiles	Medium Trucks	Heavy Trucks	Motorcycles	Buses	Automobiles	Medium Trucks	Heavy Trucks
			10:00 p.m.	0	0	88	0	0					
			11:00 p.m.	0	0	54	1	0	1				
			12:00 a.m.	0	0	25	2	0	1				
			1:00 a.m.	0	0	29	0	0		0	120	4	0
		eastbound	2:00 a.m.	0	0	15	1	0	0	U	130	4	0
			3:00 a.m.	0	0	45	2	0	1				
			4:00 a.m.	0	0	249	8	0	1				
	Lewis Road →		5:00 a.m.	0	0	534	18	0	1				
	Pancho Road		10:00 p.m.	0	0	119	1	0					
			11:00 p.m.	0	0	83	1	0	1				
			12:00 a.m.	0	1	84	1	0	1				
			1:00 a.m.	0	0	64	0	0	_		90	1	0
		westbound	2:00 a.m.	0	0	54	0	0	0	0	80	1	U
			3:00 a.m.	0	0	14	0	0					
Discount			4:00 a.m.	0	0	43	1	0	1				
Pleasant			5:00 a.m.	0	0	176	6	2	1				
Valley			10:00 p.m.	0	0	104	1	1					
Road			11:00 p.m.	0	0	57	2	0	1				
			12:00 a.m.	0	0	49	1	1	1				
		or a sublished on the selection of	1:00 a.m.	0	0	37	0	0		0	0.5	2	
		northbound	2:00 a.m.	0	0	54	2	0	0	0	96	3	0
			3:00 a.m.	0	0	27	0	1	1				
			4:00 a.m.	0	0	103	4	0	1				
	US 101 →		5:00 a.m.	0	1	339	12	0	1				
	Pancho Road		10:00 p.m.	0	0	138	5	0					
			11:00 p.m.	0	0	76	3	0	1				
			12:00 a.m.	0	0	31	2	1	1				
			1:00 a.m.	0	0	22	1	0				_	
		southbound	2:00 a.m.	0	0	20	4	0	0	0	94	5	0
			3:00 a.m.	0	0	17	0	0	1				
			4:00 a.m.	0	0	141	6	0	1				
			5:00 a.m.	0	0	309	19	2	1				
			10:00 p.m.	0	0	0	0	0					
			11:00 p.m.	0	0	2	0	0]				
			12:00 a.m.	0	0	0	0	0]				
		northbound	1:00 a.m.	0	0	0	0	0	0	0	1	0	0
		normbound	2:00 a.m.	0	0	0	0	1	J	U	1	U	U
			3:00 a.m.	0	0	0	0	0]				
	Pleasant Valley		4:00 a.m.	0	0	0	0	0					
Pancho	Road → Howard		5:00 a.m.	0	0	2	2	0	<u> </u>				<u> </u>
Road	Road → Howard		10:00 p.m.	0	0	2	0	0					
	KOAU		11:00 p.m.	0	0	0	0	0]				
			12:00 a.m.	0	0	0	0	0]				
		التحديد والماهيد والما	1:00 a.m.	0	0	0	0	0			1	•	_
		southbound	2:00 a.m.	0	0	0	0	0	0	0	1	0	0
			3:00 a.m.	0	0	0	0	0]				
			4:00 a.m.	0	0	1	0	0]				
			5:00 a.m.	0	0	7	0	0	1				

Day Measured: Tuesday Date Measured: 11/27/2018

PA01_Noise Calcs_fnl.xlsx April 12, 2019 Baseline + Project Traffic Counts

Existing/Baseline - Facility	Haul Truck/Tr	affic Data							
	Daily Limits	s - CUP 3817-3			Average Tri	ps/Hour			
Parameter	Loads	One May Trin		Baseline ^A		Project ^B			
		One-Way Trip	Daytime	Evening	Nighttime	Daytime	Evening	Nighttime	
Haul Trucks	60	120	13			5	5	5	

Note: There are no proposed changes to existing daily CUP truck trip limit (i.e. 60 loads/day, 120 one-way trips/day)

- A Per the existing CUP, a maximum of 120 haul truck trips/day occur during the operating hours of 7:00 a.m. and 4:00 p.m. (9 hours total). To model baseline traffic noise impacts at haul road receptors, it is assumed that the maximum number of haul truck trips occurs (120 trips/day) spread evenly throughout each hour of the daytime operating hours (120 trips ÷ 9 hours = 13 trips/hour).
- B There are no proposed changes to existing CUP limit 120 truck trips/day. However, the Project involves limited 24 hour/day haul truck operations during special projects. To model Project traffic noise impacts at haul road receptors, it is assumed that the maximum number of haul truck trips occurs (120 trips/day) spread evenly throughout 24-hour daytime, evening, and nighttime operating hours (120 trips ÷ 24 hours = 5 trips/hour).

				BASELINEC					PROJECT ^I)	
Road Segment	Direction		Averag	e Hourly Traffic	Trip Counts			Avera	ge Hourly Traffi	c Trip Counts	
		Motorcycles	Buses	Automobiles	Medium Trucks	Heavy Trucks	Motorcycles	Buses	Automobiles	Medium Trucks	Heavy Trucks
	Northbound	0	0	28	5	14	0	0	28	5	19
Howard Road / Pancho	Southbound	0	0	29	4	14	0	0	29	4	19
Road	Eastbound										
	Westbound										
	Northbound	0	0	717	26	13	0	0	717	26	17
Diagramt Valley Dood	Southbound	1	1	636	37	12	1	1	636	37	17
Pleasant Valley Road	Eastbound	1	1	628	37	5	1	1	628	37	6
	Westbound	1	1	625	19	3	1	1	625	19	3

Evening (7:00 p.m 10:00	p.m.) - CNEL N	<u>/lodel</u>									
				BASELINEC					PROJECT ^I	D	
Road Segment	Direction		Averag	e Hourly Traffic	Trip Counts			Avera	ge Hourly Traffi	c Trip Counts	
		Motorcycles	Buses	Automobiles	Medium Trucks	Heavy Trucks	Motorcycles	Buses	Automobiles	Medium Trucks	Heavy Trucks
	Northbound	0	0	4	1	0	0	0	4	1	5
Howard Road / Pancho Road	Southbound	0	0	3	0	0	0	0	3	0	5
Road	Eastbound										
	Westbound										
	Northbound	0	0	281	7	0	0	0	281	7	4
Diagraph Valley Boad	Southbound	0	0	227	9	0	0	0	227	9	4
Pleasant Valley Road	Eastbound	1	0	256	7	0	1	0	256	7	1
	Westbound	0	0	177	4	0	0	0	177	4	1

Nighttime (10:00 p.m 7:	00 a.m.) - CNEL	Model									
				BASELINEC					PROJECT ^I)	
Road Segment	Direction		Average	e Hourly Traffic	Trip Counts			Avera	ge Hourly Traffi	c Trip Counts	
		Motorcycles	Buses	Automobiles	Medium Trucks	Heavy Trucks	Motorcycles	Buses	Automobiles	Medium Trucks	Heavy Trucks
	Northbound	0	0	1	0	0	0	0	1	0	5
Howard Road / Pancho	Southbound	0	0	1	0	0	0	0	1	0	5
Road	Eastbound										
	Westbound										
	Northbound	0	0	96	3	0	0	0	96	3	5
Pleasant Valley Road	Southbound	0	0	94	5	0	0	0	94	5	5
Pleasallt valley Road	Eastbound	0	0	130	4	0	0	0	130	4	1
	Westbound	0	0	80	1	0	0	0	80	1	1

C - Modeled baseline traffic data was collected by VRBA on 11/28/2018 (see previous sheets). To account for existing/permitted Pacific Rock Quarry haul truck activity during daytime hours (7:00 a.m. - 4:00 p.m.), the baseline daytime "Heavy Truck" traffic numbers were scaled up appropriately.

D - Modeled Project traffic data includes the baseline VRBA/Pacific Rock data, with the Project "Heavy Truck" totals scaled up appropriately to account for the new Project truck trips during the evening and nighttime hours.

Model Results + Impact Determination

Haul Route Receptors												
Receptor	Description	Nearby Roadway	# of Floors	Existing Barriers								
R4	Residential Dwelling	Howard Road / Pancho Road	1	Front Yard Hedges/Trees								
R5-A	Residential Dwelling	Pleasant Valley Road / Pancho Road	1	5-Foot Soundwall along Pleasant Valley Road								
R5-B	Residential Dwelling	Pleasant Valley Road	1	5-Foot Soundwall along Pleasant Valley Road								
R5-C	Residential Dwelling	Pleasant Valley Road	1	5-Foot Soundwall along Pleasant Valley Road								

See Figures 3, 6, and 7 (Appendix A) which show the location of the receptors included in the SoundPLAN model.

CNEL Noise	CNEL Noise Levels @ Haul Route Receptors												
D	Baseline (dBA)	County Evening CNEL	Adjusted Evening CNEL	Total Project (dBA)	Applicable Evening CNEL	Exceed							
Receptor	Outdoor CNEL ^B	Fixed Significance Threshold ^A	Significance Threshold ^A	Outdoor CNEL ^B	Significance Threshold ^A	Threshold?							
R4	50.3	60	60	55.2	60	No							
R5-A	59.7	60	62.7	61.1	62.7	No							
R5-B	60.3	60	63.3	61.4	63.3	No							
R5-C	61.3	60	64.3	61.6	64.3	No							

As discussed on the previous sheet, it is assumed the permit limit of 120 trips would be spread evenly throughout the operating day.

Specifically, Project haul trucks would be limited to 5 loads (10 one-way trips) during the average daytime, evening, and nighttime hours.

CNEL Indoor Noise Levels @ Haul Route Receptors												
D	Baseline (dBA)	County Evening CNEL	Adjusted Evening CNEL	Total Project (dBA)	Applicable Evening CNEL	Exceed						
Receptor	Indoor CNEL ^{B, C}	Fixed Significance Threshold ^A	Significance Threshold ^A	Indoor CNEL ^{B, C}	Significance Threshold ^A	Threshold?						
R4	30.3	45	45	35.2	45	No						
R5-A	39.7	45	45	41.1	45	No						
R5-B	40.3	45	45	41.4	45	No						
R5-C	41.3	45	45	41.6	45	No						

As discussed on the previous sheet, it is assumed the permit limit of 120 trips would be spread evenly throughout the operating day.

Specifically, Project haul trucks would be limited to 5 loads (10 one-way trips) during the average daytime, evening, and nighttime hours.

Footnotes:

A - Per the Ventura County General Plan/CEQA Guidelines (see Appendix C), the outdoor "fixed" CNEL significance threshold is 60 dBA and the indoor "fixed" CNEL significance threshold is 45 dBA.

However, as with the Facility tresholds, if the modeled ambient/baseline noise levels exceed the "fixed" threshold, the modeled "ambient noise level +3 decibels (dBA)" is utilized to determine the significance of haul route noise impacts. As shown above, the "ambient +3 dBA" CNEL threshold is utilized at Receptors R5-A, R5-B, and R5-C to determine the significance of outdoor noise impacts.

However, the "fixed" CNEL thresholds are utilized to determine the significance of outdoor noise impacts (60 dBA) at R4 and indoor impacts (45 dBA) at all receptors (R4, R5-A, R5-B, R5-C).

- B Both the baseline and Project traffic noise levels at haul route receptors were modeled in SoundPLAN Essential. See previous sheets which describes the methodologies and traffic counts input into both the baseline and Project traffic noise models. Please see Figure 6 (Appendix A) for the baseline model results and Figure 7 (Appendix A) for the Project traffic model results.
- C Based on the EPA's *Protective Noise Levels* document (March, 1974), an outdoor to indoor attenuation of 20 dBA is assumed. This takes into account the average noise reduction provided while windows are closed (25 dBA) and while windows are open (15 dBA). This is a conservatively low estimate of noise attenuation as residences are expected to generally keep windows closed, especially those facing sources of noise. The 20 dBA attenuation is applied to the baseline and Project CNEL values. See Appendix B for the applicable excerpt from the EPA guidance document.

PA01_Noise Calcs_fnl.xlsx
April 12, 2019

MODEL OUTPUT FILES - ROAD NOISE (BASELINE)

Noise Emissions of Road Traffic

	Traffic va							Control	Consti	Affect.		Gradie
Statior	ADT	Vehicles type	Vehicle name	day	evenin	night	Speed		Speed			Min / M
km	Veh/2				Veh/h				km/h	%		%
Pancho	o (nort	hbound)	Traf	fic dire	ction: Ir	n entry	direction	on				
0+000		Total	-	47	5	1	_	none	-	-	OGAC (open-graded asphaltic	0.0
		Automobiles	-	28	4	1	8					
		Medium trucks	-	5	1	-	8					
		Heavy trucks	-	14	-	-	8					
		Buses	-	-	-	-	8					
		Motorcycles Auxiliary Vehicle	-	-	-	-	8 8					
0+782	588	Total	-	47	5	1	-	none	-	_	DGAC (dense-graded asphaltic	0.0
0 .02		Automobiles	-	28	4	1	24					0.0
		Medium trucks	-	5	1	-	24					
		Heavy trucks	-	14	-	-	24					
		Buses	-	-	-	-	24					
		Motorcycles	-	-	-	-	24					
1+486	588	Auxiliary Vehicle Total	-	47	5		24	none	-	_	Average (of DGAC and PCC)	0.0
11400	300	Automobiles	_	47 28	4	1	- 48	Horic		_	Average (or BOAG and 1 00)	0.0
		Medium trucks	_	5	1		48					
		Heavy trucks	-	14	-	-	48					
		Buses	-	-	-	-	48					
		Motorcvcles	-	-	-	-	48					
3+067	500	Auxiliary Vehicle	-	47	-	-	48	nono	_	_	PCC (Portland cement concrete	0.0
3+001	300	Total Automobiles	-	47 28	5 4	1	- 72	none	_	-	PCC (Portiand Cement Concrete	0.0
		Medium trucks	-	5	1 1		72					
		Heavy trucks	-	14	_	-	72					
			1		_	_	72					
		Buses	-	-								
		Motorcycles	-	-	-	-	72					
2.106			-	-	-	-						
3+105	-	Motorcycles Auxiliary Vehicle	- - -	- -	-	-	72 72	-	-	-	-	-
Pancho		Motorcycles Auxiliary Vehicle hbound)	- - - Traf		ction: Ir		72 72				- PCC (Portland coment concrete	-
		Motorcycles Auxiliary Vehicle hbound) Total	Traf	47	ction: Ir	1	72 72 direction	- on Traffic light	0.0		PCC (Portland cement concrete	0.0
Pancho		Motorcycles Auxiliary Vehicle hbound) Total Automobiles	- - - Traf	47 29	ction: Ir		72 72 direction - 72				PCC (Portland cement concrete	0.0
Pancho		Motorcycles Auxiliary Vehicle hbound) Total	Traf	47	ction: Ir	1	72 72 direction				PCC (Portland cement concrete	0.0
Pancho		Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses	Traf	47 29 4	ction: Ir	1 1 -	72 72 direction - 72 72 72 72 72				PCC (Portland cement concrete	0.0
Pancho		Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles	Traf	47 29 4	ction: Ir	1 1 - - -	72 72 direction - 72 72 72 72 72 72				PCC (Portland cement concrete	0.0
Pancho 0+000	582	Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle	- - - - - - - - -	47 29 4 14 - -	ction: Ir	1 1	72 72 direction - 72 72 72 72 72 72	Traffic light	0.0	25.0		
Pancho	582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total	Traf	47 29 4 14 - - - 47	ction: lr 3 3 - - - - 3	1 1 1	72 72 direction - 72 72 72 72 72 72 72				PCC (Portland cement concrete Average (of DGAC and PCC)	0.0
Pancho 0+000	582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles	Traf	47 29 4 14 - - - 47 29	ction: Ir	1 1	72 72 direction - 72 72 72 72 72 72 72 - 48	Traffic light	0.0	25.0		
Pancho 0+000	582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks	Traf	47 29 4 14 - - - 47	ction: lr 3 3 - - - - 3	1 1 1	72 72 direction - 72 72 72 72 72 72 - 48 48	Traffic light	0.0	25.0		
Pancho 0+000	582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Heavy trucks Buses	Traf	47 29 4 14 - - - 47 29 4	ction: Ir	1 1 1	72 72 direction - 72 72 72 72 72 72 72 - 48	Traffic light	0.0	25.0		
Pancho 0+000	582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Motorcycles	Traf	47 29 4 14 - - - 47 29 4	ction: Ir	1 1 1	72 72 72 72 72 72 72 72 72 72 72 72 72 48 48 48 48	Traffic light	0.0	25.0		
Pancho 0+000 0+032	582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle	- Traf	47 29 4 14 - - 47 29 4 14 -	3 3 3 3 3	1 1 1 1	72 72 72 72 72 72 72 72 72 72 72 72 48 48 48 48 48	Traffic light	0.0	25.0	Average (of DGAC and PCC)	0.0
Pancho 0+000	582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total	- Traf	47 29 4 14 - - 47 29 4 14 - - 47	3 3	1 1 1 1 1	72 72 72 72 72 72 72 72 72 72 72 72 48 48 48 48 48	Traffic light	0.0	25.0		0.0
Pancho 0+000 0+032	582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Total Automobiles Auxiliary Vehicle Total Automobiles	Traf	47 29 4 14 - - 47 29 4 14 - - 47 29	3 3 3 3 3	1 1 1 1	72 72 72 72 72 72 72 72 72 72 72 72 48 48 48 48 48 48	Traffic light	0.0	25.0	Average (of DGAC and PCC)	0.0
Pancho 0+000 0+032	582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Medium trucks Medium trucks	Traf	47 29 4 14 - - 47 29 4 14 - - - 47 29 4 14 - - 47 29 4	3 3	1 1 1 1 1	72 72 72 72 72 72 72 72 72 72 72 72 48 48 48 48 48 48 48 48	Traffic light	0.0	25.0	Average (of DGAC and PCC)	0.0
Pancho 0+000 0+032	582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Total Automobiles Auxiliary Vehicle Total Automobiles	Traf	47 29 4 14 - - 47 29 4 14 - - 47 29	3 3 3 3 3 3 3 3	1 1 1 1 1	72 72 72 72 72 72 72 72 72 72 72 72 48 48 48 48 48 48	Traffic light	0.0	25.0	Average (of DGAC and PCC)	0.0
Pancho 0+000 0+032	582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles		47 29 4 14 - - 47 29 4 14 - - - 47 29 4 14 - - 47 29 4	3 3 3 3 3 3 3 3	1 1 1 1 1	72 72 72 72 72 72 72 72 72 72 72 72 48 48 48 48 48 48 48 48 48 48 48 48 48	Traffic light	0.0	25.0	Average (of DGAC and PCC)	0.0
0+000 0+032 1+613	582 582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Heavy trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle		47 29 4 14 - - 47 29 4 14 - - - 47 29 4 14 - -	3 3 3 - - - 3 3 3 - - - - - 3 3 3	1 1 1 1	72 72 72 72 72 72 72 72 72 72 72 72 48 48 48 48 48 48 48 48 48 48 48 48 48	none			Average (of DGAC and PCC) DGAC (dense-graded asphaltic	0.0
Pancho 0+000 0+032	582 582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Total		47 29 4 14 - - 47 29 4 14 - - 47 29 4 14 - - - 47	3 3	1 1 1 1 1	72 72 72 72 72 72 72 72 72 72 72 72 48 48 48 48 48 48 48 48 48 48 48 48 48	Traffic light	0.0	25.0	Average (of DGAC and PCC)	0.0
0+000 0+032 1+613	582 582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Auxiliary Vehicle Total Automobiles	- Traf	47 29 4 14 - - 47 29 4 14 - - 47 29 4 14 - - - 47 29 4 14 - - - - 47 29 4 47 29 4 47 47 47 47 47 47 47 47 47 47 47 47 4	3 3 3 - - - 3 3 3 - - - - - 3 3 3	1 1 1 1	72 72 72 72 72 72 72 72 72 72 72 72 72 48 48 48 48 48 48 48 48 24 24 24 24 24 24	none			Average (of DGAC and PCC) DGAC (dense-graded asphaltic	0.0
0+000 0+032 1+613	582 582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks	- Traf	47 29 4 14 - - 47 29 4 14 - - 47 29 4 14 - - - 47 29 4 14 - - - - 47 29 4 4 14 - - - - - - - - - - - - - - - -	3 3	1 1 1 1 1	72 72 72 72 72 72 72 72 72 72 72 72 48 48 48 48 48 48 48 48 48 48 48 48 48	none			Average (of DGAC and PCC) DGAC (dense-graded asphaltic	0.0
0+000 0+032 1+613	582 582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Heavy trucks Heavy trucks	- Traf	47 29 4 14 - - 47 29 4 14 - - 47 29 4 14 - - - 47 29 4 14 - - - - 47 29 4 47 29 4 47 47 47 47 47 47 47 47 47 47 47 47 4	3 3	1 1 1 1 1	72 72 72 72 72 72 72 72 72 72 72 72 48 48 48 48 48 48 48 48 48 48 48 24 24 24 24 24 24 24 24 28 8 8 8	none			Average (of DGAC and PCC) DGAC (dense-graded asphaltic	0.0
0+000 0+032 1+613	582 582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks		47 29 4 14 - - 47 29 4 14 - - 47 29 4 14 - - - 47 29 4 14 - - - - 47 29 4 4 14 - - - - - - - - - - - - - - - -	3 3	1 1 1 1 1	72 72 72 72 72 72 72 72 72 72 72 72 48 48 48 48 48 48 48 48 48 48 48 48 48	none			Average (of DGAC and PCC) DGAC (dense-graded asphaltic	0.0
0+032 1+613	582 582 582	Motorcycles Auxiliary Vehicle hbound) Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Motorcycles Auxiliary Vehicle Total Automobiles Medium trucks Heavy trucks Buses Medium trucks Heavy trucks Heavy trucks Buses		47 29 4 14 - - 47 29 4 14 - - 47 29 4 14 - - - 47 29 4 14 - - - - 47 29 4 4 14 - - - - - - - - - - - - - - - -	3 3	1 1 1 1 1 1 1 1	72 72 72 72 72 72 72 72 72 72 72 72 72 48 48 48 48 48 48 48 24 24 24 24 24 24 24 24 24 28 8 8 8	none			Average (of DGAC and PCC) DGAC (dense-graded asphaltic	0.0

Noise Emissions of Road Traffic

			Traffic valu	es				Control	Constr	Affect		Gradie
Statior	ADT	Vehicles type	Vehicle name		evenir	niaht	Speed		Speed		Road surface	Min / N
	Veh/2					Veh/h			km/h	%		%
		thbound)	Traf	fic dire				nn .				
0+000	10827	Total	1141	756	288	99	uncen	Traffic light	0.0	25.0	PCC (Portland cement concrete	0.0
0.000	10021	Automobiles	- -	717	281	96	80	l ramo ngm	0.0	20.0	l co (r chiana coment concret] ""
		Medium trucks	-	26	7	3	80					
		Heavy trucks	_	13	-	-	80					
		Buses	-	-	-	-	80					
		Motorcycles	-	-	-	-	80					
0+104	10827	Auxiliary Vehicle	-	75.0	-	- 00	80	none	-	-	PCC (Portland cement concrete	0.0
0.10-	10021	Automobiles	_	756 717	288 281	99 96	80	TIOTIC		_	oo (i ordana cement concrete	1 0.0
		Medium trucks	_	26	7	3	80					
		Heavy trucks	_	13	-	-	80					
		Buses	-	-	-	-	80					
		Motorcycles	-	-	-	-	80					
0+790	_	Auxiliary Vehicle	-	-	-	-	80	_	_		_	<u> </u>
		thbound)	Traf	fic dire	rtion: le	n entry	directio	nn				
0+000	9843	Total	liai	687	236	99	anecul	Stop sign	0.0	50 O	PCC (Portland cement concrete	0.0
0.000	0010	Automobiles	-	636	230	99	80	otop olgin	0.0	00.0	l co (r chiana coment concret] "."
		Medium trucks	-	37	9	5	80					
		Heavy trucks	-	12	-	-	80					
		Buses	-	1	-	-	80					
		Motorcycles	-	1	-	-	80					
0+193	9843	Auxiliary Vehicle Total	-	687	236	99	80	none	_	_	PCC (Portland cement concrete	0.0
		Automobiles	- -	636	227	94	80]
		Medium trucks	-	37	9	5	80					
		Heavy trucks	-	12	-	-	80					
		Buses	-	1	-	-	80					
		Motorcycles Auxiliary Vehicle	-	1	-	-	80 80					
0+701	-	Auxiliary verlicie					- 00	-	-	-	-	-
Pleasa	nt (wes	stbound)	Traf	fic dire	ction: Ir	n entry	direction	on				
0+000	9060	Total	-	649	181	81	-	Traffic light	0.0	25.0	PCC (Portland cement concrete	0.0
		Automobiles	-	625	177	80	80					
		Medium trucks	-	19	4	1	80					
		Heavy trucks	-	3	-	-	80					
		Buses Motorcycles	_	1 1	_	_	80 80					
		Auxiliary Vehicle	_	_ '	_	_	80					
0+075	9060	Total	-	649	181	81	-	none	-	-	PCC (Portland cement concrete	0.0
		Automobiles	-	625	177	80	80					
		Medium trucks	-	19	4	1	80					
		Heavy trucks	-	3	-	-	80					
		Buses Motorcycles	-		_	_	80 80					
		Auxiliary Vehicle	-	'	_	_	80					
0+795	_							-	-	-	-	
Pleasa	nt (eas	tbound)	Traf	fic dire	ction: I	entry	direction	on				
0+000	10062		<u> </u>	672	264	134	-	none	-	-	PCC (Portland cement concrete	0.0
		Automobiles	-	628	256	130	80					
		Medium trucks	-	37 5	7	4	80 80					
		Heavy trucks Buses	- -	5	_		80 80					
		Motorcycles	_	1	1	_	80					
		Auxiliary Vehicle	-	-	-	-	80					
0+800	-							-	-	-	-	-

Receiver List

		Coordi	nates					Limit				Le	/el			Conflict	
No.	Receiver name	X	Υ	Buildir	Floor	Heigl	Day	Evenir Ni	ght	Lden	Day	Evenir	Night	Lden	Day	Evenir Nigh	nt Lden
		in me	eter	side		m		dB(A)				dB	(A)			dB(A)	
1	R4	314595.53	3785170		1.FI	1.50	-	-	-	-	53.2	34.6	25.6	50.3	-	-	
2	R5-A	314626.53	3786889		1.FI	1.50	-	-	-	-	59.9	53.8	49.8	59.7	-	-	
3	R5-B	314822.73	3787273		1.FI	1.50	-	-	-	-	60.2	54.7	50.6	60.3	-	-	
4	R5-C	314409.73	3786799		1.FI	1.50	-	-	-	-	60.8	55.4	52.1	61.3	-	_	

Contribution Levels of the Receivers

				Le	vel	
Source name		Lane	Day	Evening	Night	Lden
				dB	(A)	
R3	1.Fl		53.2	34.6	25.6	50.3
Pancho (northbound)			51.7	34.0	23.4	48.8
Pancho (southbound)			47.9	21.1	16.4	44.9
Pleasant (eastbound)		l i	23.3	18.4	15.0	24.0
Pleasant (northbound)			23.9	17.8	13.3	23.6
Pleasant (southbound)			23.8	17.1	13.7	23.6
Pleasant (westbound)			23.8	17.2	13.4	23.5
R4-A	1.Fl		59.9	53.8	49.8	59.7
Pancho (northbound)			41.8	24.1	12.8	38.9
Pancho (southbound)		l j	41.9	18.3	13.5	39.0
Pleasant (eastbound)		ĺ	46.5	41.8	38.7	47.5
Pleasant (northbound)		l j	56.6	50.7	46.2	56.3
Pleasant (southbound)		l j	55.8	49.8	46.2	55.9
Pleasant (westbound)			47.2	40.8	37.1	47.0
R4-B	1.Fl		60.2	54.7	50.6	60.3
Pancho (northbound)			34.0	15.2	3.4	31.1
Pancho (southbound)		[31.9	7.2	2.4	28.9
Pleasant (eastbound)			34.5	29.0	25.7	35.0
Pleasant (northbound)		ĺ	56.7	51.7	47.2	56.9
Pleasant (southbound)		ĺ	57.6	51.5	47.9	57.6
Pleasant (westbound)			35.2	27.9	24.1	34.6
R4-C	1.Fl		60.8	55.4	52.1	61.3
Pancho (northbound)			39.0	19.3	7.6	36.1
Pancho (southbound)		[37.6	13.2	8.5	34.6
Pleasant (eastbound)		l	57.4	52.7	49.7	58.5
Pleasant (northbound)		ĺ	43.1	35.6	31.1	42.2
Pleasant (southbound)		l j	41.8	34.2	30.7	41.1
Pleasant (westbound)			57.8	51.8	48.2	57.8

Spectra of the Receivers

No	Name	Floor	Time	50 F	63 F	80 F	100	125	160	200	250	315	400	500	630	800	1 kH	1 kH:	2 kH	2 kH	2 kH:	3 kH₄	4 kH	5 kH	6 kH	3 kH	10 k
1	R4	1.FI	Day	25.4	31.	36.	39.4	41.2	42.	43.	43.2	40.	36.0	37.	38.4	40.	41.	40.9	39.6	39.1	40.4	40.	38.	35.	35.	32.	28.
			Even	11.4	16.8	20.	22.	23.6	24.	24.0	21.8	21.0	21.	22.9	24.0	21.8	22.3	21.0	21.4	20.8	19.6	17.6	14.	11.	10.	7.6	3.8
			Night	1.9	9.1	13.	14.9	15.7	16.	15.8	11.3	11.:	11.	13.0	،.14	12.0	13.3	11.9	11.9	10.	8.0	5.6	2.4	-0.6	-1.4	-4.1	-7.3
			Lden	22.7	29.0	33.	36.0	38.4	39.	40.6	40.3	37.1	33.8	34.9	35.0	،.37	38.2	38.0	36.	36.1	،37	37.	35.	32.	32.	29.	25.
3	R5-B	1.FI	Day	28.7	36.2	40.9	43.4	45.0	46.	46.4	43.4	42.	42.	45.	49.4	51.	52.8	51.6	49.6	48.4	46.	43.	40.3	36.	35.	32.6	28.
			Even	22.8	30.2	34.	37.2	38.6	39.	39.8	36.	36.	36.8	39.9	44.0	45.:	47.6	46.6	44.	43.	40.9	36.0	31.4	25.0	25.2	22.2	20.
			Night	19.0	26.3	30.9	33.4	34.8	35.	36.0	32.8	،.32	32.8	35.8	40.0	41.4	43.	42.4	40.6	39.	36.	31.8	27.3	20.9	21.:	18.2	16.
			Lden	28.	36	40.8	43.	44.8	45.	46.	43.0	42.،	42.	45.6	49.0	51.:	53.	51.9	50.0	48.	46.	42.	39.0	، 34	34.	30.9	27.1
2	R5-A	1.FI	Day	29.7	37.	42.0	44.	46.0	47.	47.	44.4	43.	42.	44.0	47.8	49.	51.6	51.0	49.	47.	46.	44.	40.8	37.	37.0	33.6	29.
			Even	24.	31.	35.0	38.0	39.	40.	40.4	36.9	36.	35.	38.0	42.0	43.0	46.2	45.	44.8	42.	39.6	36.0	30.0	24.0	24.4	،21	19.1
ı			Night	20.0	27.	31.0	34.0	35.	36.	36.4	32.8	32.0	31.	34.0	38.	39.	42.2	41.	40.	38.	35.	31.9	26.2	20.0	،.20	17.4	15.
ᆫ			Lden	29.8	37.0	41.	44.	45.8	46.	46.9	43.6	42.0	41.8	44.2	47.8	49.	51.8	51.2	50.	47.8	45.8	43.	39.0	35.	34.	31.	27.4
4	R5-C	1.FI	Day	28.8	36.2	40.8	43.2	44.8	45.9	46.2	42.8	42.	43.4	47.	51.6	52.:	52.8	51.4	51.4	48.	46.	43.	38.9	34.	33.	30.	27.
			Even	22.9	4.00	34.9	37.	38.9	39.9	40.2	36.0	36.	37.	41.	46.	47.1	47.	46.0	46.	43.	40.9	36.8	31.6	25.	25.	،.22	20.
			Night																								
L			Lden	29.0	36.4	40.9	43.4	44.9	45.9	46.2	42.8	42.4	43.	47.8	52.0	52.	53.	52.	52.	49.	46.8	43.2	38.	33.4	32.	29.	26.

MODEL OUTPUT FILES - ROAD NOISE (PROJECT)

Noise Emissions of Road Traffic

Station ADT		Traffic v							Control	Constr	Affect.		Gradie
Mary Mary	Station	ADT	Vehicles type	Vehicle name	day	evenin	night	Speed					Min / M
0+00	km	Veh/24								km/h	%		%
0+00 SAZ Total	Pancho	o (nort	hbound)	Traf	fic dire	ction: Ir	n entry	directio	าท				
Automobiles				_				_		-	-	OGAC (open-graded asphaltic	0.0
Medium trucks				_				8					
Buses				-		1	-						
Motorcycles			1	-	6	5	5						
Mary Mary				-	-	-	-						
1+86 552 Total				-	-	-	-						
Automobiles	0+782	552	Total	-	20				none	-	_	DGAC (dense-graded asphaltic	0.0
Medium trucks	0 1 0 2	002	Automobiles	-					110110			Berte (denies graded depriant	1
Heavy trucks Heavy trucks Noticycycles Noticycles Noticycycles Noticycles Noticycycles Noticycycles Noticycycles Noticycycles Noticycycles Noticycycles Noticycycles Noticycycles Noticycycles Noticycles Noticycles Noticycles Noticycles Noticycles Noticycles Noticycles Noticycles Noticycle				-	1	l							
Motorcycles				-	6	5	5	24					
1+486 552 Total				-	-	-	-						
1+48				-	-	-	-						
Automobiles	1+486	552		-	20				none	_	_	Average (of DGAC and PCC)	0.0
Medium trucks	11.409	002		_					Hone		_	Average (or BOAO and 1 00)	0.0
Heavy trucks Buses				_		1	_						
Motorcycles				-			5						
Auxiliary Vehicle -				-	-	-	-						
3+06				-	-	-	-						
Automobiles	3+067	552	Auxiliary Vehicle	-	- 20	10	-		none	<u> </u>	_	PCC (Portland coment concrete	0.0
Medium trucks Heavy trucks Hea	3+001	332		-					lione	_	_	FOC (Fortialid Cernetit Concrete	0.0
Heavy trucks Suses - 6 5 5 72				- -	1	1							
Buses Auxiliary Vehicle Auxiliary Vehicl				-	1	1	5						
Auxiliary Vehicle - - - - 72 - - - - - - - -			Buses	=	-	-	-						
3+10 3+10				=	-	-	-						
Pancho (southbound)	2+106		Auxiliary Vehicle	-	-	-	-	/2		1			
O+000		- (1	- \)	Τ	۲۰ - ۱۰	. (*) .		-l*(*	<u>-</u>	_	-	<u>-</u>	_
Automobiles				IIai				airectio		0.0	25.0	DCC (Partland coment concrete	0.0
Medium trucks	0.000	340		-				72	Traille light	0.0	25.0	or (i ordana cement concrete	0.0
Heavy trucks - 6 5 5 72				- -	1	1							
Motorcycles				-	1	5	5						
Auxiliary Vehicle				-	-	-	-						
0+032 546				-	-		-						
Automobiles Medium trucks -	0+031	546	Auxiliary Vehicle	-	-				nono			Average (of DCAC and BCC)	0.0
Medium trucks	0+032	340		-	1				none	-	-	Average (of DGAC and PCC)	0.0
Heavy trucks				_									
Motorcycles - - - - 48				-	1	1							
Auxiliary Vehicle			Buses	-	-	-	-	48					
1+613 546 Total - 39 8 6 - none - - DGAC (dense-graded asphaltic of the product				-	-	-	-						
Automobiles - 29 3 1 24 Heavy trucks - 4 - 24 Heavy trucks - 24 Hotorcycles - 24 Auxiliary Vehicle - 29 3 1 8 Heavy trucks - 24 Auxiliary Vehicle - 29 3 1 8 Heavy trucks - 86 5 5 8 Heavy trucks - 86 5 5 8 Heavy trucks - 86 5 5 8 Heavy trucks - 86 5 5 8 Heavy trucks - 86 5 5 8 Heavy trucks - 86 5 5 8 Heavy trucks - 86 Heavy trucks - 86 5 5 8 Heavy trucks - 86 5 5 8 Heavy trucks - 86 Heavy trucks - 86 Heavy trucks - 86 Heavy trucks - 86 Heavy trucks - 86 Heavy trucks - 86 Heavy trucks - 86 Heavy trucks - 86 Heavy trucks - 86 Heavy trucks - 86 Heavy trucks - 86 Heavy trucks - 86 Heavy trucks - 86 Heavy trucks - 86 Heavy trucks - 87 Heavy trucks - 88 Heavy	1+611	546		-	-	-	-		none	-		DGAC (dense-graded asphaltic	0.0
Medium trucks	1+013	540		- -					IIIOIIE	-	_	DONO (delise-graded asplialit	1 0.0
Heavy trucks - 6 5 5 24				- -		5	'						
Buses				_	1	5	5						
Auxiliary Vehicle -			1	_	-	-	_						
2+323 546 Total - 39 8 6 - none - - OGAC (open-graded asphaltic 0 Automobiles - 29 3 1 8 Medium trucks - 4 - - 8 Heavy trucks - 6 5 5 8 Buses - - - 8 Motorcycles - - 8 Auxilliary Vehicle - - 8				_	-	-	-						
Automobiles - 29 3 1 8	2 , 224	E 40	Auxiliary Vehicle	-	-		-		200	-		OCAC (anon graded seek alti-	
Medium trucks - 4 - - 8 Heavy trucks - 6 5 5 8 Buses - - - 8 Motorcycles - - - 8 Auxiliary Vehicle - - - 8	2+323	546		_					none	_	_	OGAC (open-graded asphaltic	0.0
Heavy trucks				- -		3	1						
Buses 8 Motorcycles 8 Auxiliary Vehicle 8				_		5	5						
Auxiliary Vehicle 8				_	-	_	_						
Auxiliary Vehicle 8				-	-	-	-						
	2.404		Auxiliary Vehicle	_	-	-	-	8		-			
3+106	ა+10€	-	L						-		-	<u> -</u>	<u> </u>

Noise Emissions of Road Traffic

			Traffic valu	es				Control	Constr	Affect.		Gradie
Station	ADT	Vehicles type	Vehicle name		evenin	night	Speed		Speed		Road surface	Min / N
	Veh/24					Veh/h			km/h	%		%
		thbound)	Traf				direction	nn .				
0+000	10800	Total		749	292	104	uncen	Traffic light	0.0	25.0	PCC (Portland cement concrete	0.0
	10000	Automobiles	- -	749	281	96	80	Tramo ngin	0.0	20.0] "."
		Medium trucks	-	26	7	3	80					l
		Heavy trucks	-	6	4	5	80					l
		Buses	-	-	-	-	80					l
		Motorcycles	-	-	-	-	80					l
0+104	10000	Auxiliary Vehicle	=	-	-		80	nana			PCC (Portland cement concrete	0.0
0+104	10000		-	749	292	104	-	none	-	-	PCC (Portland Cement Concrete	0.0
		Automobiles Medium trucks	-	717 26	281 7	96 3	80 80					l
		Heavy trucks	- -	6	4	5	80					l
		Buses	_	-	_	-	80					l
		Motorcycles	-	-	-	-	80					l
		Auxiliary Vehicle	-	-	-	-	80					
0+790	-							-	-	-	-	-
Pleasar	nt (sou	thbound)	Traf	fic dire	ction: Ir	entry	direction					
0+000	9816	Total	-	680	240	104	-	Stop sign	0.0	50.0	PCC (Portland cement concrete	0.0
		Automobiles	-	636	227	94	80					
		Medium trucks	-	37	9	5	80					
		Heavy trucks	-	5	4	5	80					
		Buses	-	1	-	-	80					l
		Motorcycles	=]	-	-	80 80					l
0+193	9816	Auxiliary Vehicle Total	-	680	240	104	- 00	none	-	_	PCC (Portland cement concrete	0.0
	0010	Automobiles	_	636	240	94	80	110110] "."
		Medium trucks	_	37	9	5	80					l
		Heavy trucks	=	5	4	5	80					l
		Buses	-	1	-	-	80					l
		Motorcycles	-	1	-	-	80					l
0.704		Auxiliary Vehicle	-	-	-	-	80					
0+701	-		<u> </u>	с. I.			1	-	-	-	<u> -</u>	
0+000	nt (wes	tbound)	<u> </u>				direction	Traffic light	0.0	25.0	PCC (Portland cement concrete	0.0
0+000	9040		-	647	182	82	-	Trailic light	0.0	25.0	FCC (Fortiand cement concrete	0.0
		Automobiles Medium trucks	-	625	177	80 1	80 80					l
		Heavy trucks	_	19 1	4	1	80					l
		Buses	_		_	_ '	80					l
		Motorcycles	-	1 1	_	_	80					
		Auxiliary Vehicle	-	-	-	-	80					
0+075		Total	-	647	182	82	-	none	-	-	PCC (Portland cement concrete	0.0
		Automobiles	-	625	177	80	80					
		Medium trucks	=	19	4	1	80					
		Heavy trucks	-	1	1	1	80					
		Buses	-]	-	-	80					
		Motorcycles Auxiliary Vehicle	- -		_	_	80 80					
0+795	-	raxinary vernere					- 00	_	-	-	-	-
Pleasar	nt (eas	tbound)	Traf	fic dire	ction: Ir	n entrv	direction	on				
0+000	10062	Total	-	671	265	135	_	none	-	-	PCC (Portland cement concrete	0.0
		Automobiles	_	628	256	130	80				,	
		Medium trucks	_	37	7	4	80					
		Heavy trucks	-	4	1	1	80					
		Buses	-	1	-	-	80					
		Motorcycles	-	1	1	-	80					
0+004		Auxiliary Vehicle	-	-	-	-	80					-
0+800	-	l						-	-		<u> -</u>	
l												

Receiver List

		Coordinates					Limit			Le	vel			Conflict	
No.	Receiver name	X Y	Buildir	Floor	Heigl	Day	Evenir Nig	ht Lden	Day	Evenir	Night	Lden	Day	Evenir Nigh	t Lden
		in meter	side		m		dB(A)			dB	(A)			dB(A)	
1	R4	314595.(3785170		1.FI	1.50	-	-		50.0	48.5	48.3	55.2	-		-
2	R5-A	314626.53786889		1.FI	1.50	-	-		59.1	55.2	53.1	61.1	ı		-
3	R5-B	314822.73787273		1.FI	1.50	-	-		59.7	55.6	53.1	61.4	-	-	-
4	R5-C	314409.73786799		1.FI	1.50	-	-		60.6	55.8	52.9	61.6	-		-

Contribution Levels of the Receivers

			Le	vel	
Source name	Lane	Day	Evening	Night	Lden
			dB	(A)	
R3 1.Fl		50.0	48.5	48.3	55.2
Pancho (northbound)		48.5	47.0	46.8	53.7
Pancho (southbound)		44.6	43.2	43.1	50.0
Pleasant (eastbound)		23.2	18.7	15.6	24.3
Pleasant (northbound)		23.1	19.3	17.3	25.3
Pleasant (southbound)		23.0	18.8	17.4	25.2
Pleasant (westbound)		23.4	18.0	15.0	24.0
R4-A 1.Fl		59.1	55.2	53.1	61.1
Pancho (northbound)		38.6	37.1	36.9	43.8
Pancho (southbound)		38.7	37.0	37.0	43.9
Pleasant (eastbound)		46.4	42.0	39.1	47.7
Pleasant (northbound)		55.8	52.1	49.9	57.9
Pleasant (southbound)		55.2	51.0	49.1	57.1
Pleasant (westbound)		46.8	41.6	38.7	47.6
R4-B 1.Fl		59.7	55.6	53.1	61.4
Pancho (northbound)		30.8	29.4	29.3	36.1
Pancho (southbound)		28.7	27.0	27.0	33.9
Pleasant (eastbound)		34.3	29.6	26.9	35.5
Pleasant (northbound)		56.3	52.3	49.0	57.6
Pleasant (southbound)		56.9	52.8	50.8	58.9
Pleasant (westbound)		34.3	29.6	27.4	35.8
R4-C 1.Fl		60.6	55.8	52.9	61.6
Pancho (northbound)		35.7	34.4	34.3	41.2
Pancho (southbound)		34.4	32.8	32.8	39.6
Pleasant (eastbound)		57.3	52.9	50.1	58.6
Pleasant (northbound)		41.7	38.3	37.2	44.7
Pleasant (southbound)		40.5	36.9	36.0	43.5
Pleasant (westbound)		57.6	52.2	49.0	58.1

Spectra of the Receivers

No	Name	Floor	Time	50 F	63 F	80 F	100 1	125	160	200	250	315	400	500	630	800	1 kH	1 kH:	2 kH	2 kH:	2 kH∶	3 kH⋅	4 kH	5 kH	6 kH	8 kH	10 k
1	R4	1.FI	Day	23.	29.:	33.	36.	38.2	39.	40.2	39.8	37.0	34.0	35.	35.9	37.	38.0	37.6	36.	36.0	37.0	37.0	35.0	32.	31.	28.6	24.
ı			Even	19.7	26.	31.	34.0	36.	38.0	38.8	38.6	35.4	31.	32.	33.	35.0	36.4	36.	34.8	34.4	35.7	35.9	33.9	31.	30.	27.6	23.
ı			Night	19.	26.	31.1	34.4	36.0	37.9	38.7	38.	35.2	31.	32.2	32.8	35.4	36.0	36.	34.6	34.:	35.6	35.8	33.9	31.	30.	27.	23.
L			Lden	26.6	33.:	38.2	41.:	43.2	44	45.6	45.4	42.	38.2	39.3	40.0	42.:	43.2	43.0	41.6	41.	42.	42.6	40.	37.	37.:	34.:	30.1
3	R5-B	1.FI	Day	28.	35.	40.2	42.	44.2	45.0	45.	42.4	41.9	42.0	45.	49.0	50.	52.4	51.:	49.4	48.0	45.9	42.0	38.4	33.	33.	30.0	26.
ı			Even	23.8	31.4	36.	38.0	40.	41.2	41.	38.	37.	37.8	41.0	44.8	46.	48.2	47.	45.2	44.0	42.0	38.9	35.6	31.	31.	27.8	23.
ı			Night	21.	29.3	34.0	36.	38.2	39.4	39.8	36.9	35.9	35.2	38.	42.0	44.0	45.0	44.(41.9	41.0	39.	37.9	35.2	31.	31.:	27.8	22.
			Lden	29.8	،.37	42.	44.	46.	47.4	47.	44.8	44.0	43.6	46.9	٠.50	52.:	53.8	52.6	50.6	49.	47.7	45.	42.4	38.0	38.:	34.8	30.2
2	R5-A	1.FI	Day	29.0	36.	41.:	43.	45.	46.0	46.4	43.	42.	41.2	43.	47.1	48.9	51.	50.6	49.	47.:	45.2	42.	38.	34.	34.:	31.0	26.
ı			Even	25.0	32.:	37.	39.	41.4	42.4	42.6	39.	38.4	37.3	39.8	43.0	44.9	47.(46.	45.2	43.2	41.	39.	35.9	32.0	32.	28.	24.1
			Night	22.2	30.	35.	38.0	39.7	40.8	41.	38.4	37.	35.	38.2	40.	42.	44.4	43.	42.0	40.0	40.2	39.	35.9	33.0	32.	29.0	24.
			Lden			_	_		_	_	_		_				_	_		_		_					_
4	R5-C		, ,																								
ı			Even																								
ı			Night																								
L			Lden	29.6	37.0	41.(44	45.6	46.	47.(43.	43.	44.2	48.	52.	53.	53.7	52.:	52.	49.	47.:	44.:	40.	35.	35.	31.	28.1

Noise and Groundborne Vibration Impact Assessme	ent	LU10-0003 Mod	ification Application Pacific Rock Quarry
			APPENDIX F
	BLASTING VIBRATIO	ON IMPACT D	

Blasting Vibration Impacts

LU10-003 CUP Modification
Pacific Rock Quarry

Vibration Impacts @ Facility Receptors

Blasting Vibration Impacts

Blasting Vibration

Based on the 17th Edition ISEE *Blasters Handbook* (1998), Cleveland Ohio, for average ground response.

$$PPV = 160 \left(\frac{D}{\sqrt{W}}\right)^{-1.6}$$

Receptor:	R1	R2-A	R2-B	R3-C	R3	
D = distance from blast to structure:	1,165	1,211	1,266	943	390	feet
W = maximum lbs explosives/delay:	110	110	110	110	110	lbs
PPV = peak particle velocity:	0.085	0.080	0.075	0.120	0.492	in/sec
Significance Threshold (PPV):	0.50	0.50	0.50	0.50	0.50	
Significant?	No	No	No	No	No	

Vibration Significance Thresholds

gillicance illicanolas							
Vibration Str	ucture Damage						
Category PPV (in/sec)							
Equivalent to jumping on the floor	0.3						
Equivalent to door slam:	0.5						
Equivalent to nail driving	0.9						
No damaged to a residential structure:	< 2.0						
Probable damage to a residential structure	> 4.0						

Human Response to Blast	ing Vibration
Average Human Response	PPV (in/sec)
Barely to distinctly perceptible:	0.02 - 0.10
Distinctly to strongly perceptible:	0.1 - 0.5
Strongly perceptible to mildly unpleasant:	0.5 - 1.0
Mildly to distinctly unpleasant:	1.0 - 2.0
Distinctly unpleasant to intolerable:	2.0 - 1.0

Source: Caltrans Transportation and Construction Vibration Guidance Manual (September 2013)

Executive Summary

This Transportation Impact Study (TIS) has been prepared for the purpose of analyzing transportation-related impacts associated with the proposed Pacific Rock Quarry Expansion Project (Project) to support the County's preparation of an Environmental Impact Report (EIR) for the Project for compliance with the California Environmental Quality Act (CEQA).

Pacific Rock, Inc. (Applicant) is requesting an amendment to the existing conditional use permit (CUP) and approved reclamation plan to extend the life of the existing mining operation by an additional 30 years, expand the mining area, extend the operational days from 6 to 7 days per week (adding Sunday for material load out) with additional material load out hours and limited extended 24 hour operations (60 days maximum per year), extend the daily hours of operation (for materials hauling) from the currently permitted 7:00 AM to 4:00 PM to the proposed 5:30 AM to 10:00 PM, allow construction and mobile mining equipment in outdoor storage areas, allow concrete and asphalt recycling, allow for imported material to be used as reclamation fill, and replace an existing mobile home to be used as a 24-hour security trailer.

Although the Applicant does not propose a change in the daily number of permitted loads of aggregate that can be hauled from the site (60 loads), the Project would expand the permitted hours and days of operation and would permit other changes in operations that would create the potential for increased haul truck and worker trips as compared to existing/baseline conditions.

This TIS includes an evaluation of the Project effects on traffic delay on public roads. Traffic delay has been a traditional measure of project traffic impacts under CEQA for several decades, but recent changes to CEQA direct public agencies to no longer consider traffic delay as a CEQA impact. The CEQA Guidelines were amended in December 2018 as a result of amendments to the CEQA statute pursuant to Senate Bill 743 (SB 743) of 2013. Except as provided for certain transportation-related projects, Section 15064.3 of the CEQA Guidelines directs that a project's effect on automobile delay shall not constitute a significant environmental impact. CEQA Guidelines Section 15064.3 describes specific considerations for evaluating a project's transportation impacts and advises that vehicle miles traveled (VMT) is generally the most appropriate measure of transportation impacts. These amendments to CEQA and the CEQA Guidelines change the way that transportation studies must be conducted for environmental documents. Traffic delay-based metrics such as roadway capacity and level of service performance measures that have traditionally been used to assess transportation impacts of projects under CEQA must be replaced by new performance measures such as VMT or other similar measures. July 1, 2020 is the statewide date by which implementation of VMT or other similar metric must be used for transportation impact analysis, however, agencies may opt-in use of new metrics prior to that date.

Notwithstanding these recent changes to CEQA, the traffic operations analysis in this TIS uses the traditional practice of measuring delay, vehicle/capacity ratios, and levels of service for



informational purposes. Ultimately, the County will determine the proper characterization of this information in the EIR for consideration by decisionmakers. For instance, the County may elect to use this analysis as a means of considering the Project consistency with local agency General Plan goals and objectives associated with traffic operations, but without correlating traffic congestion to a CEQA impact. Thus, although Project effects on traffic delay are presented in this TIS, these effects should not be interpreted as an environmental "impact" under CEQA.

At the time of preparation of this TIS, Ventura County is considering VMT analysis methodologies and significance thresholds for CEQA review of projects within the County; however, the County has not yet adopted, and is not yet required under CEQA to adopt or implement, a transportation impact evaluation approach using VMT or similar metric as an alternative to the congestion-based analysis discussed above. This TIS does not include an evaluation of VMT associated with the Project; however, it is anticipated that the County will prepare and include an estimate of Project-related VMT in the EIR for the purposes of disclosure and in consideration of the intent of SB 743 and CEQA Guidelines.

TRAFFIC OPERATIONS SUMMARY

Project Trip Generation

The Project would generate up to 30 truckloads (resulting in 60 one-way trips) per hour during AM peak hours and up to 15 truckloads per hour (resulting 30 one-way trips) during PM peak hours. This study evaluates the Project as if all trips associated with haul trucks during the AM peak-hour period would be new trips that do not currently occur under baseline conditions. A "Passenger Car Equivalent" (PCE) factor of 2.5 is applied to Project truck trips. The Project is also expected to generate up to 12 worker trips during the AM and PM peak hours. Supply and equipment delivery trips are anticipated to be minimal and would not be expected to have a measurable influence on traffic operations.

Study Area and Evaluation Scenarios

This TIS evaluates traffic operations within a study area that includes four signalized intersections along Pleasant Valley Road (Lewis Road, Pancho Road, US 101 southbound ramps, and US 101 northbound ramps) and five road segments including two segments on Pancho Road, two segments on Pleasant Valley Road, and one segment of Santa Rosa Road. Each study location is evaluated for potential effects on traffic operations during the AM peak period and the PM peak period. In consideration of level of service standards of jurisdiction agencies (including Ventura County, the City of Camarillo, and California Department of Transportation [Caltrans]), level of service (LOS) "C" is considered the lowest acceptable level of service.

The following three scenarios are evaluated, each for conditions without and with the Project:

Existing Conditions



- Existing Plus Approved/Pending Projects
- Year 2030

Ventura County Traffic Impact Mitigation Fee Program and Ventura County-City of Camarillo Reciprocal Agreement

Ventura County has a Traffic Impact Mitigation Fee (TIMF) program established to collect fees from planned development projects for use in maintaining and improving County roads. The County also has a reciprocal agreement with the City of Camarillo through which the County and City have agreed to require development projects to pay traffic impact fees to the respective jurisdictions when projects would be located in one jurisdiction but would result in trips within the other jurisdiction. Because the study area roads are located within the City of Camarillo, it may be appropriate for the Project to provide funding toward both the County and the City of Camarillo's traffic mitigation fee programs. However, a determination of the specific traffic mitigation fee requirements of the Project is beyond the scope of this TIS.

Intersection Operations

Table E-1 summarizes the results of the traffic operations analysis at study area intersections for the evaluation scenarios. Results of the analysis show that the addition of Project-related trips to Existing Conditions would not cause or contribute to LOS D or worse conditions at study area intersections. However, the addition of Project-related trips to Existing Plus Approved/Pending Projects and Cumulative Year 2030 Without Project conditions on study area intersections would contribute to LOS D or worse conditions.

Table E-1
Intersection Operations

INTERSECTION	CONTROL	PEAK HOUR	EXISTING		EXISTING PLUS PROJECT		EXISTING PLUS APPROVED/ PENDING		EXISTING PLUS APPROVED/ PENDING PLUS PROJECT		CUMULATIVE YEAR 2030 WITHOUT PROJECT		CUMULATIVE YEAR 2030 PLUS PROJECT	
			ICU	LOS	ICU	LOS	ICU	LOS	ICU	LOS	ICU	LOS	ICU	LOS
1. Lewis Road / Pleasant Valley Road	Signalized	AM	62.4	В	62.6	В	63.8	В	64.2	С	77.9	D	78.3	D
1. Lewis Road / Fleasant Variey Road	Signanzeu	PM	65.4	С	65.8	С	66.3	С	66.7	С	80.2	D	80.6	D
2. Pancho Road / Pleasant Valley Road	Signalized	AM	58.5	В	61.3	В	60.0	В	62.8	В	69.5	С	72.3	С
2. Falletto Road / Fredsant valley Road	Signanzeu	PM	60.8	В	63.2	В	62.0	В	64.4	С	66.8	С	69.2	С
3. US Route 101 SB Ramps / Pleasant Valley Road	Signalized	AM	77.8	D *	77.8	D *	86.5	E	86.5	E	102.8	G	102.8	G
3. 03 Route 101 3B Ramps / Freasant Variey Road	Signanzeu	PM	62.0	В	62.7	В	69.8	С	69.8	С	98.1	F	98.1	F
4. US Route 101 NB Off Ramp / Pleasant Valley Road	Signalized	AM	47.4	Α	47.8	Α	53.2	Α	53.3	Α	69.4	С	69.4	С
a. 05 houte 101 hb on hamp / Fleasant valley hoad	Signalized	PM	54.2	Α	55.6	В	56.6	В	58.0	В	69.5	С	69.5	С

ICU = Intersection Capacity Utilization (expressed as a percentage)

For signalized controlled intersections, the LOS is based on the ICU method.

* Existing State highway facility is operating at less than the target LOS; the existing MOE shall be maintained

Project contributes to LOS D or worse (excluding U.S. 101 SB Ramps)*



Queuing Analysis

Table E-2 provides a queue length summary for left and right turn lanes at the study intersections for various study scenarios. The queuing analysis presented in this TIS is provided for informational purposes only and does not represent a CEQA impact.

Segments

Results of the study area roadway segment analysis are reflected in Table E-3. The performance criteria used for evaluating volumes and capacities on the road and highway system for this study were estimated using the Modified Arterial Level of Service Tables included in Appendix A. Results of the analysis show that the Project would contribute to existing deficient levels of service (LOS D or worse) on two segments of Pleasant Valley Road, one segment of Santa Rosa Road, and one segment of Pancho Road, and the Project would cause a deficient level of service (LOS D) on the northbound segment of Pancho Road between Calle Quetzal and Pleasant Valley Road during the AM peak hour. The Project would also contribute to unacceptable levels of service on these road segments and the study area segment of Santa Rosa Road.

Table E-2
Oueuing Operations

INTERSECTION		EXISTING QUEUE STORAGE LENGTH (ft)		EXISTING CONDITIONS		EXISTING PLUS PROJECT		EXISTING PLUS APPROVED/ PENDING		G PLUS OVED/ IG PLUS JECT	CUMULATIVE YEAR 2030 WITHOUT PROJECT		2030	TIVE YEAF PLUS DJECT
			AM	PM	AM	PM	AM	PM	AM	PM	AM	PM	AM	PM
			Queue	Queue	Queue	Queue	Queue	Queue	Queue	Queue	Queue	Queue	Queue	Queue
	NB Left	600	42	74	42	74	42	74	42	74	53	89	53	89
	NB Right	600	214	378	218	379	217	380	220	382	303	540	306	542
	SB Left	150	109	101	113	103	109	101	113	103	176	111	188	113
Lewis Road / Pleasant Valley Road	SB Right	275	170	224	170	224	203	239	203	239	208	262	208	262
	EB Left	2 @ 175	172	171	172	171	179	198	179	198	197	215	197	215
	EB Right	150	15	14	15	14	15	14	15	14	25	26	25	26
	WB Left	2 @ 200	333	266	337	268	345	282	348	283	532	308	535	310
	WB Left	175	185	224	188	226	185	224	188	226	203	247	207	248
	NB Left	2 @ 50	62	368	71	378	73	373	82	382	110	393	119	403
	NB Right	200	62	321	115	353	70	326	123	358	97	358	150	391
Pancho Road / Pleasant Valley Road	SB Left	100	22	8	22	8	22	8	22	8	27	12	27	12
rancilo koau / rieasant vaney koau	EB Left	225	9	22	9	22	9	22	9	22	14	28	14	28
	WB Left	2 @ 350		53	304	79	255	62	314	88	323	76	383	103
	W B Left	2 @ 330	243	33	304	79	233	02	314	00	323	76	303	103
	SB Left	125	14	8	14	8	14	8	14	8	18	21	18	21
	SB Right	1075	658	436	658	436	741	519	741	519	903	788	903	788
US 101 SB Ramps / Pleasant Valley Road	EB Left	2 @ 100	703	711	703	711	779	773	779	773	910	1023	910	1023
	EB Right	125	148	82	162	88	149	88	163	95	178	97	192	103
	WB Left	50	3	9	3	9	3	9	3	9	3	10	3	10
US 101 NB Off Ramp / Pleasant Valley Road	WB Left	350	202	343	245	363	203	349	247	370	321	383	364	404
03 101 ND OII Namp / Ticasant variey koau	WB Right	2 @ 200	350	495	350	495	428	564	428	564	590	631	590	631

Queue is measured in feet / BOLD denotes exceedance



Table E-3 **Segment Operations**

STREET SEGMENT	STREET SEGMENT SEGMENT DESCRIPTION		DIRECTION	ON EXISTING		EXISTING PLUS PROJECT		EXISTING PLUS APPROVED/ PENDING		EXISTING PLUS APPROVED/ PENDING PLUS PROJECT		CUMULATIVE YEAR 2030 WITHOUT PROJECT		CUMULATIVE YEAR 2030 PLU	
				VOLUME	LOS	VOLUME	LOS	VOLUME	LOS	VOLUME	LOS	VOLUME	LOS	VOLUME	LOS
Pleasant Valley Road			1												
	2 Lanes Divided	AM	EB	1,070	С	1,089	С	1,101	С	1,117	С	1,313	D	1,329	D
Lewis Road to Pancho Road		PM	EB	972	С	978	С	994	С	1,000	С	1,238	D	1,244	D
tewis notation and notation to the	2 Lanes Divided	AM	WB	966	С	977	С	1,003	С	1,014	С	1,283	D	1,294	D
	2 Lattes Divided	PM	WB	1,365	D	1,376	D	1,410	D	1,421	D	1,517	D	1,528	D
	21 20 11	AM	NB	1,149	С	1,213	С	1,198	С	1,262	D	1,266	D	1,330	D
	2 Lanes Divided	PM	NB	1,240	D	1,279	D	1,276	D	1,315	D	1,377	D	1,416	D
Pancho Road to US 101 SB Ramps		AM	SB	1,043	С	1,114	С	1,078	С	1,149	С	1,249	D	1,320	D
	2 Lanes Divided	PM	SB	1,085	С	1,117	С	1,137	С	1,169	С	1,245	D	1,277	D
Santa Rosa Road															
		AM	NB	1,819	С	1,823	С	2,038	D	2,042	D	2,312	D	2,316	D
	3 Lanes Divided	PM	NB	2,069	D	2,073	D	2,254	D	2,258	D	2,612	F	2,616	F
US 101 NB Ramps to Adolfo Road		AM	SB	2,355	D	2,361	D	2,570	F	2,576	F	3,231	F	3,237	F
	3 Lanes Divided	PM	SB	1,787	С	1,789	С	2,017	D	2,019	D	2,886	F	2,888	F
Pancho Road				_,	-	_,	_	_,	_	_,-,	_	_,		_,	
		AM	NB	150	С	225	D	173	С	248	D	250	D	325	D
	1 Lane Undivided	PM	NB	831	E	880	E	842	E	891	F	908	F	957	F
Pleasant Valley Road to Calle Quetzal		AM	SB	450	D	537	D	465	D	552	D	668	E	755	E
	1 Lane Undivided	PM	SB	140	С	178	С	161	С	199	D	268	D	306	D
		AM				-	-	-	_			69			
	1 Lane Undivided le Quetzal to Howard Road 1 Lane Undivided		NB	19	С	94	С	19	С	94	С		С	144	С
Calle Quetzal to Howard Road			NB	75	С	125	С	75	С	125	С	114	С	164	C
			SB	30	С	117	С	30	С	117	С	90	С	177	С
		PM	SB	24	С	62	С	24	С	62	С	88	С	126	С

BOLD denotes LOS standard has been exceeded.

Project causes LOS D.

Project contributes to LOS D or worse.



SUMMARY OF TRANSPORTATION IMPACTS

Appendix G of the CEQA Guidelines and the County's April 26, 2011, Initial Study Assessment Guidelines (ISAG) identify certain transportation-related topics for consideration during CEQA review. These issues include potential policy or land use plan conflicts, potential impacts associated with safety on public roads and private access driveways, potential impacts on bicycle and pedestrian circulation and safety, and potential impacts on transit operations.³ Each of these is discussed the sections below and in Section 4.0 of this TIS. (As discussed in the introduction, CEQA and the CEQA Guidelines as amended in 2018 also required that by July 1, 2020, CEQA lead agencies must evaluate transportation impacts in consideration of vehicle miles traveled or similar metric. This TIS does not include an evaluation of VMT associated with the Project, and it is anticipated that the County will separately address Project-related VMT in the EIR in consideration of SB 743 and CEQA Guidelines.)

Potential Conflict with a Program, Plan, Ordinance or Policy Addressing the Circulation System

Notwithstanding the recent CEQA amendments discussed in the introduction above, Ventura County, the City of Camarillo, and Caltrans seek to maintain acceptable levels of service along the highway, street, and road network. These agencies adopt minimum levels of service in an attempt to control congestion that may result as new development occurs. The traffic operations evaluation in this TIS as summarized above and discussed in detail in the main body of this TIS discusses the various level of service goals and policies of these agencies and evaluates predicted levels of service associated with various with-Project evaluation scenarios. An assessment of the Project's consistency with programs, plans, ordinances, and policies is beyond the scope of this TIS and it is anticipated that Project consistency will be addressed by the County in the EIR to be prepared for the Project.

Potential Impacts on Transit Services

Transit services within the City of Camarillo are served by Fixed Route, Dial-A-Ride and Ventura County Transportation Commission (VCTC) Intercity service. The Fixed Route service, provided by Camarillo Area Transit (CAT), does not include transit routes in the study area. The VCTC Intercity is a Countywide service, which connects Camarillo with Thousand Oaks, Oxnard and Ventura. The Oxnard/Camarillo/CSUCI route traverses Pleasant Valley Road along Lewis Road, with a stop located along Lewis Road just south of US 101. The additional Project trips would not interfere with these transit routes or stops and, thus, would not result in significant adverse effects on existing or planned transit facilities in the Project study area.

³ The ISAG also identifies Transportation Level of Service as an issue to consider, and levels of service are evaluated in detail in this TIS. The ISAG also identifies other transportation items associated with railroads, airports, harbor facilities, and pipelines; however, addressing those items is outside the scope of this TIS. 4 At the time of preparation of this TIS, agencies including Ventura County and Caltrans, are considering amendments to policies pertaining to congestion in efforts to implement and comply with the requirements of amendments to CEQA and the CEQA Guidelines pursuant to SB 743.



Potential Impacts on Bicycle and Pedestrian Safety and Circulation

Bicycling is considered an effective alternative mode of transportation that can help to improve air quality and reduce the number of vehicles traveling along existing highways, especially within the cities and unincorporated communities. The City of Camarillo Bikeway Master Plan identifies existing Class II bike lanes along the study segments of Pleasant Valley Road and Santa Rosa Road and a planned Class II bike lane along Pancho Road, which would be designed in accordance with City of Camarillo standards. Sidewalks presently exist along the north/west side of the Pleasant Valley Road study segment, both sides of the Santa Rosa Road study segment, and along the east side of Pancho Road.

The existing Class II bike lanes and pedestrian facilities crossing Lewis Road, Pancho Road, and US 101 NB and SB ramps, do so at traffic-controlled intersections. All of the study intersections evaluated in this TIS are signalized and include pedestrian signal phasing which accommodates pedestrians utilizing the crosswalk. Though traffic within the study area is expected to increase over time, these traffic control devices will help maintain pedestrian and bicycle safety within the study area. Class II bike lanes are identified in the City of Camarillo's General Plan Circulation Element on all study roadway segments, and it is anticipated that the City will retain and add Class II bike lanes on these segments sufficient to accommodate bicycle and pedestrian safety and circulation. The additional Project trips would not adversely affect existing or planned bicycle or pedestrian facilities in the Project study area.

Potential Impacts Associated with Hazards on Public Roads or Private Access Roads due to Design or Incompatible Uses

The proposed Project will not create any new design features on or off the Project site. The existing on-site circulation pattern will remain the same as the currently approved surface mining permit. Although there will be an increase in the volume of vehicles accessing the site during peak-hour periods and some of the incoming haul trucks will be loaded for delivery of recycle materials or fill material, the same types of vehicles (heavy-duty haul trucks and personal vehicles) will continue to access the site. The existing site access/egress is located at a sufficient distance from any intersection to allow for safe vehicular access/egress to and from the site. Therefore, this impact is considered less than significant, and no mitigation is required.

Potential Impacts Related to Emergency Access

The Project site is currently accessed/egressed via an existing entrance road from Howard Road, a private road that provides access to the Project site and to the Conejo Mountain Memorial Cemetery. Emergency access to the site would be unaffected by the Project. Therefore, this impact is considered less than significant, and no mitigation is required.



1.0 Introduction

This Transportation Impact Study (TIS) has been prepared for the purpose of identifying traffic operations and analyzing potential transportation-related impacts of the proposed Pacific Rock Quarry Expansion Project (Project). Pacific Rock, Inc. (Applicant) is requesting a Conditional Use Permit Modification to extend the life of the existing permitted mining operation by an additional 30 years, expand the mining area, extend the operational days from 6 to 7 days per week (adding Sunday for material load out) with additional material load out hours and limited extended 24 hour operations (60 days maximum per year), extend the daily hours of operation (for materials hauling) from the currently permitted 7:00 AM to 4:00 PM to the proposed 5:30 AM to 10:00 PM, allow construction and mobile mining equipment in outdoor storage areas, allow concrete and asphalt recycling, allow for imported material to be used as reclamation fill, and replace an existing mobile home to be used as a 24-hour security trailer.

1.1 Description of the Region/Project

The Project is located approximately two miles south of U.S. Highway 101 (US 101) in unincorporated Ventura County. Figures 1-1 and 1-2 show the location of the Project, major roadways and highways in the Project area, and the road segments and intersections evaluated in this TIS.

1.1.1 Project Access

Access to the Project site is provided by a gated private access road from Howard Road. Under existing operations, trucks leaving the site travel down Howard Road to Pancho Road then to Pleasant Valley Road from where they travel either turn left (west) and travel toward to Lewis Road or turn right (north) and travel toward State Highway 101 for delivery of aggregate materials to various destinations. Trucks traveling to the site use these same roads. The existing permit limits the daily number of haul trucks from the site to 60 loads, but does not prescribe haul truck routes or destinations. Limited information pertaining to existing operations hauling, routes, and destinations; however, the applicant has advised the County that material is generally delivered within Ventura, Los Angeles and Santa Barbara counties.³

1.1.2 Study Area

The study area includes intersections and roadway segments nearest the site and on which most Project-related vehicle trips would occur. Project-related vehicle trips would extend to other various intersections and road segments depending on the specific material destination and source locations. The study area for this analysis focuses on the intersections and segments with

^{3 &}quot;Trucks leave the site and travel down Howard Road to Lewis Road; Lewis Road to State Highway 101 for delivery to Ventura, Los Angeles and Santa Barbara Counties." (Sespe Consulting, "Project Description-Pacific Rock Quarry Conditional Use Permit Modification Application LU10-0003", pg. 3. April 1, 2019.)



the highest anticipated and reasonably foreseeable potential for Project-related transportation effects, and is considered sufficient for the purposes of this TIS. The following intersections and roadway segments are evaluated in this TIS:

Intersections

- 1. Lewis Road / Pleasant Valley Road
- 2. Pancho Road / Pleasant Valley Road
- 3. US 101 SB Ramps / Pleasant Valley Road
- 4. US 101 NB Off Ramp / Pleasant Valley Road

Roadway Segments

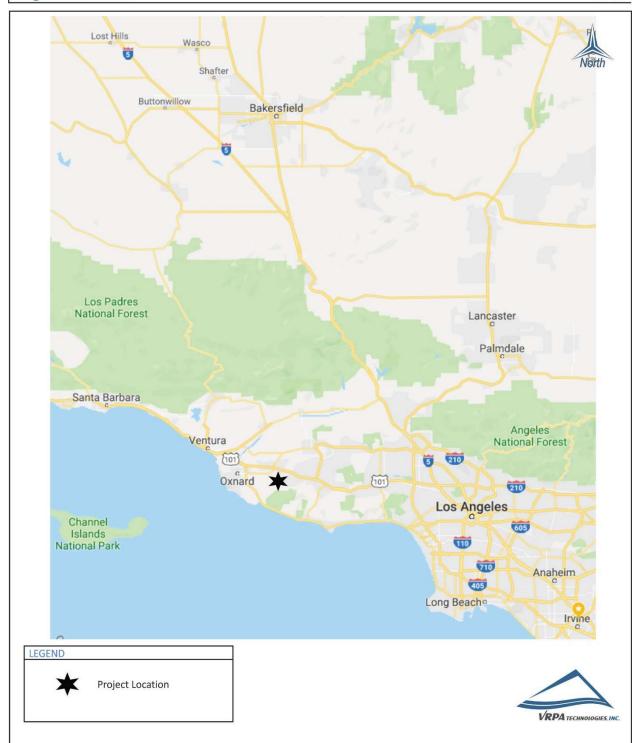
- 1. Pleasant Valley Road
 - Between Lewis Road and Pancho Road
 - Between Pancho Road and US 101 SB Ramps
- 2. Santa Rosa Road
 - Between US 101 NB Ramps and Adolfo Road
- 3. Pancho Road
 - Between Pleasant Valley Road and Calle Quetzal
 - Between Calle Quetzal and Howard Road

As shown on Figure 1-2, Howard Road provides direct access between the Project site and the southern end of Pancho Road. Howard Road also provides access to the Conejo Mountain Memorial Cemetery and a small number of agricultural parcels in the immediate area. Traffic volumes on Howard Road are minimal (less than 100 trips in the AM and PM peak hours) based upon twenty-four (24) hour classification counts collected in the study area as noted in Section 2.0. As a result, the Project would not result in the potential to cause levels of service on this segment to decline below acceptable conditions. Therefore, Howard Road is not evaluated as a study area roadway segment in this TIS.



Pacific Rock Quarry Expansion Project Regional Location

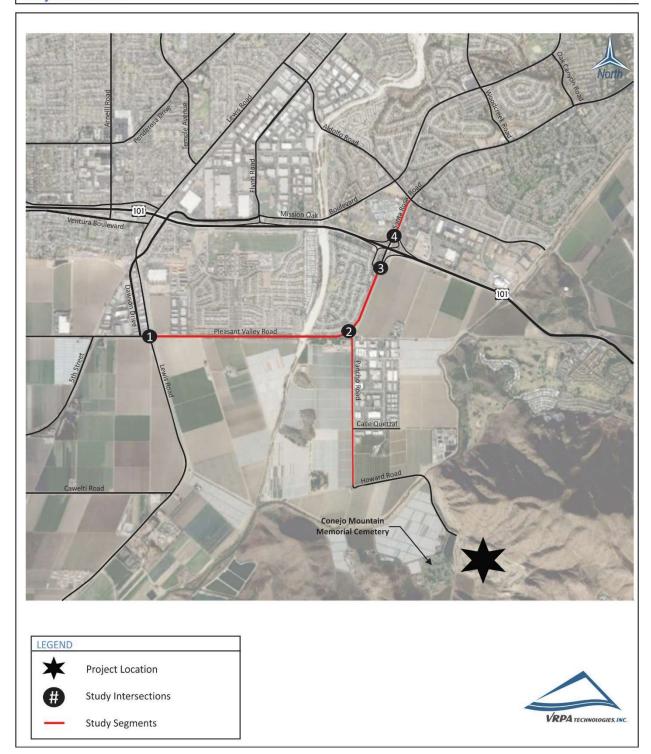
Figure 1-1





Pacific Rock Quarry Expansion Project Project Location

Figure 1-2





1.1.3 Study Scenarios

This TIS includes Level of Service (LOS) analyses for the following scenarios:

- Existing Conditions
- Existing Plus Project
- Existing Plus Approved/Pending
- Existing Plus Approved/Pending Plus Project
- Cumulative Year 2030 Without Project
- Cumulative Year 2030 Plus Project

1.2 Methodology

The sections below discuss the methods used in this TIS for analyzing street and intersection capacities and changes in levels of service for the study scenarios listed above. Intersection turning movement counts and roadway geometrics used for the analysis were obtained from field review findings and vehicle count data as described further in Section 2.1.

1.2.1 Intersection Analysis

Intersection analysis was conducted using Intersection Capacity Utilization (ICU) methodology to determine intersection levels of service for the study intersections under the various study scenarios. Thus, the 2003 ICU Worksheets for signalized intersections was used to determine the volume-to-capacity (V/C) ratio and the associated level of service (LOS) for each intersection. Traffic signal timing sheets for each of the study intersections were obtained from City of Camarillo and Caltrans staff and were incorporated into the 2003 ICU Worksheets accordingly.

Table 1-1 indicates the ICU LOS, which is based upon the critical flow ratio for the intersection. Associated levels of service ranging from LOS "A" to "H" are provided below with the corresponding Maximum ICU.

1.2.2 Roadway Segment Analysis

Roadway segment evaluation was performed for this TIS to assess the potential for the Project to cause or contribute to an exceedance of acceptable segment capacity under the various study scenarios. The Highway Capacity Manual (HCM)⁴, categorizes roadway segment levels of service based on two parameters of traffic: uninterrupted and interrupted flow. Uninterrupted flow facilities do not have fixed elements such as traffic signals that cause interruptions in traffic flow. Interrupted flow facilities do have fixed elements that cause an interruption in the flow of traffic, such as stop signs and signalized intersections along arterial roads. A roadway segment is defined

^{4 &}quot;Highway Capacity Manual, Sixth Edition: A Guide for Multimodal Mobility Analysis" (Transportation Research Board, 2016)



as a stretch of roadway generally located between signalized or controlled intersections. The roads evaluated in this TIS are considered interrupted flow facilities, and each study roadway segment is located between two signalized intersections.

Table 1-2 provides a definition of segment LOS based on the HCM interrupted flow facilities criteria. Street segment capacity was determined using information shown in Table 1-3 which is based on the LOS Tables included in Appendix A. The tables consider the capacity of individual road segments based on numerous roadway variables (design speed, passing opportunities, signalized intersections per mile, number of lanes, saturation flow, etc.). These variables were identified and applied to study roadway segments to reflect segment LOS conditions.

Table 1-1
ICU LOS Thresholds

100 103 111	
LEVEL OF SERVICE	MAXIMUM ICU
А	55%
В	64%
С	73%
D	82%
E	91%
F	100%
G	109%
н	over 109%



Table 1-2 Roadway Segment Level of Service Definitions (Highway Capacity Manual)

LEVEL OF SERVICE	DEFINITION	
А	Represents free flow. Individual vehicles are virtually unaffected by the presence of others in the traffic stream.	
В	Is in the range of stable flow, but the presence of other vehicles in the traffic stream begins to be noticeable. Freedom to select desired speeds is relatively unaffected, but there is a slight decline in the freedom to maneuver.	
С	Is in the range of stable flow, but marks the beginning of the range of flow in which the operation of individual vehicles becomes significantly affected by interactions with other vehicles in the traffic stream.	
D	Is a crowded segment of roadway with a large number of vehicles restricting mobility and a stable flow. Speed and freedom to maneuver are severely restricted, and the driver experiences a generally poor level of comfort and convenience.	
E	Represents operating conditions at or near the level capacity. All speeds are reduced to a low, but relatively uniform value. Small increases in flow will cause breakdowns in traffic movement.	
F	Is used to define forced or breakdown flow (stop-and-go gridlock). This condition exists when the amount of traffic approaches a point where the amount of traffic exceeds the amount that can travel to a destination. Operations within the queues are characterized by stop and go waves, and they are extremely unstable.	



Peak Hour Directional volumes – Orban										
	Level of Service									
Lanes Divided B C D										
State Roadways										
1	Undivided	*	200	690	930					
1	Divided	*	210	725	977					
2	Divided	50	1,350	1,790	1,870					
3	Divided	80	2,040	2,690	2,820					
		Non-State R	oadways							
1	Undivided	*	180	621	837					
1	Divided	*	190	656	884					
2	Divided	45	1,215	1,611	1,683					
3	Divided	72	1,836	2,421	2,538					

Table 1-3
Peak Hour Directional Volumes – Urban

Source: 2018 FDOT Quality/Level Of Service Handbook Tables

1.2.3 Agency Level of Service Standards

The intersections and roadway segments evaluated in this TIS are located either in the City of Camarillo or on the boundary of the City of Camarillo and unincorporated Ventura County, and the U.S. 101 ramps are under the jurisdiction of the California Department of Transportation (Caltrans). As a result of the combination of jurisdictional agencies, this TIS considers adopted level of service standards of these various agencies in assessing whether predicted operations with the Project would be within the range of levels of service considered acceptable to these agencies. The traffic operations analysis in this TIS uses the traditional practice of measuring delay, vehicle/capacity ratios, and levels of service for informational purposes. Ultimately, the County will determine the proper characterization of this information in the EIR for consideration by decisionmakers. Thus, although Project effects on traffic delay are presented in this TIS and measured against "acceptable" levels of service, these effects should not be interpreted as an environmental "impact" under CEQA.

Ventura County General Plan Goals, Policies & Programs (03-19-19 edition) Transportation and Circulation section specifies minimum acceptable levels of service for road segments and intersections that identifies, in relevant part, LOS "D" as the minimum acceptable level of service for County thoroughfares and Federal and State highways in unincorporated areas of the County and LOS "C" as the minimum acceptable level of service for all County maintained local roads. The City of Camarillo General Plan Circulation Element (2014), Policy 1.2.6 states, "The City should maintain a level of service (LOS) of "C" or better on all streets and intersections. Brief periods of LOS "D" during peak a.m. and p.m. traffic hours may be tolerated where improving to LOS "C" would be unreasonably costly."



^{*} Cannot be achieved using table input value defaults

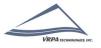
Caltrans, "A Guide For the Preparation of Traffic Impact Studies, 2002" identifies a minimum LOS of "C" as the minimum acceptable level of service for its facilities, except where the existing LOS is "D" or below, in which case Caltrans generally seeks to maintain the existing LOS.

Table 1-4 summarizes the minimum acceptable LOS for each intersection and roadway segment based upon its jurisdictional location. In consideration of these various agency level of service standards, this TIS uses LOS C as the minimum acceptable level of service for all study intersections and roadway segments, unless the existing condition (discussed further in Chapter 2) is worse than LOS C in which case the existing condition LOS is used as the minimum acceptable level of service.

Table 1-4
Minimum Acceptable LOS by Jurisdiction

INTERSECTION	JURISDICTION	MINIMUM ACCEPTABLE LOS		
1. Lewis Road / Pleasant Valley Road	City of Camarillo	С		
1. Lewis Road / Pleasant Valley Road	Ventura County	D		
2. Pancho Road / Pleasant Valley Road	City of Camarillo	С		
3. US Route 101 SB Ramps / Pleasant Valley Road	Caltrans	LOS C or existing LOS if worse than C		
4. US Route 101 NB Off Ramp / Pleasant Valley Road	Caltrans	LOS C or existing LOS if worse than C		
ROADWAY SEGMENT	JURISDICTION	MINIMUM ACCEPTABLE LOS		
Pleasant Valley Road				
Lewis Road to Pancho Road	City of Camarillo	С		
Lewis Road to Falicilo Road	Ventura County	D		
Pancho Road to US 101 SB Ramps	City of Camarillo	С		
Santa Rosa Road				
US 101 NB Ramps to Adolfo Road	City of Camarillo	С		
Pancho Road				
Pleasant Valley Road to Calle Quetzal	City of Camarillo	С		
rieasant valley roau to Calle Quetzal	Ventura County	D		
Calle Quetzal to Howard Road	City of Camarillo	С		
carre quezar to noward hoad	Ventura County	D		

Level of service standards for study area jurisdictional agencies



2.0 Existing Conditions

2.1 Existing Conditions Traffic Counts and Roadway Geometrics

To assess existing traffic conditions, AM and PM peak hour turning movements were collected at each study intersection by National Data and Surveying Services. Intersection turning movement counts were conducted for the periods of 7:00-9:00 AM and 4:00-6:00 PM for all study intersections on Tuesday, November 27, 2018. Traffic count data worksheets are provided in Appendix B. Twenty-four (24) hour classification counts were also collected on Tuesday, November 27, at three locations in within the study area to identify existing truck travel patterns in the study area. The days on which counts were taken are considered sufficiently representative of typical traffic volumes within the study area. Schools were in session and weather was mild.

The existing lane geometry at study area intersections is shown in Figure 2-1 and was determined through field reconnaissance. Figures 2-2 and 2-3 show existing traffic volumes for the AM and PM peak hours in the study area. The traffic volumes include all background trips (i.e., those trips not associated with existing Pacific Rock Quarry operations) as well as any trips associated with Pacific Rock Quarry operations on the days and during the period counts were taken. The Operator advised the County that on November 27, 2018 (the day traffic counts were taken) there were 3 worker trips to and 3 worker trips from the site (resulting in 6 total worker trips) and 9 haul loads from the site (resulting in 18 total trips when trips to the site by unloaded trucks are included). However, the time of day that these trips occurred was not provided by the Operator and it is undetermined whether these trips are within peak-hour counts. To more conservatively evaluate changes in levels of services associated with the Project (i.e., tending to overstate changes as opposed to understating changes), it is assumed for this TIS that any Project-related trips that occurred during the period when counts were taken did not occur during the AM or PM peak hour periods. The traffic counts taken in 2018 are considered representative of baseline peak-hour traffic conditions and are appropriate for this evaluation.

2.2 Existing Functional Roadway Classifications System

Functional roadway classification is the process by which streets and highways are grouped into classes, or systems, according to their design and the type of service they are intended to provide. Fundamental to this process is the recognition that individual streets and highways do not serve travel independently; rather, most travel involves movement through a network of roads. The following summarizes classifications relevant to this analysis and describes the study area roadways and their classifications based on the City of Camarillo General Plan Circulation Element.

• **Freeways** – Limited-access facilities designed for high speed regional mobility. Freeways may include up to eight lanes (four lanes in each direction). The one study segment freeway is:

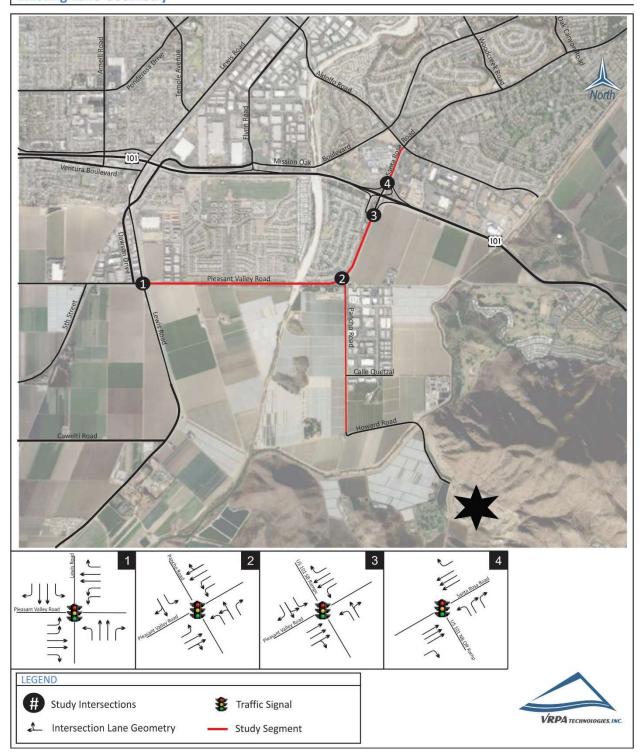


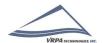
- o **US 101 (Ventura Freeway)** currently exists as a six-lane freeway with a posted speed limit of 65 miles per hour (mph) near the study area. According to the Caltrans website, the annual average daily trips (AADT) along US 101 in 2017 (most recent available year at the time of preparation of this TIS) was 132,000.
- Arterial Streets which provide for the maximum movement of large volumes of traffic between major traffic generators. The City of Camarillo Circulation Element recognizes two classes of arterial streets: primary and secondary. The three study segments that are arterials:
 - **Lewis Road** is a four-lane divided roadway with bike lanes in the study area.
 - **Pleasant Valley Road** is a four-lane divided roadway with bike lanes in the study area both east and west of the Pancho Road intersection.
 - Santa Rosa Road is a 6-lane divided roadway with bike lanes in the study area north of US 101.
- Collectors Streets which provide access and movement between residential, commercial, and industrial areas. The primary function of collector streets is to collect and distribute traffic between local streets and the arterial roadway system. The City of Camarillo Circulation Element recognizes three types of collector street: major, minor, and industrial. The one study segment that falls under the Collector classification is designated by the City of Camarillo Circulation Element as an Industrial Collector, as defined below.
- Industrial Collector Streets that are intended as the intermediate route to accommodate traffic between local industrial streets and arterial streets. This system includes those streets that provide for traffic movements within a relatively small area, such as a commercial or industrial zone. Individual streets are designed specifically to facilitate truck traffic, which is an element of the industrial district. The one study segment Industrial Collector is:
 - Pancho Road connects Howard Road and Pleasant Valley Road. Pancho Road is a four-lane undivided roadway from Pleasant Valley Road to just south of Adohr Lane. Pancho Road is a three-lane undivided roadway (Two-Way-Left-Turn-Lane) without bike lanes from just south of Adohr Lane to Calle Alto. Pancho Road is a two-lane undivided roadway (Two-Way-Left-Turn-Lane) with parallel parking on the eastside of the roadway from Calle Alto to Calle Quetzal. Finally, Pancho Road is a two-lane undivided roadway without bike lanes from Calle Quetzal to Howard Road. At its intersection with Pleasant Valley Road, Pancho Road includes two left-turn lanes, one of which is also a through lane, and a right-turn lane.
- Local Streets Roadways which provide access to individual homes and businesses. Local streets should not carry through traffic or buses and heavy trucks, except in commercial and industrial districts. None of the study segments are local streets.



Pacific Rock Quarry Expansion Project Existing Lane Geometry

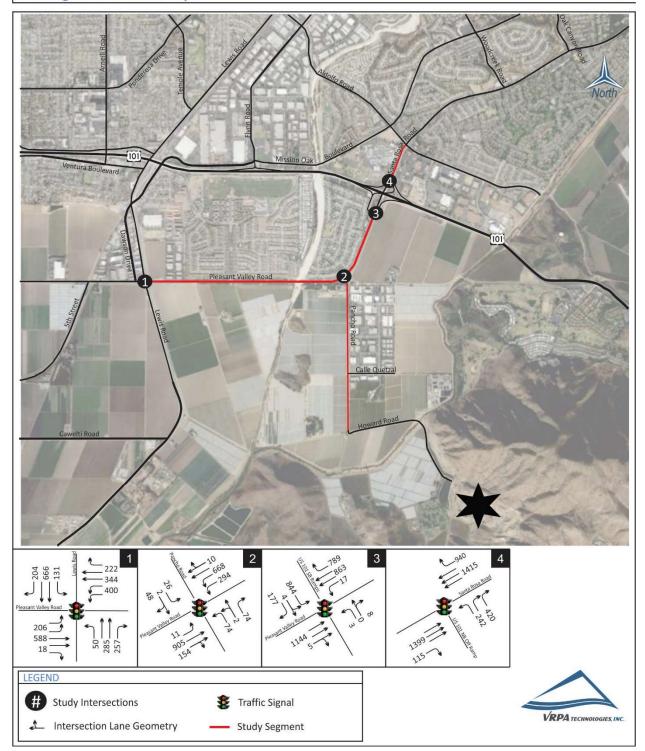
Figure 2-1





Pacific Rock Quarry Expansion Project Existing AM Peak Hour Trips

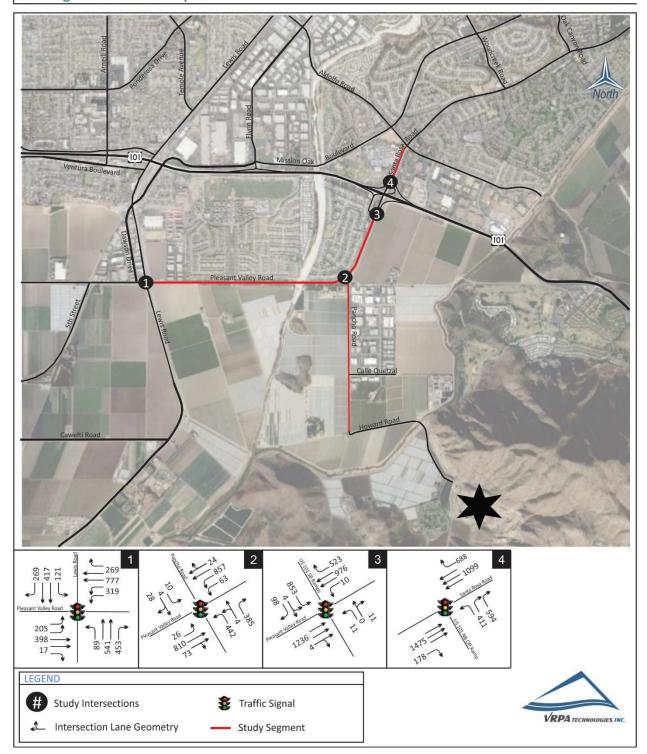
Figure 2-2





Pacific Rock Quarry Expansion Project Existing PM Peak Hour Trips

Figure 2-3





2.3 Affected Streets and Highways

Street and highway intersections and segments near and adjacent to the Project site were analyzed to determine levels of service using methodologies described previously. The study intersections and street and highway segments included in this TIS are listed below.

Intersections

- 1. Lewis Road / Pleasant Valley Road
- 2. Pancho Road / Pleasant Valley Road
- 3. US 101 SB Ramps / Pleasant Valley Road
- 4. US 101 NB Off Ramp / Pleasant Valley Road

Roadway Segments

- Pleasant Valley Road
 - Lewis Road and Pancho Road
 - Pancho Road to US 101 SB Ramps
- 2. Santa Rosa Road
 - US 101 NB Ramps and Adolfo Road
- 3. Pancho Road
 - Pleasant Valley Road to Calle Quetzal
 - Calle Quetzal to Howard Road

2.4 Level of Service

2.4.1 Intersection Capacity Analysis

Study intersection LOS analyses were assessed using 2003 ICU methodology. The roadway geometrics, traffic volumes, and signal timing properties (lost time, minimum green time, etc.) of each intersection were input into the 2003 ICU Worksheets in order to determine the LOS for each study scenario. The intersection reported LOS represents the ICU methodology.

Results of the analysis show that under Existing Conditions all of the study intersections are LOS C or better and meet the minimum acceptable level of service criteria during both the AM and PM peak hour, with the exception of the US 101 SB Ramps / Pleasant Valley Road intersection during the AM peak hour. Based on this analysis, this intersection operates at LOS "D" during the AM peak hour under Existing Conditions. Table 2-1 shows the intersection LOS for Existing Conditions. ICU Worksheets are provided in Appendix C.



2.4.2 Queuing Analysis

Table 2-2 provides a queue length summary for left and right turn lanes at the study intersections for Existing Conditions. Queuing analysis was completed using Section 400 of Caltrans' Highway Design Manual. The vehicular queue presented in Table 2-2 represents the approximate queue length requirements for the respective lane movements under Existing Conditions. As shown in Table 2-2, under Existing Conditions, the queue lengths for certain movements at each of the study intersections exceed the existing queue lane storage lengths. It should be noted that the queuing analysis presented in this traffic study is provided for informational purposes only.

2.4.3 Roadway Segment Capacity Analysis

Results of the segment analysis for Existing Conditions are presented in Table 2-3. The performance criteria used for evaluating volumes and capacities on the road and highway system for this study were estimated using the Arterial Level of Service Tables included in Table 1-3 and Appendix A. Results of the analysis show six instances in which LOS "C" is exceeded under Existing Conditions as summarized below:

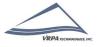
- Pleasant Valley Road westbound from Pancho Road to Lewis Road during the PM peak hour (LOS D)
- Pleasant Valley Road northbound between Pancho Road and US 101 southbound ramps during the PM peak hour (LOS D)
- Santa Rosa Road northbound between US 101 northbound ramps and Adolfo Road during the PM peak hour (LOS D)
- Santa Rosa Road southbound between Adolfo Road and US 101 northbound ramps during the AM peak hour (LOS D)
- Pancho Road northbound between Calle Quetzal and Pleasant Valley Road during the PM peak hour (LOS E)
- Pancho Road southbound between Pleasant Valley Road during the AM peak hour (LOS D)



Table 2-1 Existing Intersection Operations

INTERSECTION	CONTROL	PEAK HOUR	EXISTING											
			ICU	LOS										
1. Lewis Road / Pleasant Valley Road	ant Valley Road Signalized		62.4	В										
1. Lettis nodd / redsam variey nodd	Signanzeu	PM	65.4	С										
2. Pancho Road / Pleasant Valley Road	Signalized	AM	58.5	В										
2. Tantilo Roda / Treasunt valley Roda	Signanzeu	PM	60.8	В										
3. US Route 101 SB Ramps / Pleasant Valley Road	Signalized	AM	77.8	D *										
3. 03 Noute 1013B Namps / Freasant variety Noad	Signanzeu	PM	62.0	В										
4. US Route 101 NB Off Ramp / Pleasant Valley Road	Signalized	AM	47.4	Α										
4. 05 Noute 101 No on Namp / Fleasant variey Noad	Signanzeu	PM	54.2	Α										

ICU = Intersection Capacity Utilization (expressed as a percentage) / BOLD denotes LOS has been exceeded For signalized controlled intersections, the LOS is based on the ICU method.



^{*} Existing State highway facility is operating at less than the target LOS; the existing MOE shall be maintained.

Table 2-2 **Existing Queuing Operations**

INTERSECTION	EXISTING (STORAGE LEN		EXISTING CONDITIONS		
			AM Queue	PM Queue	
	NB Left	600	42	74	
	NB Right	600	214	378	
	SB Left	150	109	101	
Louis Bood / Bloomatt/ellouBood	SB Right	275	170	224	
Lewis Road / Pleasant Valley Road	EB Left	2 @ 175	172	171	
	EB Right	150	15	14	
	WB Left	2 @ 200	333	266	
	WB Left	175	185	224	
	NB Left	2 @ 50	62	368	
	NB Right	200	62	321	
Pancho Road / Pleasant Valley Road	SB Left	100	22	8	
	EB Left	225	9	22	
	WB Left	2 @ 350	245	53	
	SB Left	125	14	8	
	SB Right	1075	658	436	
US 101 EB Off Ramp / Pleasant Valley Road	EB Left	2 @ 100	703	711	
	EB Right	125	148	82	
	WB Left	50	3	9	
US 101 WB Off Ramp / Pleasant Valley Road	WB Left	350	202	343	
03 101 WD On Namp / Fleasant varies Noad	WB Right	2 @ 200	350	495	
Out of the state o					

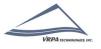
Queue is measured in feet / **BOLD** denotes exceedance



Table 2-3 Existing Segment Operations

Existing Segment Operations									
STREET SEGMENT	SEGMENT DESCRIPTION	PEAK HOUR	DIRECTION	EXISTIN					
Pleasant Valley Road				VOLOIVIE	LO3				
		AM	EB	1,070	С				
	2 Lanes Divided	PM	EB	972	С				
Lewis Road to Pancho Road	2 La mara Dividad	AM	WB	966	С				
	2 Lanes Divided	PM	WB	1,365	D				
	2 Lanes Divided	AM	NB	1,149	С				
Danaha Dand to US 101 CD Damas	2 Lanes Divided	PM	NB	1,240	D				
Pancho Road to US 101 SB Ramps	2 Lanes Divided	AM	SB	1,043	С				
	2 Lanes Divided	PM	SB	1,085	С				
Santa Rosa Road									
	3 Lanes Divided	AM	NB	1,819	С				
US 101 NB Ramps to Adolfo Road	3 Lanes Divided	PM	NB	2,069	D				
OS TOT NB Kamps to Adono Road	3 Lanes Divided	AM	SB	2,355	D				
	5 Laties Divided	PM	SB	1,787	С				
Pancho Road									
	1 Lane Undivided	AM	NB	150	С				
Pleasant Valley Road to Calle Quetzal	1 Lane Ondivided	PM	NB	831	E				
reasont variey hoad to carre Quetzar	1 Lane Undivided	AM	SB	450	D				
	I Lane Ondivided	PM	SB	140	С				
	1 Lane Undivided	AM	NB	19	С				
Calle Quetzal to Howard Road	1 Lane Ondivided	PM	NB	75	С				
carre Quetzar to rioward noad	1 Lane Undivided	AM	SB	30	С				
	1 Lane Ondivided	PM	SB	24	С				

BOLD denotes LOS standard has been exceeded.



3.0 Traffic Operations

This chapter provides an assessment of the vehicle trips the Project is expected to generate and the resulting predicted changes in traffic operations levels of service at study area road segments and intersections.

3.1 Trip Generation

To assess Project changes in traffic operations, the first step is to determine Project trip generation. The Project's trip generation was estimated based on information in the CUP amendment application. The Project's estimated AM peak hour and PM peak hour trips are shown in Table 3-1.

Proposed modifications to the existing CUP include: extend the life of the existing permitted operations by an additional 30 years, expand the mining area, extend the operational days from 6 to 7 days per week (adding Sunday for material load out) with additional material load out hours and limited extended 24 hour operations (60 days maximum per year), extend the daily hours of operation (for materials hauling) from the currently permitted 7:00 AM to 4:00 PM to the proposed 5:30 AM to 10:00 PM, allow construction and mobile mining equipment in outdoor storage areas, allow concrete and asphalt recycling, allow for imported material to be used as reclamation fill, and replace an existing mobile home to be used as a 24-hour security trailer.

The operation is currently permitted to transport up to 60 daily loads from the site (resulting 120 one-way trips), and the Project would not change this permitted daily maximum. The applicant has advised the County that the existing operation can generate up to 30 loads per hour (resulting in 60 one-way trips) during morning operations and up to 15 loads per hour (resulting 30 oneway trips) during afternoon periods. The County does not have sufficient information documenting actual AM peak-hour trips associated with the existing operation to establish baseline AM peak-hour trips, therefore, this study evaluates the Project as if all trips associated with haul trucks during the AM peak-hour period would be new trips that do not currently occur under existing operations. This approach is conservative inasmuch as it will tend to overestimate changes in traffic operations associated with the Project during the AM peak-hour period. Further, because the existing operation is permitted for operations between 7:00 AM and 4:00 PM, there are no baseline haul trips during the PM peak-hour period of 4:00 PM to 6:00 PM under Existing Conditions. Thus, this study evaluates changes in traffic operations associated with the Project's 15 loads (30 one-way trips) during the PM peak-hour period. In addition to trucks that would be used for transporting aggregate from the site, the Project would also involve truck trips associated with the delivery of asphalt and concrete to the site for recycling and for the delivery of fill material that would be used for reclamation. The application advises that these recycle material and fill import truck trips would be included within the requested permitted maximum of 60 truck loads per day. Thus, this analysis assumes that no more than 120 daily one-way haul truck trips would occur each day, consisting of up to 60 loaded trucks exiting or entering the



facility, and 60 empty trucks exiting or entering the facility. On an hourly basis, this evaluation assumes that no more than 60 AM peak-hour one-way truck trips (up to 30 loaded trucks entering or exiting the facility and 30 empty trucks entering or exiting the facility) and that no more than 30 PM peak-hour one-way truck trips (up to 15 loaded trucks entering or exiting the facility and 15 empty trucks entering or exiting the facility) would occur as a result of the Project.

Due to the size and weight of haul trucks, they operate more slowly than passenger vehicles. Therefore, a "Passenger Car Equivalent" (PCE) factor is applied to haul truck trips to account for the greater effect each truck has on traffic as compared to a passenger vehicle. For this evaluation, a PCE of 2.5 is used for Project-related haul truck trips. Thus, for the modeling, the truck trips shown in Table 3-1 are multiplied by 2.5, resulting in the total Project trips with PCE as also presented in the table.

Additional Project-related trips would be associated with ancillary delivery of supplies and equipment to the site periodically and worker trips. Supply and equipment delivery trips are anticipated to be minimal and would not be expected to substantively influence on traffic operations. Worker trips are accounted for and for this analysis assume up to 12 worker trips to the site during the AM peak hour and 12 worker trips from the site during the PM peak hour.

Table 3-1Project Trip Generation

	А	M PEA	K HOU	R	PM PEAK HOUR			JR
TRIP TYPE			IN:OUT		VOLU	IME		
	SPLIT	IN	ОUТ	TOTAL	SPLIT	IN	OUT	TOTAL
Truck Trips	50:50	30	30	60	50:50	15	15	30
Employee Trips	100:0	12	0	12	0:100	0	12	12
TOTAL DROJECT TRIDE		40	20	70		4=		40
TOTAL PROJECT TRIPS		42	30	72		15	27	42
TOTAL PROJECT TRIPS W/PC	E ¹	87	75	162		38	50	88

A "trip" is defined as a "one-way" trip 1 PCE of 2.5:1 was applied to truck trips

3.2 Trip Distribution

Project-related truck trip distribution is estimated based on consideration of the anticipated market for aggregate materials produced at the site and anticipated source locations for imported fill and recycle material (i.e., various locations primarily in Ventura, Los Angeles, and Santa Barbara counties), engineering judgement, prevailing traffic patterns in the study area, primary roads and travel routes, and population centers. Employee trip distribution is estimated considering population centers and local road system travel options. Thus, the employee trip distribution varies from the truck trip distribution, reflecting employee travel to and from the site.



The trip distribution estimates are intended to reflect anticipated typical travel patterns associated with Project-related vehicles. It is recognized that travel patterns will vary depending largely on market demand and the locations of aggregate deliveries to construction sites that cannot be presently determined. The distribution used here provides for a reasonable estimate of typical travel patterns appropriate for this TIS. Project trip distribution was assigned to the roadway system using the trip distribution percentages shown in Figure 3-1.

3.3 Project Trips and Distribution Summary

Project trips as shown in Table 3-1 were distributed to the roadway system using the trip distribution percentages shown in Figure 3-1. A graphical representation of the resulting AM and PM peak hour Project trips is shown in Figures 3-2 and 3-3. (Figures 3-2 and 3-3 include the PCE of 2.5 for Project truck trips, as discussed previously.)

3.4 Existing Plus Project Traffic Conditions

An Existing Plus Project scenario was analyzed to include existing traffic plus traffic generated by the Project. Existing Plus Project traffic conditions during the AM and PM peak hours are shown in Figures 3-4 and 3-5.

3.4.1 Existing Plus Project Intersection Capacity Analysis

Table 3-2 summarizes traffic operations under Existing Conditions without the Project and under Existing Conditions with the Project. As shown in the table, the addition of Project-related trips to Existing Conditions at study area intersections would not result in deficient levels of service. Results of the analysis show that all of the study intersections meet the minimum acceptable level of service criteria during both the AM and PM peak hour with the addition of Project related trips.

3.4.2 Existing Plus Project Queuing Analysis

Table 3-3 summarizes queuing operations under Existing Conditions without the Project and under Existing Conditions with the Project. Queuing analysis was completed using Section 400 of Caltrans' Highway Design Manual. As discussed in Section 2.0, the queuing analysis presented in this TIS is provided for informational purposes only. The City of Camarillo, Ventura County, and Caltrans have not established CEQA impact significance criteria related to the exceedance of left and right turn storage pockets.

3.4.3 Existing Plus Project Roadway Segment Capacity Analysis

Table 3-4 summarizes traffic operations under Existing Conditions without the Project and under Existing Conditions with the Project. As shown in the table, the addition of Project-related trips to Existing Conditions on study area roadway segments would contribute trips in six instances in which segment volumes already exceed LOS C and would result in one instance in which the



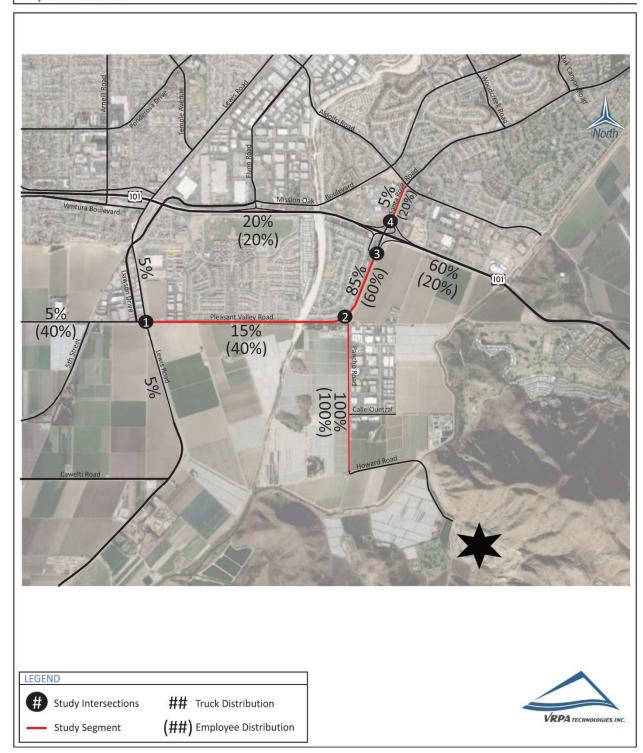
addition of Project trips would cause LOS C to worsen to LOS D, as summarized below:

- Pleasant Valley Road westbound from Pancho Road to Lewis Road during the PM peak hour (worsen existing LOS D)
- Pleasant Valley Road northbound between Pancho Road and US 101 southbound ramps during the PM peak hour (worsen existing LOS D)
- Santa Rosa Road northbound between US 101 northbound ramps and Adolfo Road during the PM peak hour (worsen existing LOS D)
- Santa Rosa Road southbound between Adolfo Road and US 101 northbound ramps during the AM peak hour (worsen existing LOS D)
- Pancho Road northbound between Calle Quetzal and Pleasant Valley Road during the AM peak hour (degrade from LOS C to LOS D)
- Pancho Road northbound between Calle Quetzal and Pleasant Valley Road during the PM peak hour (worsen existing LOS E)
- Pancho Road southbound between Pleasant Valley Road during the AM peak hour (worsen existing LOS D)



Pacific Rock Quarry Expansion Project Trip Distribution

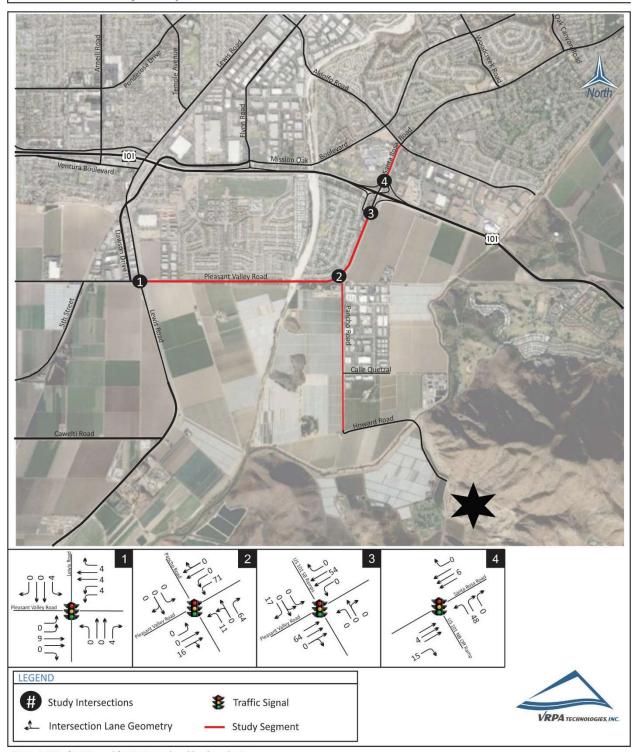
Figure 3-1





Pacific Rock Quarry Expansion Project AM Peak Hour Project Trips

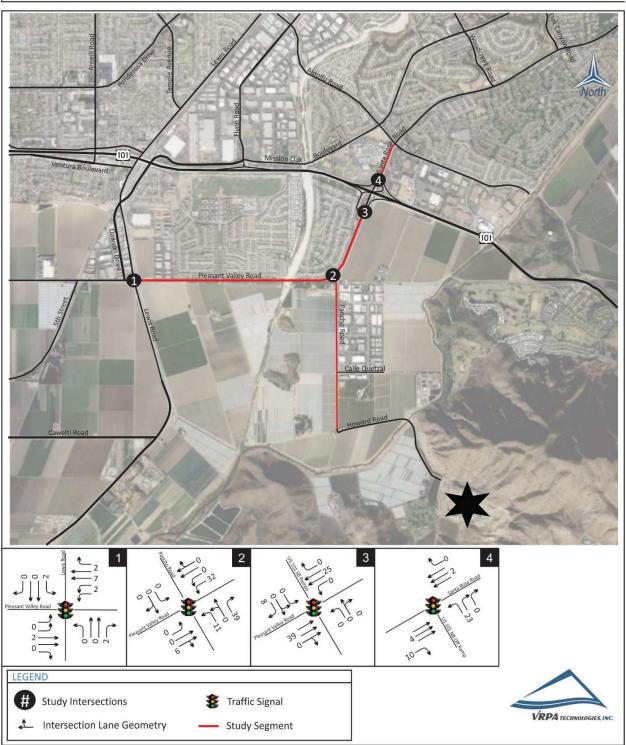
Figure 3-2





Pacific Rock Quarry Expansion Project PM Peak Hour Project Trips

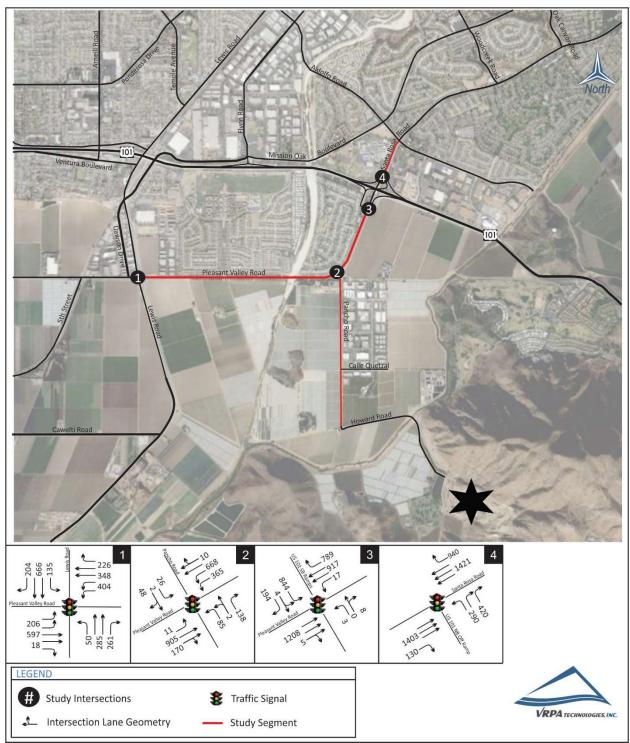
Figure 3-3





Pacific Rock Quarry Expansion Project Existing Plus Project AM Peak Hour Trips

Figure 3-4





Pacific Rock Quarry Expansion Project Existing Plus Project PM Peak Hour Trips

Figure 3-5

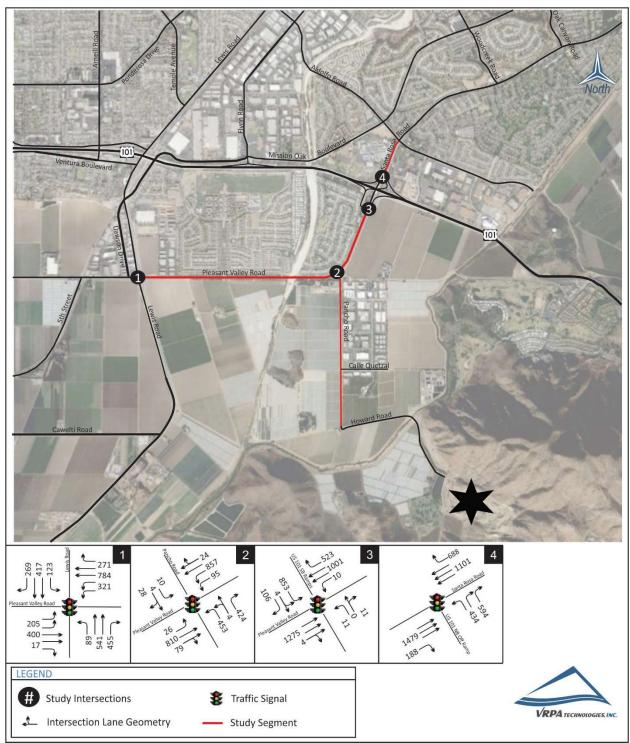




Table 3-2 Existing Plus Project Intersection Operations

	zaisting i tus i roject intersection operations										
INTERSECTION	CONTROL	PEAK HOUR	EXISTING		EXISTING PLUS PROJECT						
			ICU	LOS	ICU	LOS					
1. Lewis Road / Pleasant Valley Road	Signalized	AM	62.4	В	62.6	В					
1. LEWIS ROUGH, FREUSANC VAILEY ROUGH	Signanzeu	PM	65.4	С	65.8	С					
2. Pancho Road / Pleasant Valley Road	Cinnaliand	AM	58.5	В	61.3	В					
2. Fallello Koau / Fleasailt Valley Koau	Signalized	PM	60.8	В	63.2	В					
3. US Route 101 SB Ramps / Pleasant Valley Road	Signalized	AM	77.8	D *	77.8	D *					
3. 03 Notice 101 35 Namps / Freusant Variety Nota	Signanzeu	PM	62.0	В	62.7	В					
4. US Route 101 NB Off Ramp / Pleasant Valley Road	Signalized	AM	47.4	Α	47.8	Α					
05 Noute 101 ND On Nump / Fredsant variey Noau	Jigilalizeu	PM	54.2	Α	55.6	В					

ICU = Intersection Capacity Utilization (expressed as a percentage) / **BOLD** denotes LOS has been exceeded For signalized controlled intersections, the LOS is based on the ICU method.



 $^{^{*} \ \ \}text{Existing State highway facility is operating at less than the target LOS; the existing MOE shall be maintained.}\\$

Table 3-3 **Existing Plus Project Queuing Operations**

INTERSECTION	EXISTING (STORAGE LEI		EXISTING CONDITIONS		EXISTING PLUS PROJECT	
Lewis Road / Pleasant Valley Road Pancho Road / Pleasant Valley Road			AM	PM	AM	PM
	NB Left	600	Queue 42	Queue 74	Queue 42	Queue 74
	NB Right	600	214	378	218	379
	SB Left	150	109	101	113	103
	SB Right	275	170	224	170	224
Lewis Road / Pleasant Valley Road	EB Left	2 @ 175	172	171	172	171
	EB Right	150	15	14	15	14
	WB Left	2 @ 200	333	266	337	268
	WB Left	175	185	224	188	226
	WB Left	1/3	103		100	220
	NB Left	2 @ 50	62	368	71	378
	NB Right	200	62	321	115	353
Pancho Road / Pleasant Valley Road	SB Left	100	22	8	22	8
Pancho Road / Pleasant Valley Road US 101 SB Ramps / Pleasant Valley Road	EB Left	225	9	22	9	22
	WB Left	2 @ 350	245	53	304	79
	SB Left	125	14	8	14	8
	SB Right	1075	658	436	658	436
US 101 SB Ramps / Pleasant Valley Road	EB Left	2 @ 100	703	711	703	711
	EB Right	125	148	82	162	88
	WB Left	50	3	9	3	9
	WB Left	350	202	343	245	363
US 101 NB Off Ramp / Pleasant Valley Road	WB Right	2 @ 200	350	495	350	495
	MDVIBIII	2 W 200	330	433	330	433

Queue is measured in feet / **BOLD** denotes exceedance

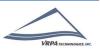


Table 3-4 **Existing Plus Project Segment Operations**

STREET SEGMENT	SEGMENT DESCRIPTION	PEAK HOUR	DIRECTION	EXISTING		EXISTING PLUS PROJECT	
				VOLUME	LOS	VOLUME	LOS
Pleasant Valley Road							1
	2 Lanes Divided	AM	EB	1,070	С	1,089	С
Lewis Road to Pancho Road		PM	EB	972	С	978	С
	2 Lanes Divided	AM	WB	966	С	977	С
	2 Euries Divided	PM	WB	1,365	D	1,376	D
	2 Lanes Divided	AM	NB	1,149	С	1,213	С
Damaha Dandta III 101 CD Damana	2 Lattes Divided	PM	NB	1,240	D	1,279	D
Pancho Road to US 101 SB Ramps	2 Lauran Dividad	AM	SB	1,043	С	1,114	С
	2 Lanes Divided	PM	SB	1,085	С	1,117	С
Santa Rosa Road							
	2. 5	AM	NB	1,819	С	1,823	С
	3 Lanes Divided	PM	NB	2,069	D	2,073	D
US 101 NB Ramps to Adolfo Road		AM	SB	2,355	D	2,361	D
	3 Lanes Divided	PM	SB	1,787	С	1,789	С
Pancho Road							
		AM	NB	150	С	225	D
	1 Lane Undivided	PM	NB	831	E	880	Е
Pleasant Valley Road to Calle Quetzal		AM	SB	450	D	537	D
	1 Lane Undivided	PM	SB	140	С	178	С
		AM	NB	19	С	94	С
	1 Lane Undivided	PM	NB	75	С	125	С
Calle Quetzal to Howard Road		AM	SB	30	С	117	С
	1 Lane Undivided	PM	SB	24	С	62	С

BOLD denotes LOS standard has been exceeded.

Project causes LOS D.

Project contributes to LOS D or worse.



3.5 Existing Plus Approved/Pending Projects and Existing Plus Approved/Pending Projects Plus Project Traffic Conditions

The Existing Plus Approved/Pending Traffic Conditions scenario considers approved or pending developments that have not yet been built in the vicinity of the Project but that are anticipated to add trips and affect traffic operation in the near-term. The Ventura County Planning Division's and City of Camarillo Community Development's approved/pending projects lists were consulted for recently approved or pending developments in the study area. The following developments were identified that are anticipated to add new trips to the study intersections and roadway segments:

- Camarillo Springs Golf Course 300 (55+ Community) dwelling units (DUs)
- St. John's Seminary Residential Development 281 single-family dwelling units (SFDUs)
- Camino Ruiz Residential Project 386 multi-family dwelling units (MFDUs)
- Teso Robles Townhomes 129 Townhomes
- Castle Building and Developments New Single-Family Development 38 SFDUs
- Mission Oaks Business Park 344,515 sq. ft. light industrial/office buildings
- Camarillo Village Homes 309 Townhomes and 12,000 sq. ft. of retail
- Park West Town Homes 87 Townhomes

An Existing Plus Approved/Pending Scenario was analyzed to include existing traffic plus traffic anticipated to be generated by the approved/pending projects in the study area. The resulting traffic operations during the AM and PM peak hour periods are shown in Figures 3-6 and 3-7.

To consider Project changes in levels of service associated with the near-term scenario, an Existing Plus Approved/Pending Projects Plus Project Scenario was analyzed to include existing traffic plus traffic generated by the approved/pending projects in the study area (as discussed above) and trips that would be generated by the Project (as discussed above in Sections 3.1 and 3.2). The resulting traffic operations during the AM and PM peak hour periods are shown in Figures 3-8 and 3-9.

3.5.1 Existing Plus Approved/Pending Projects Plus Project Intersection Capacity Analysis

Table 3-6 summarizes traffic operations under existing plus approved/pending projects conditions without the Project and under existing plus approved/pending projects conditions with the Project. As shown in the table, under existing plus approved/pending projects conditions without the Project, the US 101 SB Ramps/Pleasant Valley Road intersection is predicted to operate at LOS "E" with an ICU percentage of 86.5 during the AM peak hour. The addition of Project-related trips would not measurably change the ICU percentage or reduce the LOS at this intersection. All other intersections are predicted to operate at LOS "C" or better under existing plus approved/pending projects conditions both with and without the Project.



3.5.2 Existing Plus Approved/Pending Projects Plus Project Queuing Analysis

Table 3-8 summarizes queuing operations under existing plus approved/pending projects conditions without the Project and under existing plus approved/pending projects conditions with the Project. Queuing analysis was completed using Section 400 of Caltrans' Highway Design Manual. As discussed in Section 2.0, the queuing analysis presented in this TIS is provided for informational purposes only. The City of Camarillo, Ventura County, and Caltrans have not established CEQA impact significance criteria related to the exceedance of left and right turn storage pockets.

3.5.3 Existing Plus Approved/Pending Projects Plus Project Roadway Segment Capacity Analysis

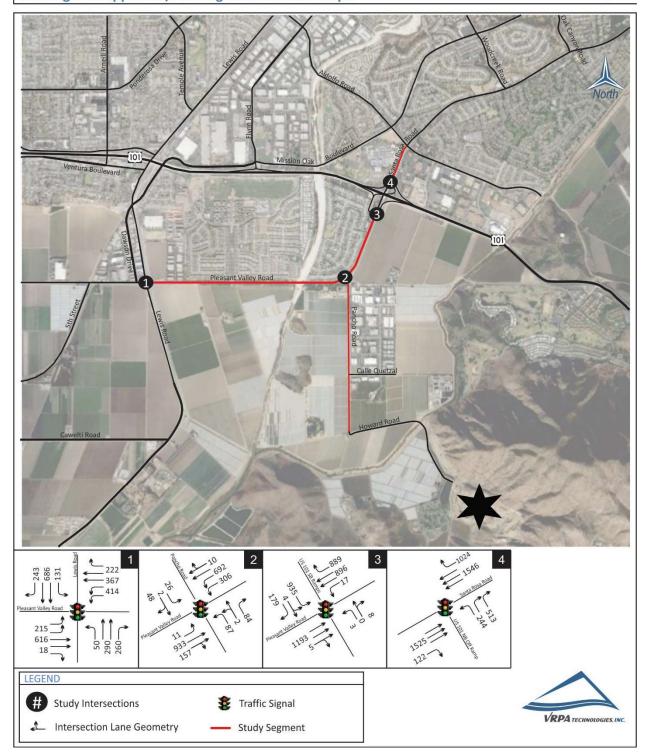
Table 3-9 summarizes traffic operations under existing plus approved/pending projects conditions without the Project and under existing plus approved/pending projects conditions with the Project and assumes that the improvements needed to address Existing Plus Project conditions have not been installed. As shown in the table, the addition of Project-related trips to Existing Plus Approved/Pending Projects conditions on study area roadway segments would result in three instances in which the Project would degrade conditions from LOS C to LOS D, six instances in which the Project would worsen LOS D conditions, one instance in which the Project would worsen LOS F conditions, as summarized below:

- Pleasant Valley Road westbound from Pancho Road to Lewis Road during the PM peak hour (worsen LOS D)
- Pleasant Valley Road northbound between Pancho Road and US 101 southbound ramps during the AM peak hour (degrade from LOS C to LOS D)
- Pleasant Valley Road northbound between Pancho Road and US 101 southbound ramps during the PM peak hour (worsen LOS D)
- Santa Rosa Road northbound between US 101 northbound ramps and Adolfo Road during the AM peak hour (worsen LOS D)
- Santa Rosa Road northbound between US 101 northbound ramps and Adolfo Road during the PM peak hour (worsen LOS D)
- Santa Rosa Road southbound between Adolfo Road and US 101 northbound ramps during the AM peak hour (worsen LOS F)
- Santa Rosa Road southbound between Adolfo Road and US 101 northbound ramps during the PM peak hour (worsen LOS D)
- Pancho Road northbound between Calle Quetzal and Pleasant Valley Road during the AM peak hour (degrade from LOS C to LOS D)
- Pancho Road northbound between Calle Quetzal and Pleasant Valley Road during the PM peak hour (degrade from LOS E to LOS F)
- Pancho Road southbound between Pleasant Valley Road and Calle Quetzal during the AM peak hour (worsen existing LOS D)
- Pancho Road southbound between Pleasant Valley Road and Calle Quetzal during the PM peak hour (degrade from LOS C to LOS D)



Pacific Rock Quarry Expansion Project Existing Plus Approved/Pending AM Peak Hour Trips

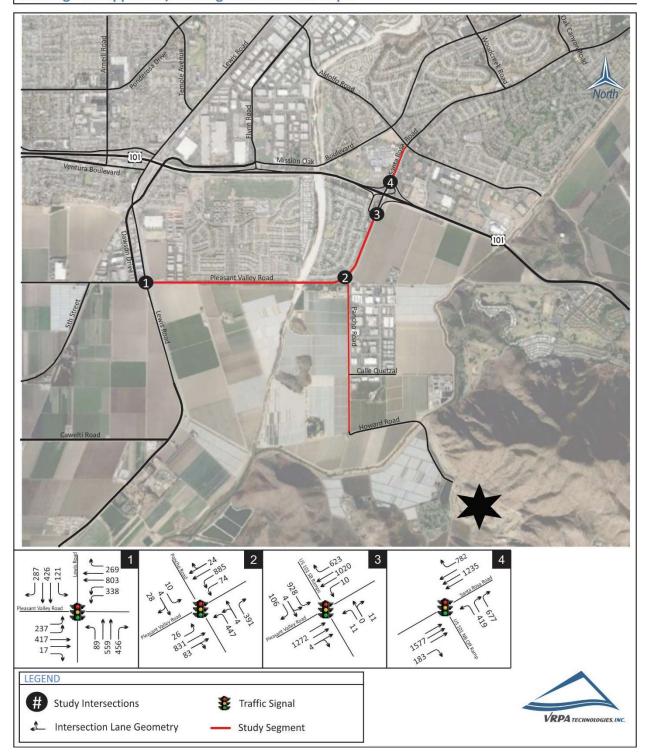
Figure 3-6





Pacific Rock Quarry Expansion Project Existing Plus Approved/Pending PM Peak Hour Trips

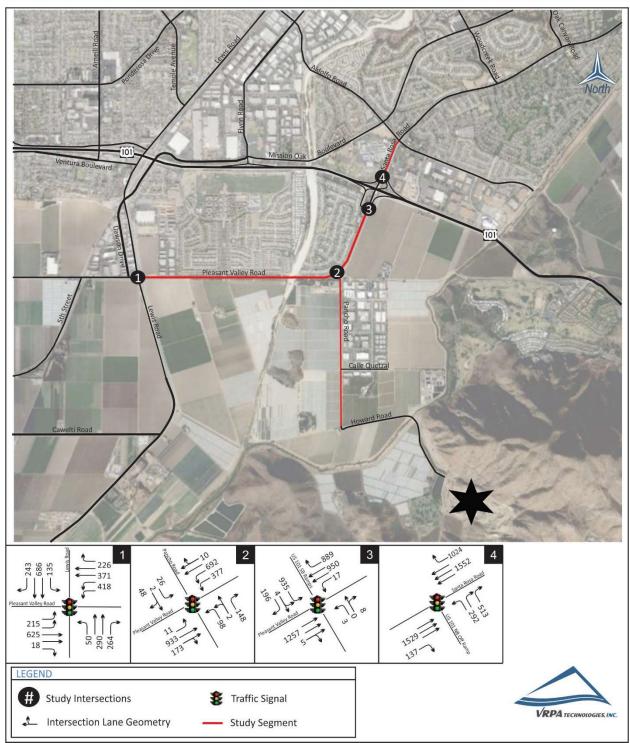
Figure 3-7





Pacific Rock Quarry Expansion Project Existing Plus Approved/Pending Plus Project AM Peak Hour Trips

Figure 3-8





Pacific Rock Quarry Expansion Project Existing Plus Approved/Pending Plus Project PM Peak Hour Trips

Figure 3-9

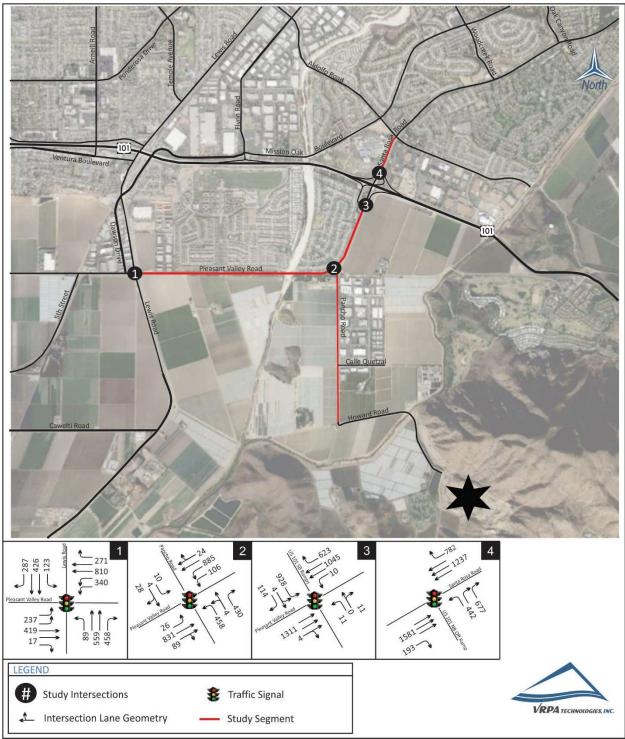




Table 3-5
Existing Plus Approved/Pending and Existing Plus Approved/Pending Plus Project
Intersection Operations

INTERSECTION	CONTROL	PEAK HOUR	EXISTING PLUS APPROVED/ PENDING		EXISTING PLUS APPROVED/ PENDING PLUS PROJECT	
			ICU	LOS	ICU	LOS
1. Lewis Road / Pleasant Valley Road	Signalized	AM	63.8	В	64.2	С
1. Lewis Road / Fredsam variey Road	Signanzeu	PM	66.3	С	66.7	С
2. Pancho Road / Pleasant Valley Road	Signalized	AM	60.0	В	62.8	В
2. Fallello Road / Freasant valley Road	Signanzeu	PM	62.0	В	64.4	С
3. US Route 101 SB Ramps / Pleasant Valley Road	Signalized	AM	86.5	E	86.5	E
3. 03 Route 101 3B Ramps / Fleasant Variety Road	Signanzeu	PM	69.8	С	69.8	С
4. US Route 101 NB Off Ramp / Pleasant Valley Road	Signalized	AM	53.2	Α	53.3	Α
4. 03 Route 101 NB On Ramp / Fleasant Variey Road	Signalized	PM	56.6	В	58.0	В

ICU = Intersection Capacity Utilization (expressed as a percentage) / **BOLD** denotes LOS has been exceeded For signalized controlled intersections, the LOS is based on the ICU method.

Project contributes to LOS D or worse.

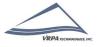


Table 3-6 **Existing Plus Approved/Pending and Existing Plus Approved/Pending Plus Project Queuing Operations**

INTERSECTION	EXISTING (STORAGE LEI		APPRO	G PLUS DVED/ DING	EXISTING PLUS APPROVED/ PENDING PLUS PROJECT	
Lewis Road / Pleasant Valley Road Pancho Road / Pleasant Valley Road			AM Queue	PM Queue	AM Queue	PM Queue
	NB Left	600	42	74	42	74
	NB Right	600	217	380	220	382
	SB Left	150	109	101	113	103
	SB Right	275	203	239	203	239
Lewis Road / Pleasant Valley Road	EB Left	2 @ 175	179	198	179	198
	EB Right	150	15	14	15	14
	WB Left	2 @ 200	345	282	348	283
	WB Left	175	185	224	188	226
	NB Left	2 @ 50	73	373	82	382
	NB Right	200	70	326	123	358
Pancho Road / Pleasant Valley Road	SB Left	100	22	8	22	8
	EB Left	225	9	22	9	22
	WB Left	2 @ 350	255	62	314	88
	SB Left	125	14	8	14	8
	SB Right	1075	741	519	741	519
US 101 SB Ramps / Pleasant Valley Road	EB Left	2 @ 100	779	773	779	773
	EB Right	125	149	88	163	95
	WB Left	50	3	9	3	9
		252	2.22	2.40		
US 101 NB Off Ramp / Pleasant Valley Road	WB Left	350	203	349	247	370
• •	WB Right	2 @ 200	428	564	428	564

Queue is measured in feet / **BOLD** denotes exceedance

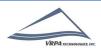
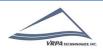


Table 3-7
Existing Plus Approved/Pending and Existing Plus Approved/Pending Plus Project
Segment Operations

STREET SEGMENT	SEGMENT DESCRIPTION	PEAK HOUR	DIRECTION	EXISTING PLUS APPROVED/ PENDING		EXISTING PLUS APPROVED/ PENDING PLUS PROJECT	
Planaryt Wallaw Board				VOLUME	LOS	VOLUME	LOS
Pleasant Valley Road					_		_
	2 Lanes Divided	AM	EB	1,101	С	1,117	С
Lewis Road to Pancho Road		PM	EB	994	С	1,000	С
	2 Lanes Divided	AM	WB	1,003	С	1,014	С
	2 201100 3111000	PM	WB	1,410	D	1,421	D
	2 Lanes Divided	AM	NB	1,198	С	1,262	D
Damaka Daad ta LIS 101 CD Damana	2 Lanes Divided	PM	NB	1,276	D	1,315	D
Pancho Road to US 101 SB Ramps	2 Lanes Divided	AM	SB	1,078	С	1,149	С
2 Ld	2 Lanes Divided	PM	SB	1,137	С	1,169	С
Santa Rosa Road			•				
		AM	NB	2,038	D	2,042	D
	3 Lanes Divided	PM	NB	2,254	D	2,258	D
US 101 NB Ramps to Adolfo Road	3 Lanes Divided	AM	SB	2,570	F	2,576	F
	3 Lanes Divided	PM	SB	2,017	D	2,019	D
Pancho Road							
		AM	NB	173	С	248	D
	1 Lane Undivided	PM	NB	842	E	891	F
Pleasant Valley Road to Calle Quetzal		AM	SB	465	D	552	D
	1 Lane Undivided	PM	SB	161	С	199	D
		AM	NB	19	С	94	С
	1 Lane Undivided	PM	NB	75	С	125	С
Calle Quetzal to Howard Road		AM	SB	30	С	117	С
	1 Lane Undivided	PM	SB	24	С	62	С

BOLD denotes LOS standard has been exceeded.

Project causes or contributes to LOS D or worse.



3.6 Cumulative Year 2030 Traffic Conditions

Traffic volumes expected in 2030 consider existing traffic and increases in traffic over time resulting from development projected in the General Plans of local agencies, including the County of Ventura and City of Camarillo. Changes in traffic operations resulting from the Project were analyzed considering the long-range buildout under the City of Camarillo General Plan which considers future development in the City of Camarillo and surrounding region (Ventura County) through the year 2030. Use of the City of Camarillo General Plan is appropriate for this TIS since all study are intersections are within the City of Camarillo's sphere of influence. The buildout traffic volumes for the study area intersections and roadway segments were derived from the City of Camarillo's Traffic Analysis Model (CTAM) as noted in the TIS prepared for the Camino Ruiz Residential Project (Stantec December 7, 2019). The CTAM was prepared and is maintained by VCTC and is a sub-area derivation of the Ventura Countywide Traffic Model (VCTM). Updated in the year 2010, the CTAM is based upon the latest VCTM projections and the latest land use projections and roadway improvement plans for the City of Camarillo and the surrounding region. Forecast adjustments were applied accordingly and were based on engineering judgment. In a few cases, the traffic volumes derived from CTAM were slightly lower than the Existing Plus Approved/Pending trips discussed in Section 3.5. Adjustments were made to eliminate any decreases in traffic volumes between the Existing scenario and the Cumulative Year 2030 Without Project scenario. Traffic operations during the AM and PM peak hour periods under the Year 2030 scenario without the Project are shown in Figures 3-10 and 3-11.

To consider changes in traffic operations resulting from the Project associated with the Year 2030 scenario, trips that would be generated by the Project (as discussed above in Sections 3.1 and 3.2) were added to the Cumulative Year 2030 without Project scenario. Traffic operations during the AM and PM peak hour periods under the Cumulative Year 2030 Plus Project scenario are shown in Figures 3-12 and 3-13.

3.6.1 Cumulative Year 2030 Intersection Capacity Analysis

Table 3-11 summarizes traffic operations under Cumulative Year 2030 conditions without the Project and under Cumulative Year 2030 conditions with the Project. As shown in the table, the addition of Project-related trips to Cumulative Year 2030 Without Project conditions on study area intersections would contribute to trips and increase delay at two intersections predicted to be below LOS "C" under Year 2030 conditions without the Project: Lewis Road at Pleasant Valley Road and US Route 101 SB Ramps at Pleasant Valley Road contributing to LOS "D" at the Lewis Road/Pleasant Valley Road intersection and contributing to LOS "F" (PM) and LOS "G" (AM) conditions at the US Route 101 SB Ramps/Pleasant Valley Road intersection.

3.6.2 Cumulative Year 2030 Queuing Analysis

Table 3-13 summarizes queuing operations under Cumulative Year 2030 conditions without the Project and under Cumulative Year 2030 conditions with the Project. Queuing analysis was completed using Section 400 of Caltrans' Highway Design Manual. As discussed in Section 2.0,



the queuing analysis presented in this TIS is provided for informational purposes only. The City of Camarillo, Ventura County, and Caltrans have not established CEQA impact significance criteria related to the exceedance of left and right turn storage pockets.

Table 3-14 identifies left turn and right turn lane pocket lengths required for the Cumulative Year 2030 scenario. Although the need for extended turn lane pockets would occur at some locations prior to the Cumulative Year 2030 scenario, this scenario provides the maximum length needed and therefore these lengths would also provide for projected traffic volumes under the Existing Plus Project and Existing Plus Approved/Pending Project Plus Project scenarios. The storage length required to provide sufficient capacity for projected traffic volumes under each evaluation scenario was determined by the queuing analysis and recommendations of storage lengths found in Chapter 400 of Caltrans' Highway Design Manual. The left turn and right turn pocket length do not include deceleration lengths.

A queuing assessment of the US 101 NB Off Ramp and US 101 SB Off Ramp to Pleasant Valley Road was also conducted to determine the adequacy of the existing ramp lengths. The Cumulative Year 2030 Plus Project traffic volume at the US 101 NB Off Ramp to Pleasant Valley Road will yield a combined storage requirement of 1,025 feet. The existing total ramp length of the US 101 SB Off Ramp is approximately 1,300 feet. The Cumulative Year 2030 Plus Project traffic volume at the US 101 SB Off Ramp to Pleasant Valley Road will yield a combined storage requirement of 1,125 feet. The existing total ramp length of the US 101 NB Off Ramp is approximately 1,225 feet. The existing ramp lengths are sufficient to accommodate Cumulative Year 2030 Plus Project traffic. It should be noted that Caltrans recommended auxiliary lane improvements between Village Park Drive and Pleasant Valley Road in the southbound direction of US 101 in the VCTC US 101 HOT Lanes Financial Feasibility Study. The auxiliary lane would provide drivers with additional space to accelerate or decelerate when entering or exiting the freeway which enhances the traffic flow along the freeway.

3.6.3 Cumulative Year 2030 Roadway Segment Capacity Analysis

Table 3-15 summarizes traffic operations under Cumulative Year 2030 conditions without the Project and under Cumulative Year 2030 conditions with the Project, and assumes existing road and intersection configurations. As shown in the table, the addition of Project-related trips to Cumulative Year 2030 Without Project conditions on study area roadway segments would result in eleven instances in which the Project would worsen LOS D conditions, one instance in which the Project would worsen LOS E conditions, and four instances in which the Project would worsen LOS F conditions, as summarized below:

- Pleasant Valley Road eastbound from Lewis Road to Pancho Road during the AM peak hour (worsen LOS D)
- Pleasant Valley Road eastbound from Lewis Road to Pancho Road during the PM peak hour (worsen LOS D)
- Pleasant Valley Road westbound from Pancho Road to Lewis Road during the AM peak hour (worsen LOS D)

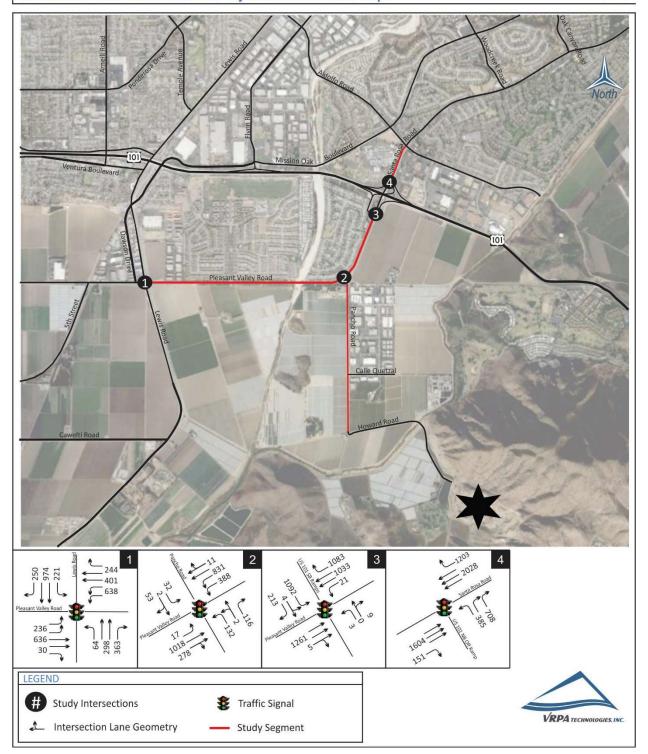


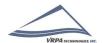
- Pleasant Valley Road westbound from Pancho Road to Lewis Road during the PM peak hour (worsen LOS D)
- Pleasant Valley Road northbound between Pancho Road and US 101 southbound ramps during the AM peak hour (worsen LOS D)
- Pleasant Valley Road northbound between Pancho Road and US 101 southbound ramps during the PM peak hour (worsen LOS D)
- Pleasant Valley Road southbound between US 101 southbound ramps and Pancho Road during the AM peak hour (worsen LOS D)
- Pleasant Valley Road southbound between US 101 southbound ramps and Pancho Road during the PM peak hour (worsen LOS D)
- Santa Rosa Road northbound between US 101 northbound ramps and Adolfo Road during the AM peak hour (worsen LOS D)
- Santa Rosa Road northbound between US 101 northbound ramps and Adolfo Road during the PM peak hour (worsen LOS F)
- Santa Rosa Road southbound between Adolfo Road and US 101 northbound ramps during the AM peak hour (worsen LOS F)
- Santa Rosa Road southbound between Adolfo Road and US 101 northbound ramps during the PM peak hour (worsen LOS F)
- Pancho Road northbound between Calle Quetzal and Pleasant Valley Road during the AM peak hour (worsen LOS D)
- Pancho Road northbound between Calle Quetzal and Pleasant Valley Road during the PM peak hour (worsen LOS F)
- Pancho Road southbound between Pleasant Valley Road and Calle Quetzal during the AM peak hour (worsen LOS E)
- Pancho Road southbound between Pleasant Valley Road and Calle Quetzal during the PM peak hour (worsen LOS D)



Pacific Rock Quarry Expansion Project Cumulative Year 2030 Without Project AM Peak Hour Trips

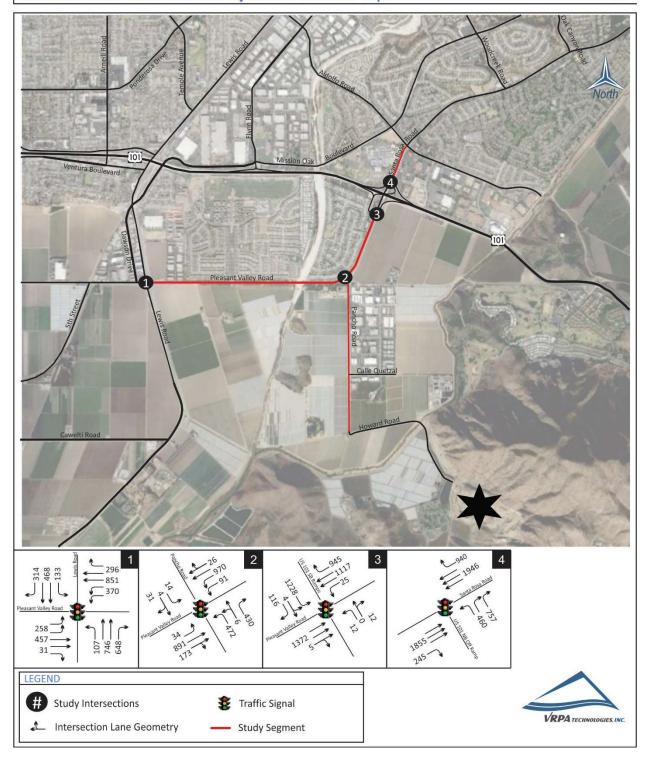
Figure 3-10





Pacific Rock Quarry Expansion Project Cumulative Year 2030 Without Project PM Peak Hour Trips

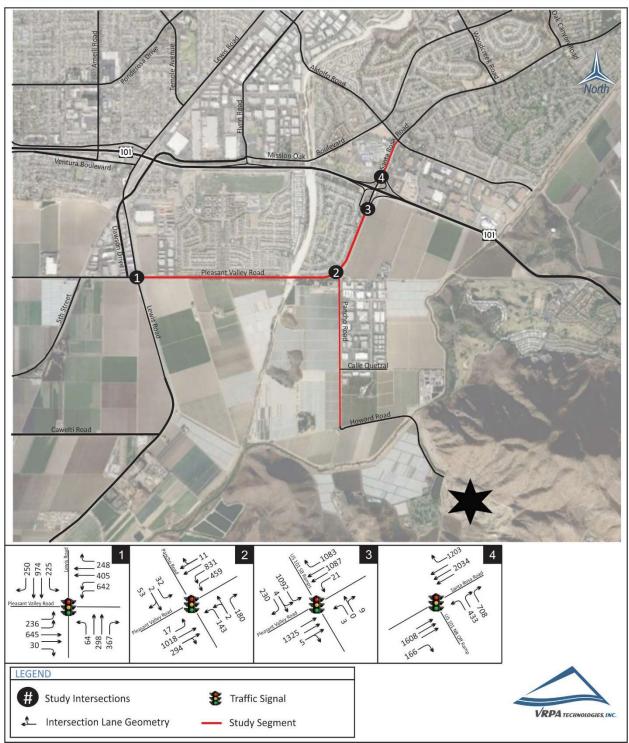
Figure 3-11





Pacific Rock Quarry Expansion Project Cumulative Year 2030 Plus Project AM Peak Hour Trips

Figure 3-12





Pacific Rock Quarry Expansion Project Cumulative Year 2030 Plus Project PM Peak Hour Trips

Figure 3-13

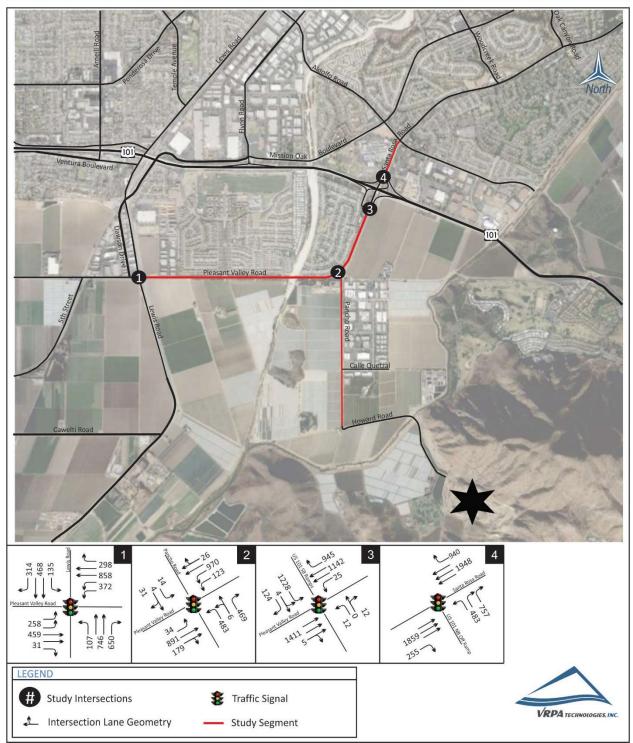




Table 3-8 Cumulative Year 2030 Intersection Operations

INTERSECTION	CONTROL	PEAK HOUR	CUMULATIVE YEAR 2030 WITHOUT PROJECT		CUMULATIVE YEAR 2030 PLUS PROJECT	
	Signalized -		ICU	LOS	ICU	LOS
1. Lewis Road / Pleasant Valley Road	Cianalizad	AM	77.9	D	78.3	D
2. Lewis Road / Fredsum variey Road	Signanzeu	PM	80.2	D	80.6	D
2. Pancho Road / Pleasant Valley Road	Signalized	AM	69.5	С	72.3	С
2. Fallello Road / Fleasant Valley Road	Signanzeu	PM	66.8	С	69.2	С
3. US Route 101 SB Ramps / Pleasant Valley Road	Signalized	AM	102.8	G	102.8	G
3. 03 Noute 101 3B Namps / Freusant Variety Road	Signanzeu	PM	98.1	F	98.1	F
4. US Route 101 NB Off Ramp / Pleasant Valley Road	Signalized	AM	69.4	С	69.4	С
4. 05 Noute 101 No on Nump / Fredsame variety Road	Signalized	PM	69.5	С	69.5	С

ICU = Intersection Capacity Utilization (expressed as a percentage) / **BOLD** denotes LOS has been exceeded For signalized controlled intersections, the LOS is based on the ICU method.

Project contributes to LOS D or worse.



Table 3-9 **Cumulative Year 2030 Queuing Operations**

Pancho Road / Pleasant Valley Road	EXISTING (STORAGE LEI		2030 W	TIVE YEAR /ITHOUT JECT	CUMULATIVE YEAR 2030 PLUS PROJECT	
			AM	PM	AM	PM
	212.1.6	500	Queue	Queue	Queue	Queue
	NB Left	600	53	89	53	89
	NB Right	600	303	540	306	542
	SB Left	150	176	111	188	113
Lewis Road / Pleasant Valley Road	SB Right	275	208	262	208	262
,,,	EB Left	2 @ 175	197	215	197	215
	EB Right	150	25	26	25	26
	WB Left	2 @ 200	532	308	535	310
	WB Left	175	203	247	207	248
	NB Left	2 @ 50	110	393	119	403
	NB Right	200	97	358	150	391
Pancho Road / Pleasant Valley Road	SB Left	100	27	12	27	12
	EB Left	225	14	28	14	28
	WB Left	2 @ 350	323	76	383	103
				-		
	SB Left	125	18	21	18	21
	SB Right	1075	903	788	903	788
US 101 SB Ramps / Pleasant Valley Road	EB Left	2 @ 100	910	1023	910	1023
	EB Right	125	178	97	192	103
	WB Left	50	3	10	3	10
	_					
US 101 NB Off Ramp / Pleasant Valley Road	WB Left	350	321	383	364	404
	WB Right	2 @ 200	590	631	590	631

Queue is measured in feet / **BOLD** denotes exceedance



Table 3-10 Left Turn and Right Turn Storage Requirements

INTERSECTION	EXISTING (STORAGE LEI		CUMULATIVE YEAR 2030 PLUS PROJECT RECOMMENDED STORAGE LENGTH (ft)		
Lewis Road / Pleasant Valley Road	NB Left	600	NB Left	600	
	NB Right	600	NB Right	600	
	SB Left	150	SB Left	200	
	SB Right	275	SB Right	275	
	EB Left	2 @ 175	EB Left	2 @ 175	
	EB Right	150	EB Right	150	
	WB Left	2 @ 200	WB Left	2 @ 275	
	WB Left	175	WB Left	250	
Pancho Road / Pleasant Valley Road	NB Left	2 @ 50	NB Left	2 @ 50	
	NB Right	200	NB Right	200	
	SB Left	100	SB Left	100	
	EB Left 225 E		EB Left	EB Left 225	
	WB Left	2 @ 350	WB Left	2 @ 350	
US 101 SB Ramps / Pleasant Valley Road	SB Left	125	SB Left	125	
	SB Right	1075	SB Right	1075	
	EB Left	2 @ 100	EB Left	2 @ 700	
	EB Right	125	EB Right	200	
	WB Left	50	WB Left	50	
US 101 NB Off Ramp / Pleasant Valley Road	WB Left	350	WB Left	400	
	WB Right	2 @ 200	WB Right	2 @ 325	

BOLD denotes change in storage length

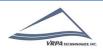


Table 3-11 Cumulative Year 2030 Segment Operations

STREET SEGMENT	SEGMENT DESCRIPTION	PEAK HOUR	DIRECTION	CUMULATIVE YEAR 2030 WITHOUT PROJECT		CUMULATIVE YEAR 2030 PLUS PROJECT	
Diograph Valley Road				VOLUME	LOS	VOLUME	LOS
Pleasant Valley Road Lewis Road to Pancho Road	2 Lanes Divided	AM	EB	1,313	D	1,329	D
		PM	EB	1,238	D	1,244	D
	2 Lanes Divided	AM	WB	1,283	D	1,294	D
		PM	WB	1,517	D	1,528	D
Pancho Road to US 101 SB Ramps	2 Lanes Divided	AM	NB	1,266	D	1,330	D
				-			
		PM	NB	1,377	D	1,416	D
	2 Lanes Divided	AM	SB	1,249	D	1,320	D
		PM	SB	1,245	D	1,277	D
Santa Rosa Road	1 1						
US 101 NB Ramps to Adolfo Road	3 Lanes Divided	AM	NB	2,312	D	2,316	D
		PM	NB	2,612	F	2,616	F
	3 Lanes Divided	AM	SB	3,231	F	3,237	F
		PM	SB	2,886	F	2,888	F
Pancho Road							
Pleasant Valley Road to Calle Quetzal	1 Lane Undivided	AM	NB	250	D	325	D
		PM	NB	908	F	957	F
	1 Lane Undivided	AM	SB	668	E	755	E
		PM	SB	268	D	306	D
Calle Quetzal to Howard Road	1 Lane Undivided	AM	NB	69	С	144	С
		PM	NB	114	С	164	С
	1 Lane Undivided	AM	SB	90	С	177	С
		PM	SB	88	С	126	С

BOLD denotes LOS standard has been exceeded.

Project contributes to LOS D or worse.



4.0 Impact Determinations

Appendix G of the CEQA Guidelines and the County's April 26, 2011, Initial Study Assessment Guidelines (ISAG) identify certain transportation-related topics for consideration during CEQA review. These issues include potential policy or land use plan conflicts, potential impacts associated with safety on public roads and private access driveways, potential impacts on bicycle and pedestrian circulation and safety, and potential impacts on transit operations.³ Each of these is discussed the following sections. (As discussed in the introduction, CEQA and the CEQA Guidelines as amended in 2018 also required that by July 1, 2020, CEQA lead agencies must evaluate transportation impacts in consideration of vehicle miles traveled or similar metric. This TIS does not include an evaluation of VMT associated with the Project, and it is anticipated that the County will separately address Project-related VMT in the EIR in consideration of SB 743 and CEQA Guidelines.)

Potential Conflict with a Program, Plan, Ordinance or Policy Addressing the Circulation System

Ventura County, the City of Camarillo, and Caltrans seek to maintain acceptable levels of service along the highway, street, and road network. These agencies adopt minimum levels of service in an attempt to control congestion that may result as new development occurs.4 The traffic operations evaluation in this TIS discusses the various level of service goals and policies of these agencies and evaluates predicted levels of service associated with various with-Project evaluation scenarios. As assessment of the Project's consistency with programs, plans, ordinances, and policies is beyond the scope of this TIS and it is anticipated that Project consistency will be addressed by the County in the EIR to the prepared for the Project.

Potential Impacts on Transit Services

Transit services within the City of Camarillo are served by Fixed Route, Dial-A-Ride and Ventura County Transportation Commission (VCTC) Intercity service. The Fixed Route service, provided by Camarillo Area Transit (CAT), does not include transit routes in the study area. The VCTC Intercity is a Countywide service, which connects Camarillo with Thousand Oaks, Oxnard and Ventura. The Oxnard/Camarillo/CSUCI route traverses Pleasant Valley Road along Lewis Road, with a stop located along Lewis Road just south of US-101. The additional Project trips would not interfere with these transit routes or stops and, thus, would not result in significant adverse effects on existing or planned transit facilities in the Project study area.

Potential Impacts on Bicycle and Pedestrian Safety and Circulation

³ The ISAG also identifies Transportation Level of Service as an issue to consider, and levels of service are evaluated in detail in this TIS. The ISAG also identifies transportation items associated with railroads, airports, harbor facilities, and pipelines; however, addressing those items is outside the scope of this TIS. 4 At the time of preparation of this TIS, agencies including Ventura County and Caltrans, are considering amendments to policies pertaining to congestion in efforts to implement and comply with the requirements of amendments to CEQA and the CEQA Guidelines pursuant to SB 743.



Bicycling is considered an effective alternative mode of transportation that can help to improve air quality and reduce the number of vehicles traveling along existing highways, especially within the cities and unincorporated communities. The City of Camarillo Bikeway Master Plan identifies existing Class II bike lanes along the study segments of Pleasant Valley Road and Santa Rosa Road and a planned Class II bike lane along Pancho Road, which would be designed in accordance with City of Camarillo standards. Sidewalks presently exist along the north/west side of Pleasant Valley Road study segment, both sides of the Santa Rosa Road study segment, and along the east side of Pancho Road.

The existing Class II bike lanes and pedestrian facilities crossing Lewis Road, Pancho Road, and US 101 NB and SB ramps, do so at traffic-controlled intersections. All of the study intersections evaluated in this TIS are signalized and include pedestrian signal phasing which accommodates pedestrians utilizing the crosswalk. Though traffic within the study area is expected to increase over time, these traffic control devices will help maintain pedestrian and bicycle safety within the study area. Class II bike lanes are identified in the City of Camarillo's General Plan Circulation Element on all study roadway segments, and it is anticipated that the City will retain and add Class II bike lanes on these segments sufficient to accommodate bicycle and pedestrian safety and circulation. The additional Project trips would not adversely affect existing or planned bicycle or pedestrian facilities in the Project study area.

Potential Impacts Associated with Hazards on Public Roads or Private Access Roads due to Design or Incompatible Uses

The proposed Project will not create any new design features on or off the Project site. The existing on-site circulation pattern will remain the same as the currently approved surface mining permit. Although there will be an increase in the volume of vehicles accessing the site during peak-hour periods and some of the incoming haul trucks will be loaded for delivery of recycle materials or fill material, the same types of vehicles (heavy-duty haul trucks and personal vehicles) will continue to access the site. The existing site access/egress is located at a sufficient distance from any intersection to allow for safe vehicular access/egress to and from the site. Therefore, this impact is considered less than significant, and no mitigation is required.

Potential Impacts Related to Emergency Access

The Project site is currently accessed/egressed via an existing entrance road from Howard Road, a private road that provides access to the Project site and to the Conejo Mountain Memorial Cemetery. Emergency access to the site would be unaffected by the Project. Therefore, this impact is considered less than significant, and no mitigation is required.



APPENDIX A

HCM-Based LOS Tables (Florida Tables)

Generalized **Peak Hour Directional** Volumes for Florida's **Urbanized Areas**¹

03/14/2018

			w-80005-0000								03/14/2018
	INTERRU	PTED FL	OW FAC	ILITIES			UNINTE	RRUPTED	FLOW FA	CILITIES	
ST	TATE SIG	NALIZ	ED AR	FERIAL	S			FREE	WAYS		
1 U	Princip ledian ndivided ivided ivided	bal (1 signa B * 50 80	nl per half C 200 1,350 2,040	mile) D 690 1,790 2,690	E 930 1,870 2,820	Lanes 2 3 4 5 6	B 2,510 3,660 4,820 6,580 8,150	C 3,410 5,030 6,670 9,240 10,99	6, 8, 10	D 230 240 310 ,840	E 4,330 6,500 8,670 **
1 Ui 2 Di	Minor edian ndivided ivided ivided	(1 signal po	er quarter C * 470 880	mile) D 210 1,390 2,190	E 710 1,840 2,780	U	,	ŕ	Adjustmen	•	
		orresponding the indicated	g state volu d percent.)		ents						
1 Di 1 Ur Multi Ur	edian [] ivided idivided idivided idivided — One-Wa Multiply tl	Exclusive Left Lanes Yes No Yes No - Ty Facility he correspones in this	Excl Right N N N Y Y Y Adjust nding dire	usive Lanes To To To To To To To To To To To To To	Adjustment Factors +5% -20% -5% -25% + 5%	Lanes 1 2 3 Lanes 1 Multi	UNINTERF Median Undivided Divided Divided Uninterrup Median Divided Undivided Undivided Undivided	B 610 1,840 2,770 ted Flow Exclusiv	C 930 2,660 3,990	D 1,260 3,350 5,020 Adjustmer Adjustm	E 1,690 3,760 5,640
Paved Shou Lane C 0-4 50-		cycle in hicle volumes to determine volumes	MODE ² es shown b ine two-ways.)	elow by nun		¹ Values and are f constitute computer planning corridor based on Capacity	shown are presente for the automobile/te e a standard and sh models from which applications. The to intersection desi- planning application and Quality of Ser f service for the bio of motorized vehicle	d as peak hour ruck modes un ould be used o ch this table is able and deriv- gn, where mor ons of the High vice Manual.	directional voluless specifically his for general derived should ding computer me refined techniuway Capacity lustrian modes in	umes for levels y stated. This to planning application be used for mo todols should n iques exist. Cal Manual and the this table is ba	of service able does not autions. The re specific ot be used for culations are Transit
directional Sidewalk 0-4 50-	PEDE motorized vel roadway lane Coverage 19% 84%		es shown be ne two-way	elow by nun		** Not ap volumes been reac not achie	t hour shown are on the achieved using opticable for that le greater than level of hed. For the bicycl wable because there are defaults.	table input va vel of service l f service D be e mode, the le	lue defaults. letter grade. For	r the automobil intersection catter grade (incl	e mode, apacities have uding F) is
BUS Sidewalk 0-8	S MODE		led Fixe	d Route	_	Systems 1	epartment of Trans Planning Office state fl.us/planning		os/default.shtm		

TABLE 7 (continued)

Generalized **Peak Hour Directional** Volumes for Florida's **Urbanized Areas**

03/14/2018

INPUT VALUE	Uninterru	pted Flov	v Facilities		In	terrupted	Flow Facil	lities	
ASSUMPTIONS	Freeways	Hig	hways	Principa	al Arterials	Minor	r Arterials	Bicycle	Pedestr
ROADWAY CHARACTERISTICS								•	
Area type (urban, rural)	urban								
Number of through lanes (both dir.)	4-12	2	4-6	2-4	6	2-4	6	4	4
Posted speed (mph)	70	50	50	50	50	40	40	45	45
Free flow speed (mph)	75	55	55	55	55	45	45	50	50
Auxiliary Lanes (n, y)	n								
Median (d, u, twlt)			d						
Terrain (l,r)	1	1	1	1	1	1	1	1	1
% no passing zone		80							
Exclusive left turn lane impact (n, y)		[n]	у	у	у	у	у	у	у
Exclusive right turn lanes (n, y)				n	у	n	y	1	1
Facility length (mi)	3	5	5	2	2	2	2	2	2
Interchange Density (intch/mi)	1						1 -		
TRAFFIC CHARACTERISTICS									
Planning analysis hour factor (K)	0.090	0.090	0.000	0.000	0.000	0.000	0.000	0.000	1 0 000
Directional distribution factor (D)	0.090		0.090	0.090	0.090	0.090	0.090	0.090	0.090
Peak hour factor (PHF)		0.55	0.55	0.55	0.55	0.55	0.55	0.565	0.565
Base saturation flow rate (pcphpl)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
	2,400	1,700	2,100	1,950	1,950	1,950	1,950	1,950	1,950
Heavy vehicle percent	4.0	2.0	2.0	2.0	2.0	2.0	2.0	2.5	2.0
Speed Adjustment Factor (SAF)	0.950		0.950						
Capacity Adjustment Factor (CAF) % left turns	0.939		0.939						
				12	12	12	12	12	12
% right turns				12	12	12	12	12	12
CONTROL CHARACTERISTICS									
Number of signals				5	5	9	9	4	6
Arrival type (1-6)				3	3	3	3	4	4
Signal type (a, c, p)				c	С	С	С	С	С
Cycle length (C)				150	150	120	120	120	120
Effective green ratio (g/C)				0.44	0.44	0.44	0.44	0.44	0.44
MULTIMODAL CHARACTERISTI	CS								1
Paved shoulder/bicycle lane (n, y)	CS							n 500/	
Outside lane width								n, 50%, y	n
Pavement condition								t	t
On-street parking								t	
Sidewalk (n, y)								n	n 500/
Sidewalk/roadway separation (a, t, w)									n, 50%,
Sidewalk protective barrier (n, y)									t
Bracowark protective burner (ii, y)									n
		EL OF SE	RVICE T	HRESHO	LDS				
	Freeways		Highways		Arte	rials	Bicycle	Ped	Bus
	Density	Two-Lan	e M	ultilane	Principal	& Minor			
Level of Service	pc/mi/ln	%ffs		ensity	%b:	ffs	Score	Score	Buses/h
	- 10		p	c/mi/ln					
В	≤ 18	> 83.3		≤18	> 6		≤2.75	≤ 2.75	≤6
C	≤ 26	> 75.0		≤26	> 5		≤ 3.50	≤ 3.50	<u>≤4</u>
D	≤ 35	> 66.7		≤ 35	> 4	0	≤ 4.25	≤4.25	< 3
Е	≤ 45	≤66.7		≤ 45	> 3	.0	≤5.00	≤ 5.00	< 2

APPENDIX B

Traffic Count Data Worksheets

Location: S Lewis Rd & Pleasant Valley RIntersection Turning Movement Count City: Camarillo Control: Siganlized Control: Sigan

		Т	T			Т																				_	_	_		_		_					
	PEAK HR FACTOR:	PEAK HR VOL:	27.100001.000	ADDROACH 06'S			5:45 PM	5:30 PM	5:15 PM	5:00 PM	4:45 PM	4:30 PM	4:15 PM	4:00 PM		<u>₽</u>			PEAK HR FACTOR:	PEAK HR VOL:	PEAK HR:	APPROACH %'s:	TOTAL VOLUMES:		0:43 AM	8:30 AM	8:15 AM	8:00 AM	7:45 AM	7:30 AM	7:15 AM	7:00 AM		AΜ		NS/EW Streets:	
	0.695	89	0.02/0	168	2		11	24	19	24	14	32	220	18	2	,			0.735	50		8.83%	87	2	7) (C	10	6	17	17	10	9	몬	just.			
0.900	0.867	541	J1.02 /0	1082	Ŋ		141	122	125	135	125	100	109 109	109	4	2	NORTH	0.881	0.858	285	07:15 AM -	48.63%	479	N	90	41	42	89	76	83	58	45	목	2	NORTH	S Lewis Rd	
3	0.928	453		846	NR.		118	91	122	118	TOO	11.5	500	116	NR.	<u></u>	NORTHBOUND	81	0.945	257	08:15 AM	42.54%	419	R	45	38	41	58	68	68	63	40	R	<u>,</u>	NORTHBOUND	/is Rd	
	0.000	9	0.0076	0	2		0	0	0	0	· c		0 0	0	2	0			0.000	0		0.00%	0	2	c	0	0	0	0	0	0	0	2	0			
	0.840	121	07.07.FT	227	SI		31	21	32	31	36	2 2	2 %	26	SE	<u>-</u>			0.910	131		12.54%	249	ZZ	3/	22	32	36	34	ၾ	26	27	<u>S</u>	jud.			
0 956	0.974	417	JT. 7170	873	ST		106	134	104	107	105	101	2 2	123	ST	2	HTUOS	0.937	0.910	666		69.32%	1376	SŢ	787	224	168	183	171	165	147	136	ST	2	HTUOS	S Lewis Rd	
2	0.873	269	30.0270	490	SR		41	71	53	69	6	1	4/	62	SR	jk	SOUTHBOUND	37	0.850	204		18.14%	360	SR	4	39	37	48	54	60	42	40	SR	J	SOUTHBOUND	is Rd	
	0.000	>	0.00%	0	SU		0	0	0	0	c	0 0	0	0	SU	0			0.000	0		0.00%	0	દ	C	0	0	0	0	0	0	0	S	0			וטנמו
	0.869	205	34.11.70	412	円		45	65	51	50	59	1.5	50	47	四	2			0.858	206		25.45%	336	ᆈ	34	21	30	52	57	60	37	45	四	2			9
0 923	0.881	308	05.00%	762	띄		98	100	106	113	97	82	8 83	8 8	9	2	EASTBOUND	0.871	0.860	588		71.97%	950		6	82	110	116	171	137	164	100	四	2	EASTE	Pleasant Valley Rd	
2	0.850	17	2,01%	34	焸		4	pak A	4	U	4	4.	7	l GI	罗	<u></u>	GINDO	71	0.643	18		2.58%	34	뮈	7	4	<u> </u>	2	UI	7	4	4	罗	;	EASTBOUND	alley Rd	
	0.000	>	0.00%	0	ᄪ		0	0	0	0	0	0	0	0	Ξ	0			0.000	0		0.00%	0		0	0	0	0	0	0	0	0	2	0			
	0.858	310	23.9/%	598	WL		37	76	68	81	93	77	103	63	M۲	2			0.862	400		45.73%	797	¥	89	116	98	116	89	109	86	94	×	2			
0 00	0.863	777	20.40%	1459	M		170	180	171	198	183	225	172	160	¥	2	WESTBOUND	0.866	0.711	344		34.19%	596	5	63	57	70	73	121	75	75	62	¥	2	WESTBOUND	Pleasant Valley Ro	
ň	63 0.897	260	1/.50%	438	WR		45	41	62	71	61	75	33	50	WR	<u></u>	GNDO	36	0.804	222		20.08%	350	₽	30	24	46	54	69	56	43	28	₩R	_		alley Rd	
	0.000	>	0.00%	0	٤		0	0	0	0	0	0	0	0	Æ	0			0.000	0		0.00%	0	٤	0	0	0	0	0	0	0	0	<u></u>	0			
0.960	20/2	TOTAL		7389	TOTAL		847	926	917	1002	947	1009	879	862	TOTAL			0.307	200	3371	TOTAL		6033	TOTAL	670	677	685	812	932	872	755	630	TOTAL				

0.960

Location: Pleasant Valley Rd & Pancho RIntersection Turning Movement Count City: Camarillo Control: Siganlized Control: Siganlized Control: Siganlized

Total

	PEAK HR FACTOR:	PEAK HR VOL:	PEAK HR:	APPROACH %'s:	TOTAL VOLUMES:		8:45 AM	8:30 AM	8:15 AM	8:00 AM	7:45 AM	7:30 AM	7:15 AM	7:00 AM		AIVI		NS/EW Streets:	
	0.625	10		1.24%	22	Z	7	ш	. р.	4	ω	<u></u>	2	ω	ZF	-			
0.9	0.920	905	07:15 AM	83.07%	1472	Ŋ	122	129	158	198	223	238	246	158	4	2	NORTH	Pleasant Valley Rd	
0.949	0.688	154	07:15 AM - 08:15 AM	15.63%	277	NR	34	24	27	39	56	34	25	<u>კ</u>	NR	<u>_</u>	NORTHBOUND	Valley Rd	
	0.250	1		0.06%	1	S	0	0	0	 	0	0	0	0	NC	0			
	0.835	294		29.71%	541	SL	58	53	72	85	88	50	71	64	SL	2			
0.767	0.739	668		69.36%	1263	ST	129	174	161	129	226	166	147	131	ST	2	HTUOS	Pleasant Valley Rd	
67	0.625	10		0.93%	17	SR	 4	Jacob	ω	4	ω	2	p.4	2	SR	0	SOUTHBOUND	/alley Rd	
	0.000	0		0.00%	0	SU	0	0	0	0	0	0	0	0	SU	0			
	0.813	26		37.93%	4	띧	7	6	<u> </u>	6	œ	Сī	7	4	P	<u>_</u>			
0.792	0.250	2		3.45%	4	曰	 	0	just	0	2	0	0	0	Д	0.5	EASTBOUND	Pancho Rd	
92	0.706	48		58.62%	88	ER	7	ω	6	7	14	17	10	4	贸	0.5	DUND	o Rd	
	0.000	0		0.00%	0	핃	0	0	0	0	0	0	0	0	EU	0			
	0.804	74		46.67%	133	WL	17	18	12	21	23	16	14	12	WL	1.5			
0.938	0.500	2		0.70%	2	TW	0	0	0	0	<u> </u>	Ľ	0	0	TW	0.5	WESTBOUND	Pancho Rd	
88	0.740	74		52.63%	150	WR	25	13	20	11	16	22	25	18	WR	 	QNNO	Rd	
	0.000	0		0.00%	0	W)	0	0	0	0	0	0	0	0	WU	0			
0.855	0 0 0	2268	TOTAL		3994	TOTAL	408	422	462	505	663	552	548	434	TOTAL				

	PEAK HR FACTOR:	PEAK HR VOL:	PEAK HR:	APPROACH %'s:	TOTAL VOLUMES:		5:45 PM	5:30 PM	5:15 PM	5:00 PM	4:45 PM	4:30 PM	4:15 PM	4:00 PM	Contract of the last	₹	
	0.813	26		2.45%	43	2	4	ω	6	UI	00	7	2	00	Z	щ	
0.916	0.908	810	04:30 PM - 05:30 PM	89.68%	1573	N	191	193	223	201	202	184	179	200	NT	2	NORTH
16	0.702	73	05:30 PM	7.87%	138	NR	13	15	19	14	26	14	21	16	NR	posk	NORTHBOUND
	0.000	0		0.00%	0	N	0	0	0	0	0	0	0	0	Z	0	
	0.606	63		6.26%	114	SL	ω	14	7	17	13	26	12	22	2	2	
0.911	0.916	857		91.48%	1665	ST	199	224	189	234	221	213	199	186	ST	2	SOUTHBOUND
	0.750	24		2.25%	41	SR	00	ω	4	œ	Сī	7	ω	ω	SR	0	BOUND
	0.000	0		0.00%	0	SU	0	0	0	0	0	0	0	0	S	0	
	0.625	10		26.56%	17	핃	0	2	ω	0	ω	4	_	4	臣	<u>_</u>	
0.808	0.500	4		7.81%	5	티	0	0	0	2	0	2	<u>1</u>	0	甲	0.5	EASTBOUND
08	0.875	28		65.63%	42	ER	4	ω	00	00	ហ	7	2	ហ	됬	0.5	OUND
	0.000	0		0.00%	0	EU	0	0	0	0	0	0	0	0	E	0	
	0.767	442		54.94%	801	WL	94	109	91	129	78	144	77	79	WL	1.5	
0.7.	1.000	4		0.62%	9	TW	 	ω	<u>-</u>	<u></u>	just.		<u>-</u>	0	TW	0.5	WESTBOUND
0.758	0.746	385		44.44%	648	WR	52	43	86	110	60	129	65	103	WR	<u></u>	GNUOS
	0.000	0		0.00%	0	٧u	0	0	0	0	0	0	0	0	٧u	0	
0.323	0000	2726	TOTAL		5096	TOTAL	569	612	637	729	622	738	563	626	TOTAL		

Location: Pleasant Valley Rd & SR-101 EB Ramps City: Camarillo Control: Siganlized

Intersection Turning Movement Count

Project ID: 18-05719-003 Date: 11/27/2018

PEAK HR VOL: PEAK HR FACTOR:	PEAK HR:	TOTAL VOLUMES : APPROACH %'s :	5.40 PM	E-25 DM	5-30 PM	5:15 PM	5:00 PM	4:45 PM	4:30 PM	4:15 PM	4:00 PM		PM			PEAK HE SACTOR .	PEAK HK:	APPROACH %'s :	TOTAL VOLUMES:		8:45 AM	8:30 AM	8:15 AM	8:00 AM	7:45 AM	7:30 AM	7:15 AM	7:00 AM	AM		MS/EW Streets:
0.000		0.00%	Î			0	Ī				0	Г	0			990	Ť	0.00%	0	2	c			0		Τ		T	Z 0		
814 0.881	04;	1495 64.11%	OCT	100	163	181	205	197	231	163	199	Z				000	603	ľ	1119	4	103	113	117	162	179	215	127	103	N H		2
4 1.000 0.925	04:30 PM - 05:30 PM	NR 9 0.39%		> +	٠,		_	_	_	2	2	NR.	0	NORTHBOUND	0.823	0 417	15 AM - 08		10	NR.	2	0	×		ω	0	pa	2	Ř ○	NORTHROLIND	I madult valley ive
0.000	:30 PM	0.00%	c	0 0	0 1	0	0	0	0	0	0	2		UND		000	15 A	% 0.00%	0	1	c	0	0	0	0	0	0	0	Z 🗢		cy iwa
422 0.812		NT2 828 % 35.51%	90	101	101	130	98	91	103	95	112	NT2	0			704			779	NT2	66	68	84	101	88	134	138	100	NT2		
0.750		SL 14 0.48%	6) +	- ,	- 1	2	jui.	2	0	4	2	<u></u>		H	0 70	,	Т	13	SI	4	0	0	ω		0	2	w	S <u>⊢</u>		
976 0.897	i	ST 1874 % 63.61%	220	27.5	243	216	272	229	259	217	210	TS	2			003		1	1630	- 1	170	222	193	215	265	218	165	182	ST 2		
523 0.822 0.867		SR 1050 % 35.64%	LU9	31	144	116	159	114	134	130	144	SR		SOUTHBOUND	0.913			1	1392	- 1	142	144	155	173	187	223	206	162	SR ±	SOUTHBOILING	t tomount suncy in
0.375		SU 7 % 0.24%	-		0	0 1	2			2				OUND		20	,		7	1	pea	0	0						SI O		incy inc
0.250		SU2 1 9% 0.03%			0	0 (0	_	0	0	0	SU2	0			0 636		ľ	œ	1	2	0	0	2	0	2	pa)	_	SU2		
0 0.784		EL 1697 3% 89.22%	1/6	717	210	786	260	131	192	230					Н	678	-	8	1498	-	187	16:	197	189	239	200	202	1	P 15	1	
4 0.500	ı	ET 7 6 2% 0.32%	~) H		01			-		0					9			10	1	7 0		0			0			7.5 7.5		
98 0 0.875 0.796	d	ER 183 2% 9.62%	15	TO	101	24	27	19	28	22				EASTBOUND	0.912			18	299		28	31	28	53	4	ω.	ф 8			FASTROLIND	od ros to mino
0.000										0				QUND					0 (0								20	2	coming
9 00 0.563		3%			۱. د	<u> </u>		2 (_						15		8		1	6	0	4						- - 		
63 0.875		%			3 8		. ,	S 1	N	0		2 WL			-	2	-	Г	28	7	-	_	_	2		_			y - ĕ <u>⊢</u>		_
		WL W 11 (23.91% 0								_								8			_							ľ			
0.000 0.0		WT V 0 2 0.00% 63								0		ľ		WEST		0		%	0 :	1	0	0	0						₹ <u>`</u>	LS:#W	OU TOT
11 0.688 0.		WR V 29 63.04% C									- 1		0	BOLIND	0.688			64.00% (1	0	part.	w						WR I	BOIND	Tot Co Valilba
0.000 0.		0.00% 13	0							0						0		0.00% 1		ı									W		
0,500 0	-	WR2 6 13.04%	-	-		0 0	0 10	۱ د	7	0					-	1		12,00%		1	0	0	jab	0	0 0	2	-	+	S C		
0.000		S2L 0										S2L	0			0			ט גר	2								r	S o		
0.000		S2U 0								0		Γ				0			0 20										0		
0.000		S21.2	0	O	0	0 0	0 0	0	0 (0	0	S21.2	0 0	SHEE		0			ט ני	2	0	0	0	0	0 0	5 (00	2		SALLINS	
0.000		S2T2 0	0	0	0 0	0 0	0	0	0 0	0	0	S2T2	0 0	CONI		0			0 2/12	STS	0	0	0	0	0	5 (00	2	CLCS O C	COM	
0.000		SZRZ 0	0	0	0	0 0	0	0	0 0	0	0	S2R2	0		0.000	0			22.5	COCO	0	0	0	0	0 0	o (00	22142	0		
0.000		S2U2 0	0	0	0				0 0	0	0	S2U2	0		0.000	0		•	2026	2112	0	0	0	0	> 0	2 (00	2020	50		
3726	ATOTA	TOTAL 7226	801	896	926	1043	190	707	200	866	937	TOTAL			0.926	3854	TOTAL	00.00	6818	MEG	711	742	778	907	1000	100	897	722	MICH		

Location: Pleasant Valley Rd & SR-101 Washitersection Turning Movement Count City: Camarillo Control: Siganlized Control: Siganlized Control: Siganlized

	PEAK HR FACTOR:	PEAK HR :	APPROACH %'s:	TOTAL VOLUMES :		5.45 FM	E:45 DM	5:30 PM	5:15 PM	5:00 PM	4:45 PM	4:30 PM	4:15 PM	4:00 PM	The state of the s	۲N			PEAK HR FACTOR:	PEAK HR VOL:	PEAK HR:	APPROACH %'s:	TOTAL VOLUMES :		8:45 AM	8:30 AM	8:15 AM	8:00 AM	7:45 AM	7:30 AM	7:15 AM	7:00 AM	The state of the s	AΜ		NS/EW Streets:	
	0.000		0.00%	0	2	c	> 0	0	0	0	0	0	0	0	2	0			0.000	0		0.00%	0	2	0	0	0	0	0	0	0	0	Z	0			
0.843	0.838	04:30 PM -	90.16%	2922	3	223	300	366	407	440	274	354	375	3//	3	ω	NORTH	0.880	0.890	1399	07:15 AM -	91.77%	2409	4	256	251	286	335	393	380	291	217	N.	ω	NORTH	Pleasant Valley Rd	
43	0.695	12	9.84%	319	NR.	3	יים ל	23	34	50	30	64	33	60	둙		NORTHBOUND	80	0.777	115	08:15 AM	8.23%	216	NR.	30	25	19	20	37	30	28	27	NR.		NORTHBOUND	Valley Rd	
	0.000		0.00%	0 ;	E	c	0 0	0	0	0	0	0	0	0	2	0			0.000	0		0.00%	0 ह		0	0	0	0	0	0	0	0	N.	0			
	0.000	>	0.00%	0	2	c	0	0	0	0	0	0	0	0	2	0			0.000	0		0.00%	o ¦	Δ	0	0	0	0	0	0	0	0	SI.	0			
0.833	0.820	200	61.58%	2148	ST	233	100	270	225	335	228	311	251	293	ST	ω	SOUTH	0.887	0.914	1415		58.20%	2524	4	261	270	300	323	387	380	325	278	ST	ω	HTUOS	Pleasant Valley Ro	
33	0.856	3	38.42%	1340	æ	140	4 10	125	147	201	140	200	171	216	SR	2	SOUTHBOUND	87	0.848	940		41.80%	1813	8	217	248	232	205	277	238	220	176	SR	2	SOUTHBOUND	/alley Rd	
	000	,	0.00%	0 8	2	c	0 0	0	0	0	0	0	0	0	S	0			0.000	0		0.00%	ه ه	2	0	0	0	0	0	0	0	0	SU	0			i ocai
	0000	,		0 }	₽	c	0 0	0	0	0	0	0	0	0	四	0			0.000	0			o P	0	0	0	0	0	0	0	0	0	田	0			la l
0.000	000	,		o !		c	0 0	0 (0	0	0	0	0	0	П	0	EASTE		0.000	0			o <u>"</u>	1	0	0	0	0	0	0	0	0	四	0	EASTE	SR-101 WB Ramps	
0.000	000	,		o <u>;</u>	E	C	0 0	0 (0	0	0	0	0	0	Ð	0	EASTBOUND		0.000	0		,	o 9	9	0	0	0	0	0	0	0	0	贸	0	EASTBOUND	B Ramps	
0.000	000			o [c	0 0	0 (0	0	0	0	0	0	Ξ	0			0.000	0		,	- C		0	0	0	0	0	0	0	0	巴	0			
0.505	0 900		39.65%	780	š	105	101	101	113	109	101	88	87	76	WL	<u></u>			0.829	242		39.33%	479	i i	52	79	58	52	73	48	69	48	¥				
0.941	000		0.00%	<u>-</u>	Ä	O	0 0	0 (0	0	0	0	0	0	ĭ	0	WESTE	0.796	0.000	0		0.00%	⊃ ₹	Á	0	0	0	0	0	0	0	0	ş	0	WEST	SR-101 WB Ramps	
#1	594		60.35%	1187	W/B	143	100	199	154	137	158	145	140	122	₩R	2	WESTBOUND	96	0.778	420		60.67%	73g	5	83	79	96	110	135	90	85	61	WR	2	WESTBOUND	B Ramps	
0.000	000		0.00%	o	MI-	c	0 0	-	0	0	0	0	0	0	WU	0			0.000	0		0.00%	> 8		0	0	0	0	0	0	0	0	<u></u>	0			
0.874	4445	TOTAL	-	866	INTOT	9//	10/0	1073	1080	1272	931	1162	1057	1144	TOTAL			0.870		4531	TOTAL	o to o	8180 E		899	952	991	1045	1302	1166	1018	807	TOTAL				

Pleasant Valley Rd Bet. Lewis Rd & Pancho Rd

Day: Tuesday

Date: 11/27/2018

Project #: CA18_5720_001e City: Camarillo

East Bound

	,	Dir	Volume	PM Peak Hour	% PM	PM Volumes	Volume	AM Peak Hour	% AM	AM Volumes	% of Totals	Totals	23:00	22:00	21:00	20:00	19:00	18:00	17:00	16:00	15:00	14:00	13:00	12:00 PM	11:00	10:00	09:00	08:00	07:00	06:00	05:00	04:00	03:00	02:00	01:00	00:00 AM	Time
		Directional Peak Periods	3	16:00	0%	13	2	07:00	0%	7	0%	20	0	0	0	0	2	ω	1	ω	2	<u></u>	0	Д	1	0	⊢	2	2	1	0	0	0	0	0	0	#1
	All Classes	ak Periods	779	16:00	47%	4923	744	07:00	35%	3714	82%	8637	50	81	183	235	271	525	640	779	761	491	446	461	340	304	373	527	744	638	470	217	40	14	24	23	#2
1593	Volume		127	15:00	7%	704	128	07:00	6%	626	13%	1330	4	7	20	26	34	60	96	98	127	77	81	74	58	69	59	88	128	115	64	32	5	<u></u>	5	2	#3
1		AM 7-9	4	13:00	0%	11	з	07:00	0%	00	0%	19	0	0	0	0	0	0	2	0	1	2	4	2		0	2	2	w	0	0	0	0	0	0	0	#4
15%	%		41	15:00	3%	273	49	07:00	2%	245	5%	518	1	0	4	7	10	27	32	35	41	40	38	38	25	33	34	37	49	36	18	00	2	1	0	2	# 5
1156	Volume		3	12:00	0%	7	1	06:00	0%	5	0%	12	0	0	0	0	0	0	0	0	w	1	0	ω	1	0	ш	1		⊢	0	0	0	0	0	0	9#
1		NOON 12-2	1	15:00	0%	I .				0	0%		0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7 #
11%	%		3	12:00	0%	13	4	11:00	0%	10	0%	23	0	0	0	0	0		2	ω	1	2	1	ω	4	0	0	2	1	ω	0	0	0	0	0	0	8#
1692	Volume		3	12:00	0%	9	5	08:00	0%	00	%0	14	0	0	0	0	0	0		0	0	ш	1	ω	0	2	0	5	1	0	0	0	0	0	0	0	6#
1		PM 4-6				0				0			0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	#10
16%	%					0				0			0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	#11
6133	Volume	윷				0				0		Safety and	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	# 12
1		Off Peak Volumes				0				0							X									0											# 13
58%	%	- 1	937	15:00	56%		929	07:00	44%	4623	100%	10574	55		N		n						W.						H		552					27	Total

9 5-Axle Single Trailer:	6 3-Axle Single Units	6
8 <=4-Axle Single Trail	5 2-Axle, 6-Tire Single Units	5
7 >=4-Axle Single Unit	4 Buses	4
Classification Definitions		

Motorcycles
 Passenger Cars
 2-Axle, 4-Tire Single Units

9 5-Axle Single Trailers Axle Single Units

Axle Single Trailers

10 >=6-Axle Single Trailers11 <=5-Axle Multi-Trailers12 6-Axle Multi-Trailers

Pleasant Valley Rd Bet. Lewis Rd & Pancho Rd

Date: 11/27/2018 Day: Tuesday

City: Camarillo

Project #: CA18_5720_001w

		Dire	Volume	PM Peak Hour	% PM	PM Volumes	Volume	AM Peak Hour	% AM	AM Volumes	% of Totals	Totals	23:00	22:00	21:00	20:00	19:00	18:00	17:00	16:00	15:00	14:00	13:00	12:00 PM	11:00	10:00	09:00	08:00	07:00	06:00	05:00	04:00	03:00	02:00	01:00	00:00 AM	Time
		Directional Peak Periods	4	15:00	0%	11	3	06:00	0%	00	0%	19	0	0	0	0	0	0	2	2	4	ᆸ	⊢	1	2	0	1	1	1	ω	0	0	0	0	0	0	#1
	All Classes	k Periods	986	17:00	55%	5276	627	07:00	31%	2981	86%	8257	78	107	136	144	203	529	986	956	875	516	389	357	313	288	375	599	627	386	156	35	13	50	60	79	#2
1407	Volume		143	16:00	7%	678	74	07:00	4%	363	11%	1041	5	12	10	17	20	52	117	143	120	73	55	54	48	46	35	64	74	54	20	00	1	4	4	5	#3
†		AM 7-9	4	16:00	0%	11	1		0%	2	0%	13	0	0	0	0	0	┙	2	4	2	<u> </u>	-	0	0	0	0	0	<u></u>	0	0	0	0	0	0	1	# 4
15%	%		34	16:00	2%	169	21	07:00	1%	95	3%	264	1	<u>, , , , , , , , , , , , , , , , , , , </u>	دعو	ω	7	14	26	34	33	21	15	13	13	10	13	18	21	12	6	1	0	0	0	1	# 5
887	Volume		1	16:00	0%	1	1	08:00	0%	1	0%	2	0	0	0	0	0	0	0		0	0	0	0	0	0	0	р	0	0	0	0	0	0	0	0	#6
		NOON 12-2		1		0				0			0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	#7
9%	%		₽	14:00	0%	1	2	05:00	0%	4	0%	5	0	0	0	0	0	0	0	0	0	₽	0	0	1	0	_	0	0	0	2	0	0	0	0	0	#8
2273	Volume		1	13:00	0%	3	2	10:00	0%	2	0%	5	0	0	0	0	0	0	0	0	1	ы	1	0	0	2	0	0	0	0	0	0	0	0	0	0	# 9
1		PM 4-6				0				0			0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	# 10
24%	%					0				0			0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	#11
5039	Volume	Off				0				0																0											# 12
		Off Peak Volumes				0		The state of		0													A 10 10			0											# 13
7.7%	%	mes	1140	16:00	64%	10.0	724	07:00	36%	3456	100%	9606	H										Ú		ľ	346									d		Total

Motorcycles
 Passenger Cars
 2-Axle, 4-Tire Single Units

4 Buses
5 2-Axle, 6-Tire Single Units
6 3-Axle Single Units

Classification Definitions

7 >=4-Axle Single Units8 <=4-Axle Single Trailers9 5-Axle Single Trailers

10 >=6-Axle Single Trailers11 <=5-Axle Multi-Trailers12 6-Axle Multi-Trailers

Pleasant Valley Rd Bet. Pancho Rd & SR-101 EB Ramps

Day: Tuesday

Date: 11/27/2018

Project #: CA18_5720_002n City: Camarillo

North Bound

		Dir	Volume	PM Peak Hour	% PM	PM Volumes	Volume	AM Peak Hour	% AM	AM Volumes	% of Totals	Totals	23:00	22:00	21:00	20:00	19:00	18:00	17:00	16:00	15:00	14:00	13:00	12:00 PM	11:00	10:00	09:00	08:00	07:00	06:00	05:00	04:00	03:00	02:00	01:00	00:00 AM	Time
		Directional Peak Periods	Ľ	15:00	0%	3				0	0%	3	0	0	0	0	0	0	1	ш	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	#1
	All Classes	ak Periods	987	16:00	54%	6174	822	07:00	33%	3721	87%	9895	54	103	193	226	380	645	958	987	853	654	530	591	478	423	408	584	822	469	289	89	24	53	35	47	#2
1635	Volume		98	15:00	5%	571	94	07:00	4%	469	9%	1040	3	ш	7	10	27	43	98	80	98	68	68	60	62	43	54	80	94	64	50	14	ω	р	2	2	#3
†		AM 7-9				0	3	10:00	0%	7	0%	7	0	0	0	0	0	0	0	0	0	0	0	0	1	ω		0	1	0		0	0	0	0	0	#4
14%	%		31	15:00	2%	214	31	10:00	1%	170	3%	384	2	ш	6	7	9	28	20	27	31	28	26	29	23	31	26	25	28	18	12	4	0	2	0	1	#5
1311	Volume		ь	13:00	0%	1	33	09:00	0%	3	0%	4	0	0	0	0	0	0	0	0	0	0	1	0	0	0	ω.	0	0	0	0	0	0	0	0	0	# 6
‡		NOON 12-2				0				0		110 20 3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	#7
12%	%		3	12:00	0%	9	3	06:00	0%	10	0%	19	0	0	0	0	0	2	₽-	0	0	1	2	ω	0	ω	1	Ь	0	ω	0	0		0	0	1	#8
2182	Volume		ᆫ	12:00	0%	3	1	10:00	0%	2	0%	5	0	 2	0	0	0	0	0		0	0	0	Д	1		0	0	0	0	0	0	0	0	0	0	# 9
1		PM 4-6				0				0			0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	# 10
19%	%					0				0			0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	# 11
6229	Volume	Off				0				0			0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	# 12
†		Off Peak Volumes				0				0			0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	# 13
55%	%	nes	1104	16:00	61%	6975	945	07:00	39%	4382	100%	11357	59	106	206	243	416	718	1078	1104	983	751	627	684	565	504	493	690	945	554	352	107	28	56	37	51	Total

6	٥	4	
6 3-Axle Single Units	5 2-Axle, 6-Tire Single Units	Buses	
9	00	7	Classific
9 5-Axle Single Trailer	8 <=4-Axle Single Trail	7 >=4-Axle Single Uni	Classification Definitions

Motorcycles
 Passenger Cars
 2-Axle, 4-Tire Single Units

7 >=4-Axle Single Units
8 <=4-Axle Single Trailers
9 5-Axle Single Trailers

10 >=6-Axle Single Trailers11 <=5-Axle Multi-Trailers12 6-Axle Multi-Trailers

Pleasant Valley Rd Bet. Pancho Rd & SR-101 EB Ramps

Date: 11/27/2018 Day: Tuesday

Project #: CA18_5720_002s City: Camarillo

South Bound

			Volume	PM Peak Hour	% PM	PM Volumes	Volume	AM Peak Hour	%AM	AM Volumes		% of Totals	Totals	23:00	22:00	21:00	20:00	19:00	18:00	17:00	16:00	15:00	14:00	13:00	12:00 PM	11:00	10:00	09:00	08:00	07:00	06:00	05:00	04:00	03:00	02:00	01:00	00:00 AM	Time
		Directional Peak Periods	e 1	12:00	W 0%	4	e 2	ar 07:00	0%	25 7		ls 0%	s 11	0	0	0	0	0	0	0	1	1	0	1	1	1	1	1	1	2	1	0	0	0	0	0		#1
	All Classes	eak Periods	728	16:00	47%	4841	747	07:00	35%	3591		82%	8432	73	120	174	178	251	516	723	728	650	496	456	476	367	328	425	706	747	540	276	123	15	16	18	30	#2
1780	Volume		120	17:00	7%	727	113	07:00	5%	538		12%	1265	3	18	26	25	26	64	120	107	100	78	82	78	67	49	68	99	113	80	33	18	2	4	4	1	#3
1		AM 7-9	3	16:00	0%	10	3	07:00	0%	6		0%	16	0	0	0	0	0	0	1	ω	1	1	2	2	1	0	1	1	З	0	0	0	0	0	0	0	# 4
17%	%		59	16:00	3%	296	59	07:00	2%	230		5%	526	w	5	7	00	12	24	37	59	40	40	28	33	21	23	24	39	59	33	18	6	0	4	1	2	#5
1163	Volume	Z	2	13:00	0%	4	4	08:00	0%	9		0%	13	0	0	0	0	0	0	0	0		1	2	0	μ.	2	0	4	1	0		0	0	0	0	0	#6
1		NOON 12-2				0	1	08:00	0%	ω		0%	3	0	0	0	0	0	0	0	0	0	0	0	0	0	₽	р-	<u></u>	0	0	0	0	0	0	0	0	#7
11%	%		2	14:00	0%	4	2	07:00	0%	00	3	0%	12	0	0	0	0	0	0	Д.	0	0	2	1	0	0	⊢	р.	2	2	0	<u>⊢</u> ∆	0	0	0	0	1	#8
1781	Volume		1	13:00	0%	2	1	05:00	0%	2		0%	4	0	0	0	0	0	0	<u></u>	0	0	0	ר	0	ы	0	0	0	0	0	ы	0	0	0	0	0	#9
1		PM 4-6				0				0				0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	#10
17%	%					0		di di		0				0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	#11
5558	Volume	Off P				0				0				0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	# 12
1		Off Peak Volumes				0				0				0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	# 13
54%	%	S	898	16:00	57%	5888	927	07:00	43%	4394		100%	10282	79	143	207	211	289	604	883	898	793	618	573	590	459	405	521	853	927	654	330	147	17	24	23	34	Total

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	ın	_
	5 2-Axle, 6-Tire Single Units	4 Buses
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Motorcycles
 Passenger Cars
 2-Axle, 4-Tire Single Units

10 >=6-Axle Single Trailers11 <=5-Axle Multi-Trailers12 6-Axle Multi-Trailers

Pancho Rd Bet. Calle Quetzal & Howard Rd

Date: 11/27/2018 Day: Tuesday

Project #: CA18_5720_003n City: Camarillo

North Bound

		Dire	Volume	PM Peak Hour	% PM	PM Volumes	Volume	AM Peak Hour	%AM	AM Volumes		% of Totals	Totals	23:00	22:00	21:00	20:00	19:00	18:00	17:00	16:00	15:00	14:00	13:00	12:00 PM	11:00	10:00	09:00	08:00	07:00	06:00	05:00	04:00	03:00	02:00	01:00	00:00 AM	Time
	Þ	Directional Peak Periods	1	12:00	0%	1	Þ	11:00	0%	1		0%	2	0	0	0	0	0	0	0	0	0	0	0	Ц	1	0	0	0	0	0	0	0	0	0	0	0	#1
	All Classes	k Periods	56	16:00	51%	241	26	11:00	15%	70		66%	311	2	0	0	2	00	4	19	56	55	43	20	32	26	9	00	11	10	5	Ы	0	0	0	0	0	# 2
36	Volume		13	15:00	12%	58	5	09:00	3%	16		16%	74	0	0	0	0	1	ц	6	11	13	10	S	11	4	2	5		2			0	0	0	0	0	#3
1		AM 7-9	1	14:00	0%	2	1	06:00	0%	2		1%	4	0	0	0	0	0	0	0	1	0	<u></u>	0	0		0	0	0	0	1	0	0	0	0	0	0	#4
8%	%		10	15:00	8%	38	6	09:00	6%	28		14%	66	0	0	2	0	1	2	4	7	10	6	4	2	5	w	6	4	5	ω	2	0	0	0	0	0	#5
77	Volume			12	0%	2	1	10:00	0%	1		1%		0	0	0	0	0	0	0								0	0	0	0							#6
		NOON 12-2				0	1	11:00	0%	1		0%							0													iu iu						#7
16%	%	2					2	09:00	1%			1%							0							0.54												#8
104	Volume		2	12:0	9	0			6 1%	4		2														N. Y. S.												# 9
1		PM 4-6	-	0				0	8			%	9	Total St					0							To the same of the												# 10
22%	%					0				0									0																			#11
257	Volume	0				0		The second		0									0																			# 12
		Off Peak Volumes				0		THE PARTY OF THE P		0									0	The same														N.				#13
54%	%	umes	78	15:00		0 344	40	11:00	27%	0 130	9	100%	474	S					0 7															0				Total

9 5-Axle Single Trailer:	6 3-Axle Single Units	6
8 <=4-Axle Single Trail	5 2-Axle, 6-Tire Single Units	5
7 >=4-Axle Single Uni	4 Buses	4
Classification Definitions		

Motorcycles
 Passenger Cars
 2-Axle, 4-Tire Single Units

7 >=4-Axle Single Units8 <=4-Axle Single Trailers9 5-Axle Single Trailers

10 >=6-Axle Single Trailers11 <=5-Axle Multi-Trailers12 6-Axle Multi-Trailers

Pancho Rd Bet. Calle Quetzal & Howard Rd

Day: Tuesday

Date: 11/27/2018

City: Camarillo

Project #: CA18_5720_003s

South Bound

		Dire	Volume	PM Peak Hour	% PM	PM Volumes	Volume	AM Peak Hour	% AM	AM Volumes	% of Totals	Totals	23:00	22:00	21:00	20:00	19:00	18:00	17:00	16:00	15:00	14:00	13:00	12:00 PM	11:00	10:00	09:00	08:00	07:00	06:00	05:00	04:00	03:00	02:00	01:00	00:00 AM	Time
		Directional Peak Periods	1	12:00	0%	1	2	09:00	1%	4	1%	5	0	0	0	0	0	0	0	0	0	0	0	1	1	0	2		0	0	0	0	0	0	0	0	#1
	All Classes	ak Periods	32	12:00	31%	148	89	06:00	40%	187	71%	335	0	₽	2	2	6	11	11	17	26	21	19	32	27	19	13	16	17	89	5	1	0	0	0	0	# 2
56	Volume		9	12:00	6%	30	14	06:00	7%	34	14%	64	0	ш	0	0	0	2	4	ω	w	ω	(J	9	C5	1	2	Cī	S	14	2	0	0	0	0	0	#3
1		AM 7-9	1	14:00	0%	1	1	06:00	0%	1	0%	2	0	0	0	0	0	0	0	0	0	ш	0	0	0	0	0	0	0	ш	0	0	0	0	0	0	# 4
12%	%		7	13:00	4%	21	8	06:00	7%	32	11%	53	0	0	0	0	0	0	2	4	w	2	7	ω	6	ω	5	7	u	8	0	0	0	0	0	0	#5
79	Volume		—	12:00	0%	2	1	08:00	1%	3	1%	5	0																						0		#6
1		NOON 12-2				0				0									A 20 A 18 A		A.				1000										0		#7
17%	%		1	21:00	0%	1	1	07:00	0%	1	0%	2	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0		#8
41	Volume				0%	1		10:00	1%	5	1%		0		0	0	0	0	0	0	0		0			2					0		0		0		#9
1		PM 4-6				0				0			0	0	0	0	0	0	0	0	0	0	0		N. A. A. A. A. A. A. A. A. A. A. A. A. A.										0		# 10
9%	%					0				0																									0		#11
296	Volume	읓				0				0		14.00																							0		# 12
1		Off Peak Volumes				0				0																			B						0		# 13
63%	%	mes	47	12:00			113	06:00	57%	267	100%				w							27						30							0		Total

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6	ر.	4
6 3-Axle Single Units	2-Axle, 6-Tire Single Units	Buses

Motorcycles
 Passenger Cars
 2-Axle, 4-Tire Single Units

7 >=4-Axle Single Units8 <=4-Axle Single Trailers9 5-Axle Single Trailers

10 >=6-Axle Single Trailers11 <=5-Axle Multi-Trailers12 6-Axle Multi-Trailers

APPENDIX C

ICU 2003 Worksheets

EXISTING WORKSHEETS

Intersection Location: Pleasant Valley / Lewis Road
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

								4		_			
1	Movement	J	\rightarrow	7		←	L	7	T		4	1	4
		EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2	Lanes	2	2	1	2	2	1	1	2	1	1	2	1
3	Shared LT Lane (y/n)	Yes			Yes			Yes			Yes		
	Volume	206	588	18	400	344	222	50	285	257	131	666	204
5	Pedestrians			10			10			10			10
	Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes			✓ Yes	
	Pedestrian Timing Required		20			17			28			27	
	Free Right (y/n)			Yes			Yes			Yes			Yes
	Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
	Lost Time	3	4	4	3	4	4	3	4	4	3	4	4
	Minimum Green	4	10	10	4	10	10	4	10	10	4	10	10
	Reference Cycle Length	120											
	Volume Combined	206.0	588.0	18.0	400.0	344.0	222.0	50.0	285.0	257.0	131.0	666.0	204.0
14	Volume Separate Left	206.0	588.0		400.0	344.0		50.0	285.0		131.0	666.0	
	Lane Utilization Factor	0.971	0.952	1.000	0.971	0.952	1.000	1.000	0.952	1.000	1.000	0.952	1.000
16	Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
	Saturated Flow Combined	3505.3	3617.6	1615.0	3505.3	3617.6	1615.0	1805.0	3617.6	1615.0	1805.0	3617.6	1615.0
	Saturated Flow Separate	3505.3	3617.6		3505.3	3617.6		1805.0	3617.6		1805.0	3617.6	
19	Pedestrian Interference Time		0.0	1.2		0.0	1.2		0.0	1.2		0.0	1.2
	Pedestrian Frequency		28.3%			28.3%			28.3%			28.3%	
21	Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
22	Reference Time	7.1	19.5	1.3	13.7	11.4	16.5	3.3	9.5	19.1	8.7	22.1	15.2
23	Adjusted Reference Time	10.1	23.6	14.0	16.7	17.0	20.5	7.0	19.1	23.1	11.7	27.5	19.2
	Permitted Option												
	Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
	Volume Left Lane	103	294		200	172		50	143		131	333	
26	Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
	Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
	Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		1.07	1.00	
29	Permitted Sat Flow	116.8	1808.8		116.8	1808.8		120.3	1808.8		1925.3	1808.8	
30	Reference Time A	105.8	19.5		205.4	11.4		49.9	9.5		8.7	22.1	
31	Adjusted Saturation B		3617.6			3617.6			3617.6			3617.6	
	Reference Time B		NA			NA			NA			NA	
33	Reference Time Lefts	NA			NA			NA			NA		
34	Reference Time		105.8			205.4			49.9			22.1	
35	Adjusted Reference Time		109.8			209.4			53.9			27.5	
	Split Timing												
	Ref Time Combined		19.5			11.4			9.5			22.1	
	Ref Time By Movement	7.1	19.5		13.7	11.4		3.3	9.5		8.7	22.1	
	Reference Time	7.1	19.5		10.1	13.7		0.0	9.5		0.7	22.1	
	Adjusted Reference Time	23.6	23.6		18.6	18.6		19.1	19.1		27.5	27.5	
	Summary		West	North		. 5.5					_,.0		
	Protected Option).3	34									
	Permitted Option		9.4	53									
	Split Option		2.3	46									
	Minimum).3	34									
_	Combined		74										
_	Right Turns	EBR	WBR	NBR	SBR								
	Adjusted Reference Time	14.0	20.5	23.1	19.2								
	Cross Through Direction	NBT	SBT	WBT	EBT								
	Cross Through Adj Ref Time	19.1	27.5	17.0	23.6								
	Oncoming Left Direction	WBL	EBL	SBL	NBL								
	Oncoming Left Adj Ref Time	16.7	10.1	11.7	7.0								
	Combined Left Adj Rei Time	49.8	58.0	51.8	49.8								
	Intersection Capacity Utilizat			J1.0	→ ∂.0								
	Level Of Service	uon	62.4% B								Revision	2002.0	
52	Level Of Service		5								IVENIZIOU	2003.0	

Intersection Location: Pleasant Valley / Lewis Road
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

						A	_					
1 Movement	J	\rightarrow	7	F	+	L	7	T)	↓	1
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	2	2	1	2	2	1	_ 1	2	1	1	2	1
3 Shared LT Lane (y/n)	Yes			Yes			Yes			Yes		
4 Volume	205	398	17	319	777	269	89	541	453	121	417	269
5 Pedestrians			10			10			10			10
6 Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes			✓ Yes	
7 Pedestrian Timing Required		20			17			28			27	
8 Free Right (y/n)	1000	1000	Yes 1900	1000	1000	Yes	1000	1000	Yes 1000	1000	1000	Yes
9 Ideal Flow 10 Lost Time	1900	1900	1900	1900	1900 4	1900	1900	1900 4	1900 4	1900	1900	1900
11 Minimum Green	3	10	10	3	10	4 10	3	10	10	3	4 10	4 10
12 Reference Cycle Length	120	10	10	-	10	10	4	10	10	4	10	10
13 Volume Combined	205.0	398.0	17.0	319.0	777.0	269.0	89.0	541.0	453.0	121.0	417.0	269.0
14 Volume Separate Left	205.0	398.0	17.0	319.0	777.0	209.0	89.0	541.0	455.0	121.0	417.0	209.0
15 Lane Utilization Factor	0.971	0.952	1.000	0.971	0.952	1.000	1.000	0.952	1.000	1.000	0.952	1.000
16 Turning Factor Adjust	0.971	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	3505.3	3617.6	1615.0	3505.3	3617.6	1615.0	1805.0	3617.6	1615.0	1805.0	3617.6	1615.0
18 Saturated Flow Separate	3505.3	3617.6	10.0	3505.3	3617.6	10.0	1805.0	3617.6	10.0	1805.0	3617.6	10.0
19 Pedestrian Interference Time	0000.0	0.0	1.2	0000.0	0.0	1.2	1000.0	0.0	1.2	1000.0	0.0	1.2
20 Pedestrian Frequency		28.3%			28.3%			28.3%			28.3%	
21 Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
22 Reference Time	7.0	13.2	1.3	10.9	25.8	20.0	5.9	17.9	33.7	8.0	13.8	20.0
23 Adjusted Reference Time	10.0	19.1	14.0	13.9	29.8	24.0	8.9	24.8	37.7	11.0	21.6	24.0
Permitted Option												
24 Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	102.5	199		159.5	389		89	271		121	209	
26 Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		1.07	1.00	
29 Permitted Sat Flow	116.8	1808.8		116.8	1808.8		120.3	1808.8		1925.3	1808.8	
30 Reference Time A	105.3	13.2		163.8	25.8		88.8	17.9		8.0	13.8	
31 Adjusted Saturation B		3617.6			3617.6			3617.6			3617.6	
32 Reference Time B		NA			NA			NA			NA	
33 Reference Time Lefts	NA			NA			NA			NA		
34 Reference Time		105.3			163.8			88.8			13.8	
35 Adjusted Reference Time		109.3			167.8			92.8			21.6	
Split Timing		40.0			05.0			47.0			40.0	
36 Ref Time Combined	7.0	13.2		40.0	25.8			17.9		0 0	13.8	
37 Ref Time By Movement 38 Reference Time	7.0	13.2 13.2		10.9	25.8 25.8		5.9	17.9 17.9		8.0	13.8 13.8	
39 Adjusted Reference Time	19.1	19.1		29.8	29.8		24.8	24.8		21.6	21.6	
Summary	East		North		29.0		24.0	24.0		21.0	21.0	
40 Protected Option	39		35									
41 Permitted Option		7.8		2.8								
42 Split Option		3.9	46									
43 Minimum		0.8		5.8								
44 Combined		75										
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	14.0	24.0	37.7	24.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	24.8	21.6	29.8	19.1								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	13.9	10.0	11.0	8.9								
50 Combined	52.7	55.6	78.5	52.0								
51 Intersection Capacity Utilizat		65.4%			1							
52 Level Of Service		С								Revision	2003.0	

Intersection Location: Pleasant Valley / Pancho
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

							4			Ι.		
1 Movement	-	- FDT	TDD	₩DI	WDT	WDD		NDT	NDD	95	•	000
2 Lanes	EBL 1	EBT 2	EBR 0	WBL 2	WBT 2	WBR 0	NBL 1	NBT 1	NBR 1	SBL 1	SBT 1	SBR 0
3 Shared LT Lane (y/n)	Yes	2	U	☐ Yes		0	✓ Yes	1	1	☐ Yes	'	
4 Volume	11	905	154	294	668	10	74	2	74	26	2	48
5 Pedestrians			10			10		_	10			10
6 Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes	_		Yes	
7 Pedestrian Timing Required		17			17			23			0	
8 Free Right (y/n)			Yes			Yes			Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	3	4	4	3	4	4	2	3	3	2	3	3
11 Minimum Green	2	5	5	2	5	5	3	5	5	3	5	5
12 Reference Cycle Length	120											
13 Volume Combined	11.0	1059.0	0.0	294.0	678.0	0.0	0.0	76.0	74.0	26.0	50.0	0.0
14 Volume Separate Left	11.0	1059.0		294.0	678.0		74.0	2.0		26.0	50.0	
15 Lane Utilization Factor	1.000	0.952	1.000	0.971	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000
16 Turning Factor Adjust	0.950	0.978	0.850	0.950	0.998	0.850	0.950	0.951	0.850	0.950	0.856	0.850
17 Saturated Flow Combined	1805.0	3538.7	0.0	3505.3	3609.6	0.0	0.0	3615.0	1615.0	1805.0	1626.4	0.0
18 Saturated Flow Separate	1805.0	3538.7		3505.3	3609.6		3610.0	1900.0		1805.0	1626.4	
19 Pedestrian Interference Time		0.2	1.2		0.0	1.2		0.0	1.2		1.2	1.2
20 Pedestrian Frequency		28.3%			28.3%			28.3%			100.0%	
21 Protected Option Allowed	0.7	TRUE	0.0	10.1	TRUE	0.0	A10	FALSE		110	FALSE	
22 Reference Time	0.7	36.1	0.0	10.1	22.6	0.0	NA	NA	5.5	NA	NA	0.0
23 Adjusted Reference Time	5.0	40.1	9.0	13.1	26.6	9.0	NA	NA	8.5	NA	NA	8.0
Permitted Option	4	0.00		1	0.00		4	0.07		4	0.00	
24 Proportion Lefts 25 Volume Left Lane	1 11	0.00 530		147	0.00 339		1	0.97 76		26	0.00	
26 Proportion Lefts Left	1	0.00		147	0.00		1	0.97		1	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.1		0.9	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	0.07		1.07	1.00	
29 Permitted Sat Flow	120.3	1769.3		116.8	1804.8		0.0	245.2		1925.3	1626.4	
30 Reference Time A	11.0	36.1		151.0	22.6		0.0	37.2		1.7	4.9	
31 Adjusted Saturation B		3538.7		10110	3609.6		0.0	0.0			1626.4	
32 Reference Time B		NA			NA			10.5			4.9	
33 Reference Time Lefts	NA			NA			10.5			9.7		
34 Reference Time		36.1			151.0			10.5			4.9	
35 Adjusted Reference Time		40.1			155.0			17.1			8.0	
Split Timing												
36 Ref Time Combined		36.1			22.6			2.5			4.9	
37 Ref Time By Movement	0.7	36.1		10.1	22.6		2.5	0.1		1.7	4.9	
38 Reference Time		36.1			22.6			2.5			4.9	
39 Adjusted Reference Time	40.1	40.1		26.6	26.6		13.1	13.1		8.0	8.0	
Summary		West	North				· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·			· · · · · · · · · · · · · · · · · · ·	
40 Protected Option	53		N									
41 Permitted Option		5.0	17									
42 Split Option		5.7	21									
43 Minimum	53	3.2	17	.1								
44 Combined		70										
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	9.0	9.0	8.5	8.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	13.1	8.0	26.6	40.1								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	13.1	5.0	8.0	13.1								
50 Combined	35.2	22.0	43.1	61.2								
51 Intersection Capacity Utiliza	tion	58.5%								Davidada	2002.2	
52 Level Of Service		В								Revision	∠003.0	

Intersection Location: Pleasant Valley / Pancho
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

							4		_			
1 Movement	J	ightharpoons	7	F	+	L	7	T		4	↓	4
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	1	2	0	2	2	0	_ 1	1	1	1	1	0
3 Shared LT Lane (y/n)	Yes			Yes			✓ Yes			Yes		
4 Volume	26	810	73	63	857	24	442	4	385	10	4	28
5 Pedestrians			10			10			10		l lv	10
6 Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes			∐ Yes 0	
7 Pedestrian Timing Required 8 Free Right (y/n)		17	Yes		17	Yes		23	Yes		U	Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	3	1900	1900	3	1900	4	1900	3	3		3	3
11 Minimum Green	2	5		2	5	5	3	5	5		5	5
12 Reference Cycle Length	120		U		U	U	U	Ü		U	Ü	$\overline{}$
13 Volume Combined	26.0	883.0	0.0	63.0	881.0	0.0	0.0	446.0	385.0	10.0	32.0	0.0
14 Volume Separate Left	26.0	883.0		63.0	881.0	0.0	442.0	4.0	303.0	10.0	32.0	0.0
15 Lane Utilization Factor	1.000	0.952	1.000	0.971	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000
16 Turning Factor Adjust	0.950	0.988	0.850	0.950	0.996	0.850	0.950	0.950	0.850	0.950	0.869	0.850
17 Saturated Flow Combined	1805.0	3572.7	0.0	3505.3	3602.8	0.0	0.0	3611.7	1615.0		1650.6	0.0
18 Saturated Flow Separate	1805.0	3572.7		3505.3	3602.8		3610.0	1900.0		1805.0	1650.6	
19 Pedestrian Interference Time		0.1	1.2		0.0	1.2		0.0	1.2		1.1	1.2
20 Pedestrian Frequency		28.3%			28.3%			28.3%			100.0%	
21 Protected Option Allowed		TRUE			TRUE			FALSE			FALSE	
22 Reference Time	1.7	29.8	0.0	2.2	29.4	0.0	NA	NA	28.6	NA	NA	0.0
23 Adjusted Reference Time	5.0	33.8	9.0	5.2	33.4	9.0	NA	NA	31.6	NA	NA	8.0
Permitted Option												
24 Proportion Lefts	1	0.00		1	0.00		1	0.99		1	0.00	
25 Volume Left Lane	26	442		31.5	441		0	446		10	32	
26 Proportion Lefts Left	1	0.00		1	0.00		1	0.99		1	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	0.07		1.07	1.00	
29 Permitted Sat Flow	120.3	1786.4		116.8	1801.4		0.0	242.2		1925.3	1650.6	
30 Reference Time A	25.9	29.8		32.4	29.4		0.0	221.0		0.7	3.4	
31 Adjusted Saturation B		3572.7			3602.8			0.0			1650.6	
32 Reference Time B 33 Reference Time Lefts	NA	NA		NA	NA		22.7	22.8		8.7	3.4	
34 Reference Time	INA	29.8		INA	32.4		22.1	22.8		0.1	3.4	
35 Adjusted Reference Time		33.8			36.4			25.9			8.0	
Split Timing		33.0			30.4			25.9			0.0	
36 Ref Time Combined		29.8			29.4			14.8			3.4	
37 Ref Time By Movement	1.7	29.8		2.2	29.4		14.7	0.3		0.7	3.4	
38 Reference Time	1.7	29.8			29.4		17.7	14.8		0.7	3.4	
39 Adjusted Reference Time	33.8	33.8		33.4	33.4		20.1	20.1		8.0	8.0	
Summary		West	North									
40 Protected Option		3.9	N									
41 Permitted Option		6.4		5.9								
42 Split Option		'.1		3.1								
43 Minimum		6.4		i.9								
44 Combined		62	2.2									
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	9.0	9.0	31.6	8.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	20.1	8.0	33.4	33.8								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	5.2	5.0	8.0	20.1								
50 Combined	34.3	22.0	73.0	61.9								
51 Intersection Capacity Utiliza	tion	60.8%										
52 Level Of Service		В								Revision	2003.0	

Intersection Location: Pleasant Valley / US 101 SB Ramps
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

City: Ventura County
Alternative: Existing
Project: Pacific Rock

							_	4					
1	Movement	Ĵ	ightharpoons	7		←	L	7	1		L	I	4
'	ino venient	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2	Lanes	1	1	1	1	1	0	0	3	0	1	2	1
	Shared LT Lane (y/n)	✓ Yes			Yes			Yes			Yes		
	Volume	844	4	177	3	0	8	0	1144	5		863	789
5	Pedestrians			10			10			10			10
6	Ped Button (y/n)		✓ Yes			Yes			✓ Yes			✓ Yes	
7	Pedestrian Timing Required		14			0			14			14	
8	Free Right (y/n)			Yes			Yes			Yes			Yes
9	Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10	Lost Time	3	3	3	3	3	3	0	3.5	3.5	3	3.5	3.5
11	Minimum Green	4	4	4	4	4	4	0	10	10	4	10	10
12	Reference Cycle Length	120											
13	Volume Combined	0.0	848.0	177.0	3.0	8.0	0.0	0.0	1149.0	0.0	17.0	863.0	789.0
14	Volume Separate Left	844.0	4.0		3.0	8.0		0.0	1149.0		17.0	863.0	
15	Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	0.908	1.000	1.000	0.952	1.000
	Turning Factor Adjust	0.950	0.950	0.850	0.950	0.850	0.850	0.950	0.999	0.850	0.950	1.000	0.850
	Saturated Flow Combined	0.0	3610.9	1615.0	1805.0	1615.0	0.0	0.0	5172.2	0.0	1805.0	3617.6	1615.0
18	Saturated Flow Separate	3610.0	1900.0		1805.0	1615.0		0.0	5172.2		1805.0	3617.6	
	Pedestrian Interference Time		0.0	1.2		1.2	1.2		0.0	1.2		0.0	1.2
20	Pedestrian Frequency		28.3%			100.0%			28.3%			28.3%	
_	Protected Option Allowed		FALSE			FALSE			TRUE			TRUE	
	Reference Time	NA	NA	13.2	NA	NA	0.0	0.0	26.7	0.0	1.1	28.6	58.6
	Adjusted Reference Time	NA	NA	16.2	NA	NA	7.0	0.0	30.2	13.5	7.0	32.1	62.1
	Permitted Option												
	Proportion Lefts	1	1.00		1	0.00		1	0.00		1	0.00	
	Volume Left Lane	0	848		3	8		0	383		17	432	
	Proportion Lefts Left	1	1.00		1	0.00		1	0.00		1	0.00	
27	Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
	Left turn Factor	0.07	0.07		0.07	1.00		0.07	1.00		1.07	1.00	
	Permitted Sat Flow	0.0	241.5		120.3	1615.0		0.0	1724.1		1925.3	1808.8	
	Reference Time A	0.0	421.5		3.0	1.8		0.0	26.7		1.1	28.6	
	Adjusted Saturation B		0.0			1615.0			5172.2			3617.6	
	Reference Time B		36.2			1.8			NA			NA	
	Reference Time Lefts	36.1			8.2			NA			NA		
	Reference Time		36.2			3.0			26.7			28.6	
-	Adjusted Reference Time		39.2			7.0			30.2			32.1	
	Split Timing												
	Ref Time Combined		28.2			1.8			26.7			28.6	
	Ref Time By Movement	28.1	0.3		0.2	1.8		0.0	26.7		1.1	28.6	
	Reference Time	20.1	28.2		Ų. <u>P</u>	1.8		0.0	26.7			28.6	
	Adjusted Reference Time	31.2	31.2		7.0	7.0		30.2	30.2		32.1	32.1	
	Summary		West	North				J J J J	JJ.2		U	<u></u>	
_	Protected Option	N		37									
	Permitted Option		9.2	32									
	Split Option		3.2	62									
	Minimum		3.2	32									
	Combined		7.2										
	Right Turns	EBR	WBR	NBR	SBR								
	Adjusted Reference Time	16.2	7.0	13.5	62.1								
	Cross Through Direction	NBT	SBT	WBT	EBT								
	Cross Through Adj Ref Time	30.2	32.1	7.0	31.2								
41	Oncoming Left Direction	WBL	EBL	SBL	NBL								
	Oncoming Left Adj Ref Time	7.0	31.2	7.0	0.0								
	Combined	53.3	70.3	27.5	93.3								
	Intersection Capacity Utilizat		77.8%	21.0	00.0								
	Level Of Service		77.6% D								Revision	2003.0	
02	-0701 O1 O61 VICE		<u>'</u>								. 10 (131011	_000.0	

Intersection Location: Pleasant Valley / US 101 SB Ramps
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

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1 Movement	J	\rightarrow	1	—	Į	L				9	. ↓	—
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	✓ Yes	1	1	1	1	0	0	3	0	1	2	1
3 Shared LT Lane (y/n) 4 Volume	853	4	98	Yes 11	0	11	☐ Yes 0	1236	4	Yes 10	976	F02
5 Pedestrians	000	4	10	11	U	10	U	1230	4 10	10	976	523 10
6 Ped Button (y/n)		✓ Yes	10		Yes	10		✓ Yes	10		✓ Yes	10
7 Pedestrian Timing Required		14			0			14			14	
8 Free Right (y/n)			Yes			Yes			Yes		17	Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	3	3	3	3	3	3	0	3.5	3.5	3	3.5	3.5
11 Minimum Green	4	4	4	4	4	4	0	10	10	4	10	10
12 Reference Cycle Length	120											
13 Volume Combined	0.0	857.0	98.0	11.0	11.0	0.0	0.0	1240.0	0.0	10.0	976.0	523.0
14 Volume Separate Left	853.0	4.0		11.0	11.0		0.0	1240.0		10.0	976.0	
15 Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	0.908	1.000	1.000	0.952	1.000
16 Turning Factor Adjust	0.950	0.950	0.850	0.950	0.850	0.850	0.950	1.000	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	0.0	3610.9	1615.0	1805.0	1615.0	0.0	0.0	5173.1	0.0	1805.0	3617.6	1615.0
18 Saturated Flow Separate	3610.0	1900.0		1805.0	1615.0		0.0	5173.1		1805.0	3617.6	
19 Pedestrian Interference Time		0.0	1.2		1.2	1.2		0.0	1.2		0.0	1.2
20 Pedestrian Frequency		28.3%			100.0%			28.3%			28.3%	
21 Protected Option Allowed		FALSE			FALSE			TRUE			TRUE	
22 Reference Time	NA	NA	7.3	NA	NA	0.0	0.0	28.8	0.0	0.7	32.4	38.9
23 Adjusted Reference Time	NA	NA	10.3	NA	NA	7.0	0.0	32.3	13.5	7.0	35.9	42.4
Permitted Option												
24 Proportion Lefts	1	1.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	0	857		11	11		0	413		10	488	
26 Proportion Lefts Left	1 1 0	1.00		1 1 1 0	0.00		1 1	0.00		1	0.00	
27 Left turn Equivalents 28 Left turn Factor	15.0	15.0 0.07		15.0	15.0 1.00		15.0 0.07	15.0 1.00		0.9 1.07	15.0 1.00	
29 Permitted Sat Flow	0.07	241.4		0.07 120.3	1615.0		0.07	1724.4		1925.3	1808.8	
30 Reference Time A	0.0	425.9		11.0	2.1		0.0	28.8		0.7	32.4	
31 Adjusted Saturation B	0.0	0.0		11.0	1615.0		0.0	5173.1		0.7	3617.6	
32 Reference Time B		36.5			2.1			NA			NA	
33 Reference Time Lefts	36.4	00.0		8.7	2.1		NA	147.		NA	1.47.	
34 Reference Time	00.1	36.5		0.7	8.7		1,0	28.8		.,,	32.4	
35 Adjusted Reference Time		39.5			11.7			32.3			35.9	
Split Timing								,			,	
36 Ref Time Combined		28.5			2.1			28.8			32.4	
37 Ref Time By Movement	28.4	0.3		0.7	2.1		0.0	28.8		0.7	32.4	
38 Reference Time		28.5			2.1			28.8			32.4	
39 Adjusted Reference Time	31.5	31.5		7.0	7.0		32.3	32.3		35.9	35.9	
Summary	East	West	North	South								
40 Protected Option	N		39									
41 Permitted Option		9.5	35									
42 Split Option		3.5	68									
43 Minimum	38	3.5	35	.9								
44 Combined			.4									
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	10.3	7.0	13.5	42.4								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	32.3	35.9	7.0	31.5								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	7.0	31.5	7.0	0.0								
50 Combined	49.5	74.4	27.5	73.8								
51 Intersection Capacity Utiliza	ation	62.0%								Davidada	2002.2	
52 Level Of Service		В								Revision	∠003.0	

Intersection Location: Pleasant Valley / US 101 NB Off Ra
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

								4					
1	Movement	J		7	—	+	L		T		5	1	1
		EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2	Lanes	0	0	0	1	0	2	0	3	1	0	3	0
3	Shared LT Lane (y/n)	Yes			Yes			Yes			Yes		
	Volume	0	0	0	242	0	420	0	1399	115	0	1415	0
5	Pedestrians			0			0			0			0
6	Ped Button (y/n)		Yes			Yes			Yes			Yes	
7	Pedestrian Timing Required		0			0			0			0	
8	Free Right (y/n)			Yes			Yes			✓ Yes			Yes
9	Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10	Lost Time	0	0	0	3	0	3	0	3.5	3.5	0	3.5	0
11	Minimum Green	0	0	0	4	0	4	0	10	10	0	10	0
12	Reference Cycle Length	120											
13	Volume Combined	0.0	0.0	0.0	242.0	0.0	420.0	0.0	1399.0	115.0	0.0	1415.0	0.0
14	Volume Separate Left	0.0	0.0		242.0	0.0		0.0	1399.0		0.0	1415.0	
15	Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	0.885	1.000	0.908	1.000	1.000	0.908	1.000
16	Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
	Saturated Flow Combined	0.0	0.0	0.0	1805.0	0.0	2858.6	0.0	5175.6	1615.0	0.0	5175.6	0.0
	Saturated Flow Separate	0.0	0.0		1805.0	0.0		0.0	5175.6		0.0	5175.6	
19	Pedestrian Interference Time		0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0
	Pedestrian Frequency		0.0%			0.0%			0.0%			0.0%	
21	Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
22	Reference Time	0.0	0.0	0.0	16.1	0.0	17.6	0.0	32.4	8.5	0.0	32.8	0.0
23	Adjusted Reference Time	0.0	0.0	0.0	19.1	0.0	20.6	0.0	35.9	13.5	0.0	36.3	0.0
	Permitted Option												
24	Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
	Volume Left Lane	0	0		242	0		0	466		0	472	
26	Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
27	Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0	
28	Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		0.07	1.00	
29	Permitted Sat Flow	0.0	0.0		120.3	0.0		0.0	1725.2		0.0	1725.2	
30	Reference Time A	0.0	0.0		241.3	0.0		0.0	32.4		0.0	32.8	
	Adjusted Saturation B		0.0			0.0			5175.6			5175.6	
32	Reference Time B		0.0			0.0			NA			NA	
	Reference Time Lefts	0.0			24.1			NA			NA		
	Reference Time		0.0			24.1			32.4			32.8	
	Adjusted Reference Time		0.0			24.1			35.9			36.3	
	Split Timing												
	Ref Time Combined		0.0			0.0			32.4			32.8	
-	Ref Time By Movement	0.0	0.0		16.1	0.0		0.0	32.4		0.0	32.8	
	Reference Time		0.0			16.1			32.4			32.8	
_	Adjusted Reference Time	0.0	0.0		16.1	16.1		35.9	35.9		36.3	36.3	
	Summary		West	North							_	_	· <u> </u>
	Protected Option	19		36									
	Permitted Option		l.1	36									
	Split Option		6.1	72									
_	Minimum	16	5.1	36	.3								
	Combined		52										
	Right Turns	EBR	WBR	NBR	SBR								
	Adjusted Reference Time	0.0	20.6	13.5	0.0								
	Cross Through Direction	NBT	SBT	WBT	EBT								
	Cross Through Adj Ref Time	35.9	36.3	0.0	0.0								
	Oncoming Left Direction	WBL	EBL	SBL	NBL								
	Oncoming Left Adj Ref Time	16.1	0.0	0.0	0.0								
	Combined	52.0	56.9	13.5	0.0								
	Intersection Capacity Utiliza	tion	47.4%		_						_		
52	Level Of Service		Α								Revision	2003.0	

Intersection Location: Pleasant Valley / US 101 NB Off Ra
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

								_			_		
1	Movement		\rightarrow	7		—			1		L		
'	Wovement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	■ NBT	NBR	SBL	SBT	SBR
2	Lanes	0		0	1	0	2	0	3	1	0	3	0
	Shared LT Lane (y/n)	Yes			Yes			Yes	,		Yes		
4	Volume	0	0	0	411	0	594	0	1475	178		1099	0
5	Pedestrians			0			0			0			0
6	Ped Button (y/n)		Yes			Yes			Yes			Yes	
	Pedestrian Timing Required		0			0			0			0	
	Free Right (y/n)			Yes			Yes			✓ Yes			Yes
9	Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10	Lost Time	0	0	0	3	0	3	0	3.5	3.5	0	3.5	0
11	Minimum Green	0	0	0	4	0	4	0	10	10	0	10	0
12	Reference Cycle Length	120											
13	Volume Combined	0.0	0.0	0.0	411.0	0.0	594.0	0.0	1475.0	178.0	0.0	1099.0	0.0
14	Volume Separate Left	0.0	0.0		411.0	0.0		0.0	1475.0		0.0	1099.0	
15	Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	0.885	1.000	0.908	1.000	1.000	0.908	1.000
16	Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
17	Saturated Flow Combined	0.0	0.0	0.0	1805.0	0.0	2858.6	0.0	5175.6	1615.0	0.0	5175.6	0.0
	Saturated Flow Separate	0.0	0.0		1805.0	0.0		0.0	5175.6		0.0	5175.6	
	Pedestrian Interference Time		0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0
20	Pedestrian Frequency		0.0%			0.0%			0.0%			0.0%	
	Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
22	Reference Time	0.0	0.0	0.0	27.3	0.0	24.9	0.0	34.2	13.2	0.0	25.5	0.0
23	Adjusted Reference Time	0.0	0.0	0.0	30.3	0.0	27.9	0.0	37.7	16.7	0.0	29.0	0.0
	Permitted Option												
	Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
	Volume Left Lane	0	0		411	0		0	492		0	366	
26	Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
	Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0	
	Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		0.07	1.00	
	Permitted Sat Flow	0.0	0.0		120.3	0.0		0.0	1725.2		0.0	1725.2	
	Reference Time A	0.0	0.0		409.9	0.0		0.0	34.2		0.0	25.5	
_	Adjusted Saturation B		0.0			0.0			5175.6			5175.6	
	Reference Time B		0.0			0.0			NA			NA	
	Reference Time Lefts	0.0			35.3			NA			NA		
	Reference Time		0.0			35.3			34.2			25.5	
	Adjusted Reference Time		0.0			35.3			37.7			29.0	
	Split Timing												
	Ref Time Combined		0.0		07.5	0.0			34.2			25.5	
	Ref Time By Movement	0.0	0.0		27.3	0.0		0.0	34.2		0.0	25.5	
	Reference Time	^ ^	0.0		07.0	27.3		07.7	34.2		00.0	25.5	
39	Adjusted Reference Time	0.0	0.0	N. 41	27.3	27.3		37.7	37.7		29.0	29.0	
40	Summary Drate stad Ontion		West	North									
	Protected Option		0.3	37									
	Permitted Option		5.3	37									
	Split Option		7.3	66									
	Minimum	21	7.3	37	.1								
44	Combined	- EDD	65		000								
4 =	Right Turns	EBR	WBR	NBR	SBR								
	Adjusted Reference Time	0.0	27.9	16.7	0.0								
	Cross Through Adi Rof Time	NBT	SBT	WBT	EBT								
	Cross Through Adj Ref Time Oncoming Left Direction	37.7 WBL	29.0 EBL	0.0 SBL	0.0 NBL								
	Oncoming Left Adj Ref Time	27.3	0.0	0.0	0.0								
	Combined Leit Adj Rei Time	65.0	56.9	16.7	0.0								
	Intersection Capacity Utiliza			10.7	0.0	l							
	Level Of Service	แบบ	54.2%								Revision	2002.0	
52	Level Of Service		Α								Revision	∠003.0	

EXISTING PLUS PROJECT WORKSHEETS

Intersection Location: Pleasant Valley / Lewis Road
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

2 Lanes	J		L		1	1	t	—		7	\rightarrow	1	1 Movement
2 2 1 2 2 1 1 2 1 1	SBR	SBT	SBL	NBR	■ NBT	NBL	WBR	WBT	WBL	EBR	EBT	EBL	Wovement
Volume V		2											2 Lanes
4 Volume			Yes			Yes			Yes			Yes	3 Shared LT Lane (y/n)
6 Pedestrian Timing Required 2 Ves 2 Ves 17	6 204	666	135	261	285	50	226	348	404	18	597	206	. ,
T Pedestrian Timing Required S Free Right (y/n) S Uves	10			10			10			10			5 Pedestrians
S Free Right (y/n)		✓ Yes			✓ Yes			✓ Yes			✓ Yes		6 Ped Button (y/n)
Section 1900 1000	7	27			28			17			20		7 Pedestrian Timing Required
10 Lost Time	Yes			Yes			Yes			Yes			8 Free Right (y/n)
11 Minimum Green	0 1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	9 Ideal Flow
12 Reference Cycle Length 120 13 Volume Combined 206.0 597.0 18.0 404.0 348.0 226.0 50.0 285.0 261.0 135.0 666.0 135.0 135.0 666.0 135.0 666.0 135.0 135.0 666.0 135.0 135.0 666.0 135.0 135.0 140	4 4	4	3	4	4	3	4	4	3	4	4	3	10 Lost Time
13 Volume Combined 206.0 597.0 18.0 404.0 348.0 226.0 50.0 285.0 261.0 135.0 666.0 14 Volume Separate Left 206.0 597.0 404.0 348.0 50.0 285.0 261.0 135.0 666.0 15 Lane Utilization Factor 0.971 0.952 1.000 0.971 0.952 1.000 0.952 1.000 0.952 16 Turning Factor Adjust 0.950 1.000 0.880 0.950 1.000 0.952 1.000 0.000 1.	0 10	10	4	10	10	4	10	10	4	10	10	4	11 Minimum Green
14 Volume Separate Left 206.0 597.0 404.0 348.0 50.0 285.0 135.0 666.0 15 Lane Utilization Factor 0.971 0.952 1.000 0.971 0.952 1.000 0.850 0.950 1.000 0.000 1.												120	12 Reference Cycle Length
15 Lane Utilization Factor 0.971 0.952 1.000 0.971 0.952 1.000 1.000 0.952 1.000 1.000 0.952 1.000 0.850 0.950 1.000 0.000 1.000 1.200 1.200 0.000 1.200	0 204.0	666.0	135.0	261.0	285.0	50.0	226.0	348.0	404.0	18.0	597.0	206.0	13 Volume Combined
Turning Factor Adjust		666.0	135.0		285.0	50.0		348.0	404.0		597.0	206.0	14 Volume Separate Left
17 Saturated Flow Combined 3505.3 3617.6 1615.0 3505.3 3617.6 1615.0 1805.0 3617.6 3617.6	2 1.000	0.952	1.000	1.000	0.952	1.000	1.000	0.952	0.971	1.000	0.952	0.971	15 Lane Utilization Factor
17 Saturated Flow Combined 3505.3 3617.6 1615.0 3505.3 3617.6 1615.0 1805.0 3617.6 3617.6		1.000			1.000			1.000			1.000	0.950	
19 Pedestrian Interference Time 2.0 1.2 2.0 1.2 2.0 1.2 2.0 2.8 28.3%		3617.6		1615.0	3617.6		1615.0	3617.6		1615.0		3505.3	17 Saturated Flow Combined
20 Pedestrian Frequency	6	3617.6	1805.0		3617.6	1805.0		3617.6	3505.3		3617.6	3505.3	18 Saturated Flow Separate
Protected Option Allowed TRUE T		0.0		1.2			1.2			1.2			19 Pedestrian Interference Time
22 Reference Time 7.1 19.8 1.3 13.8 11.5 16.8 3.3 9.5 19.4 9.0 22.1	6	28.3%			28.3%			28.3%			28.3%		20 Pedestrian Frequency
23 Adjusted Reference Time 10.1 23.9 14.0 16.8 17.1 20.8 7.0 19.1 23.4 12.0 27.5 Permitted Option 24 Proportion Lefts 1 0.00 1 0.00 1 0.00 1 0.00 25 Volume Left Lane 103 299 202 174 50 143 135 333 26 Proportion Lefts Left 1 0.00 1 0.00 1 0.00 1 0.00 27 Left turn Equivalents 15.0 15.0 15.0 15.0 15.0 15.0 15.0 0.9 15.0 28 Left turn Factor 0.07 1.00 0.07 1.00 0.07 1.00 0.07 1.00 1.07 1.00 29 Permitted Sat Flow 116.8 1808.8 116.8 1808.8 120.3 1808.8 1925.3 1808.8 30 Reference Time A 105.8 19.8 207.5 11.5 49.9 9.5 9.0 22.1 31 Adjusted Saturation B 3617.6 3617.6 3617.6 3617.6 3617.6 32 Reference Time Lefts NA NA NA NA NA 34 Reference Time Lefts NA NA NA NA NA 35 Adjusted Reference Time 105.8 207.5 49.9 22.1 36 Ref Time Combined 19.8 13.8 11.5 3.3 9.5 9.0 22.1 37 Ref Time By Movement 7.1 19.8 13.8 11.5 3.3 9.5 9.0 22.1 38 Reference Time By Summary East West North South 40.7 34.5 40 Protected Option 42.6 46.6 46.6 40.6 40.7 34.5 41 Permitted Option 42.6 46.6 46.6 40.6 40.7 34.5 42 Split Turns EBR WBR NBR SBR	E	TRUE			TRUE			TRUE			TRUE		21 Protected Option Allowed
Permitted Option	1 15.2	22.1	9.0	19.4	9.5	3.3	16.8	11.5	13.8	1.3	19.8	7.1	22 Reference Time
24 Proportion Lefts	5 19.2	27.5	12.0	23.4	19.1	7.0	20.8	17.1	16.8	14.0	23.9	10.1	23 Adjusted Reference Time
25 Volume Left Lane 103 299 202 174 50 143 135 333 326 Proportion Lefts Left 1 0.00 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 1 0.00 0													Permitted Option
26 Proportion Lefts Left	0	0.00	1		0.00	1		0.00	1		0.00	1	24 Proportion Lefts
27 Left turn Equivalents	3	333	135		143	50		174	202		299	103	
28 Left turn Factor	0	0.00	1		0.00	1		0.00	1		0.00	1	26 Proportion Lefts Left
28 Left turn Factor	0	15.0	0.9		15.0	15.0		15.0	15.0		15.0	15.0	
30 Reference Time A 105.8 19.8 207.5 11.5 49.9 9.5 9.0 22.1		1.00	1.07			0.07			0.07			0.07	
30 Reference Time A 105.8 19.8 207.5 11.5 49.9 9.5 9.0 22.1	8	1808.8	1925.3		1808.8	120.3		1808.8	116.8		1808.8	116.8	29 Permitted Sat Flow
31 Adjusted Saturation B 3617.6 3	1	22.1	9.0		9.5	49.9			207.5		19.8		30 Reference Time A
NA NA NA NA NA NA NA NA		3617.6			3617.6						3617.6		31 Adjusted Saturation B
34 Reference Time 105.8 207.5 49.9 22.1 35 Adjusted Reference Time 109.8 211.5 53.9 27.5 Split Timing 36 Ref Time Combined 19.8 11.5 9.5 22.1 37 Ref Time By Movement 7.1 19.8 13.8 11.5 3.3 9.5 9.0 22.1 38 Reference Time 19.8 13.8 9.5 22.1 39 Adjusted Reference Time 23.9 23.9 18.7 18.7 19.1 19.1 27.5 27.5 Summary East West North South 40 Protected Option 40.7 34.5 41 Permitted Option 211.5 53.9 42 Split Option 42.6 46.6 43 Minimum 40.7 34.5 44 Combined 75.2 Right Turns EBR WBR NBR SBR	A	NA			NA			NA			NA		32 Reference Time B
35 Adjusted Reference Time			NA			NA			NA			NA	33 Reference Time Lefts
Split Timing 19.8 11.5 9.5 22.1	1	22.1			49.9			207.5			105.8		34 Reference Time
36 Ref Time Combined 19.8 11.5 9.5 22.1 37 Ref Time By Movement 7.1 19.8 13.8 11.5 3.3 9.5 9.0 22.1 38 Reference Time 19.8 13.8 9.5 22.1 39 Adjusted Reference Time 23.9 23.9 18.7 18.7 19.1 19.1 19.1 27.5 27.5 Summary East West North South 40 Protected Option 40.7 34.5 41 Permitted Option 211.5 53.9 42 Split Option 42.6 46.6 43 Minimum 40.7 34.5 44 Combined 75.2 Right Turns EBR WBR NBR SBR	5	27.5			53.9			211.5			109.8		35 Adjusted Reference Time
36 Ref Time Combined 19.8 11.5 9.5 22.1 37 Ref Time By Movement 7.1 19.8 13.8 11.5 3.3 9.5 9.0 22.1 38 Reference Time 19.8 13.8 9.5 22.1 39 Adjusted Reference Time 23.9 23.9 18.7 18.7 19.1 19.1 19.1 27.5 27.5 Summary East West North South 40 Protected Option 40.7 34.5 41 Permitted Option 211.5 53.9 42 Split Option 42.6 46.6 43 Minimum 40.7 34.5 44 Combined 75.2 Right Turns EBR WBR NBR SBR													
37 Ref Time By Movement 7.1 19.8 13.8 11.5 3.3 9.5 9.0 22.1 38 Reference Time 19.8 13.8 9.5 22.1 39 Adjusted Reference Time 23.9 23.9 18.7 18.7 19.1 19.1 19.1 27.5 27.5 Summary East West North South 40 Protected Option 40.7 34.5 41 Permitted Option 211.5 53.9 42 Split Option 42.6 46.6 43 Minimum 40.7 34.5 44 Combined 75.2 Right Turns EBR WBR NBR SBR	1	22.1			9.5			11.5			19.8		
39 Adjusted Reference Time 23.9 23.9 18.7 18.7 19.1 19.1 27.5 27.5 Summary	1	22.1	9.0		9.5	3.3			13.8		19.8	7.1	
Summary East West North South 40 Protected Option 40.7 34.5 41 Permitted Option 211.5 53.9 42 Split Option 42.6 46.6 43 Minimum 40.7 34.5 44 Combined 75.2 Right Turns EBR WBR NBR SBR	1	22.1			9.5			13.8			19.8		38 Reference Time
40 Protected Option 40.7 34.5 41 Permitted Option 211.5 53.9 42 Split Option 42.6 46.6 43 Minimum 40.7 34.5 44 Combined 75.2 Right Turns EBR WBR NBR SBR	5	27.5	27.5		19.1	19.1		18.7	18.7		23.9	23.9	39 Adjusted Reference Time
40 Protected Option 40.7 34.5 41 Permitted Option 211.5 53.9 42 Split Option 42.6 46.6 43 Minimum 40.7 34.5 44 Combined 75.2 Right Turns EBR WBR NBR SBR								,	South	North	West	East	
41 Permitted Option 211.5 53.9 42 Split Option 42.6 46.6 43 Minimum 40.7 34.5 44 Combined 75.2 Right Turns EBR WBR NBR SBR									.5	34).7	40	
42 Split Option 42.6 46.6 43 Minimum 40.7 34.5 44 Combined 75.2 Right Turns EBR WBR NBR SBR													
43 Minimum 40.7 34.5 44 Combined 75.2 Right Turns EBR WBR NBR SBR													42 Split Option
44 Combined 75.2 Right Turns EBR WBR NBR SBR									.5	34			
Right Turns EBR WBR NBR SBR													
									SBR			EBR	
43 Aujusteu Neierence Time 14.0 20.0 23.4 13.2									19.2	23.4	20.8	14.0	45 Adjusted Reference Time
46 Cross Through Direction NBT SBT WBT EBT													
47 Cross Through Adj Ref Time 19.1 27.5 17.1 23.9													
48 Oncoming Left Direction WBL EBL SBL NBL													48 Oncoming Left Direction
49 Oncoming Left Adj Ref Time 16.8 10.1 12.0 7.0													
50 Combined 49.9 58.3 52.5 50.0													
51 Intersection Capacity Utilization 62.6%								ı					
52 Level Of Service B Revision 2003.0		2003.0	Revision										

Intersection Location: Pleasant Valley / Lewis Road
Analyzed by: VRPA Technologies, Inc
PM Peak

								_			_		
1	Movement		\rightarrow	7		←	1		1			L	
'	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2	Lanes	2	2	1	2	2	1	1	2	1	1	2	1 1
	Shared LT Lane (y/n)	Yes		•	☐ Yes		'	Yes		•	Yes		•
_	Volume	205	400	17	321	784	271	89	541	455	123	417	269
	Pedestrians	200	100	10	021	701	10		011	10	120		10
_	Ped Button (y/n)		✓ Yes	. •		✓ Yes			✓ Yes			✓ Yes	
	Pedestrian Timing Required		20			17			28			- 27	
	Free Right (y/n)			Yes			Yes			Yes			Yes
	Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	 1900
10	Lost Time	3	4	4	3	4	4	3	4	4	3	4	4
11	Minimum Green	4	10	10	4	10	10	4	10	10	4	10	10
12	Reference Cycle Length	120											
13	Volume Combined	205.0	400.0	17.0	321.0	784.0	271.0	89.0	541.0	455.0	123.0	417.0	269.0
14	Volume Separate Left	205.0	400.0		321.0	784.0		89.0	541.0		123.0	417.0	
15	Lane Utilization Factor	0.971	0.952	1.000	0.971	0.952	1.000	1.000	0.952	1.000	1.000	0.952	1.000
16	Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
	Saturated Flow Combined	3505.3	3617.6	1615.0	3505.3	3617.6	1615.0	1805.0	3617.6	1615.0	1805.0	3617.6	1615.0
	Saturated Flow Separate	3505.3	3617.6		3505.3	3617.6		1805.0	3617.6		1805.0	3617.6	
19	Pedestrian Interference Time		0.0	1.2		0.0	1.2		0.0	1.2		0.0	1.2
	Pedestrian Frequency		28.3%			28.3%			28.3%			28.3%	
	Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
22	Reference Time	7.0	13.3	1.3	11.0	26.0	20.1	5.9	17.9	33.8	8.2	13.8	20.0
23	Adjusted Reference Time	10.0	19.2	14.0	14.0	30.0	24.1	8.9	24.8	37.8	11.2	21.6	24.0
	Permitted Option												
	Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
	Volume Left Lane	102.5	200		160.5	392		89	271		123	209	
	Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
	Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
	Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		1.07	1.00	
	Permitted Sat Flow	116.8	1808.8		116.8	1808.8		120.3	1808.8		1925.3	1808.8	
	Reference Time A	105.3	13.3		164.8	26.0		88.8	17.9		8.2	13.8	
_	Adjusted Saturation B		3617.6			3617.6			3617.6			3617.6	
_	Reference Time B	210	NA		N10	NA		A10	NA		210	NA	
	Reference Time Lefts	NA	405.0		NA	404.0		NA	00.0		NA	42.0	
	Reference Time		105.3			164.8			88.8			13.8	
	Adjusted Reference Time		109.3			168.8			92.8			21.6	
	Split Timing Ref Time Combined		42.2			26.0			47.0			12.0	
	Ref Time Combined Ref Time By Movement	7.0	13.3 13.3		11.0	26.0 26.0		5.9	17.9 17.9		8.2	13.8 13.8	
	Reference Time	7.0	13.3		11.0	26.0		ວ.ອ	17.9		0.2	13.8	
	Adjusted Reference Time	19.2	19.2		30.0	30.0		24.8	24.8		21.6	21.6	
	Summary		West	North		30.0		24.0	24.0		21.0	21.0	
_	Protected Option).0	36									
	Permitted Option		8.8	92									
	Split Option		0.0	46									
	Minimum		0.0	36									
	Combined		76		-								
	Right Turns	EBR	WBR	NBR	SBR								
	Adjusted Reference Time	14.0	24.1	37.8	24.0								
	Cross Through Direction	NBT	SBT	WBT	EBT								
	Cross Through Adj Ref Time	24.8	21.6	30.0	19.2								
	Oncoming Left Direction	WBL	EBL	SBL	NBL								
	Oncoming Left Adj Ref Time	14.0	10.0	11.2	8.9								
	Combined	52.8	55.7	79.0	52.1								
	Intersection Capacity Utiliza	tion	65.8%			ı							
	Level Of Service		C								Revision	2003.0	

Intersection Location: Pleasant Valley / Pancho
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

							4					
1 Movement			•	4			, ,	II.		95	•	200
2 Lanes	EBL 1	EBT 2	EBR 0	WBL 2	WBT 2	WBR 0	NBL 1	NBT 1	NBR 1	SBL 1	SBT 1	SBR 0
3 Shared LT Lane (y/n)	Yes		U	☐ Yes		U	✓ Yes	I	1	☐ Yes	<u> </u>	U
4 Volume	11	905	170	365	668	10	85	2	138	26	2	48
5 Pedestrians		000	10	000	000	10	00		100	20		10
6 Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes			Yes	
7 Pedestrian Timing Required		–			17			23			_ 0	
8 Free Right (y/n)			Yes			Yes			Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	 1900	1900	1900	1900
10 Lost Time	3	4	4	3	4	4	2	3	3	2	3	3
11 Minimum Green	2	5	5	2	5	5	3	5	5	3	5	5
12 Reference Cycle Length	120											
13 Volume Combined	11.0	1075.0	0.0	365.0	678.0	0.0	0.0	87.0	138.0	26.0	50.0	0.0
14 Volume Separate Left	11.0	1075.0		365.0	678.0		85.0	2.0		26.0	50.0	
15 Lane Utilization Factor	1.000	0.952	1.000	0.971	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000
16 Turning Factor Adjust	0.950	0.976	0.850	0.950	0.998	0.850	0.950	0.951	0.850	0.950	0.856	0.850
17 Saturated Flow Combined	1805.0	3531.8	0.0	3505.3	3609.6	0.0	0.0	3614.4	1615.0	1805.0	1626.4	0.0
18 Saturated Flow Separate	1805.0	3531.8		3505.3	3609.6		3610.0	1900.0		1805.0	1626.4	
19 Pedestrian Interference Time		0.2	1.2		0.0	1.2		0.0	1.2		1.2	1.2
20 Pedestrian Frequency		28.3%			28.3%			28.3%			100.0%	
21 Protected Option Allowed		TRUE			TRUE			FALSE			FALSE	
22 Reference Time	0.7	36.7	0.0	12.5	22.6	0.0	NA	NA	10.3	NA	NA	0.0
23 Adjusted Reference Time	5.0	40.7	9.0	15.5	26.6	9.0	NA	NA	13.3	NA	NA	8.0
Permitted Option												
24 Proportion Lefts	1	0.00		1	0.00		1	0.98		1	0.00	
25 Volume Left Lane	11	538		182.5	339		0	87		26	50	
26 Proportion Lefts Left	1	0.00		1	0.00		1	0.98		1	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.1		0.9	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	0.07		1.07	1.00	
29 Permitted Sat Flow	120.3	1765.9		116.8	1804.8		0.0	244.6		1925.3	1626.4	
30 Reference Time A	11.0	36.7		187.4	22.6		0.0	42.7		1.7	4.9	
31 Adjusted Saturation B		3531.8			3609.6			0.0			1626.4	
32 Reference Time B 33 Reference Time Lefts	NIA	NA		NA	NA		40.0	10.9		0.7	4.9	
34 Reference Time Letts	NA	36.7		NA NA	107.4		10.8	10.9		9.7	4.0	
35 Adjusted Reference Time		40.7			187.4 191.4			17.3			4.9 8.0	
Split Timing		40.7			191.4			17.3			0.0	
36 Ref Time Combined		36.7			22.6			2.9			4.9	
37 Ref Time By Movement	0.7	36.7		12.5	22.6		2.8	0.1		1.7	4.9	
38 Reference Time	0.7	36.7		12.5	22.6		2.0	2.9		1.7	4.9	
39 Adjusted Reference Time	40.7	40.7		26.6	26.6		13.1	13.1		8.0	8.0	
Summary	East	_	North		20.0		10.1	10.1		0.0	0.0	
40 Protected Option	56		N									
41 Permitted Option		1.4	17									
42 Split Option		7.3	21									
43 Minimum		5.2	17									
44 Combined		73		-								
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	9.0	9.0	13.3	8.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	13.1	8.0	26.6	40.7								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	15.5	5.0	8.0	13.1								
50 Combined	37.6	22.0	47.8	61.8								
51 Intersection Capacity Utiliza		61.3%	-	-								
52 Level Of Service	-	В								Revision	2003.0	
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Intersection Location: Pleasant Valley / Pancho
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

	•						4			Ι.		
1 Movement		EDT	TDD	₩	WDT	WDD	ND!	NDT	NDD	951	↓	000
2 Lanes	EBL 1	EBT 2	EBR 0	WBL 2	WBT 2	WBR 0	NBL 1	NBT 1	NBR 1	SBL 1	SBT 1	SBR 0
3 Shared LT Lane (y/n)	Yes		U	Yes	2	J	✓ Yes	'	1	☐ Yes		U
4 Volume	26	810	79	95	857	24	453	4	424	10	4	28
5 Pedestrians		0.0	10		00.	10	.00	·	10		·	10
6 Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes			Yes	
7 Pedestrian Timing Required		17			17			23				
8 Free Right (y/n)			Yes			Yes			Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	3	4	4	3	4	4	2	3	3	2	3	3
11 Minimum Green	2	5	5	2	5	5	3	5	5	3	5	5
12 Reference Cycle Length	120											
13 Volume Combined	26.0	889.0	0.0	95.0	881.0	0.0	0.0	457.0	424.0	10.0	32.0	0.0
14 Volume Separate Left	26.0	889.0		95.0	881.0		453.0	4.0		10.0	32.0	
15 Lane Utilization Factor	1.000	0.952	1.000	0.971	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000
16 Turning Factor Adjust	0.950	0.987	0.850	0.950	0.996	0.850	0.950	0.950	0.850	0.950	0.869	0.850
17 Saturated Flow Combined	1805.0	3569.4	0.0	3505.3	3602.8	0.0	0.0	3611.7	1615.0	1805.0	1650.6	0.0
18 Saturated Flow Separate	1805.0	3569.4		3505.3			3610.0	1900.0		1805.0	1650.6	
19 Pedestrian Interference Time		0.1	1.2		0.0	1.2		0.0	1.2		1.1	1.2
20 Pedestrian Frequency		28.3%			28.3%			28.3%			100.0%	
21 Protected Option Allowed		TRUE			TRUE			FALSE			FALSE	
22 Reference Time	1.7	30.0	0.0	3.3	29.4	0.0	NA	NA	31.5	NA	NA	0.0
23 Adjusted Reference Time	5.0	34.0	9.0	6.3	33.4	9.0	NA	NA	34.5	NA	NA	8.0
Permitted Option												
24 Proportion Lefts	1	0.00		1	0.00		1	0.99		1	0.00	
25 Volume Left Lane	26	445		47.5	441		0	457		10	32	
26 Proportion Lefts Left	1	0.00		1	0.00		1	0.99		1	0.00	
27 Left turn Equivalents	15.0			15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	0.07		1.07	1.00	
29 Permitted Sat Flow	120.3	1784.7		116.8	1801.4		0.0	242.1		1925.3	1650.6	
30 Reference Time A	25.9	30.0		48.8	29.4		0.0	226.5		0.7	3.4	
31 Adjusted Saturation B		3569.4			3602.8			0.0			1650.6	
32 Reference Time B		NA			NA			23.2			3.4	
33 Reference Time Lefts	NA			NA			23.1			8.7		
34 Reference Time		30.0			48.8			23.2			3.4	
35 Adjusted Reference Time		34.0			52.8			26.2			8.0	
Split Timing												
36 Ref Time Combined		30.0			29.4		4= 1	15.2			3.4	
37 Ref Time By Movement	1.7	30.0		3.3	29.4		15.1	0.3		0.7	3.4	
38 Reference Time	04.0	30.0		00.1	29.4		00.4	15.2			3.4	
39 Adjusted Reference Time	34.0	34.0	N1	33.4	33.4		20.4	20.4		8.0	8.0	
Summary		West	North									
40 Protected Option		0.3	N									
41 Permitted Option		2.8	26									
42 Split Option		7.4	28									
43 Minimum	40).3	26). <u>Z</u>								
44 Combined	E55	66		000								
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	9.0	9.0	34.5	8.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time48 Oncoming Left Direction	20.4	8.0	33.4 SBL	34.0								
	WBL	EBL		NBL 20.4								
49 Oncoming Left Adj Ref Time50 Combined	6.3	5.0	8.0	20.4								
		22.0	75.9	62.4								
51 Intersection Capacity Utiliz	ation	63.2%								Davisles	2002.0	
52 Level Of Service		В								Revision	∠∪∪3.U	

Intersection Location: Pleasant Valley / US 101 SB Ramps
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

							_		_			
1 Movement	J	\rightarrow	7	₽	←	L	7	T		4	↓	4
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	1	1	1	1	1	0	0	3	0	1	2	1
3 Shared LT Lane (y/n)	✓ Yes			Yes			Yes			Yes		
4 Volume	844	4	194	– 3	0	8	0	1208	5	17	917	789
5 Pedestrians			10			10			10			10
6 Ped Button (y/n)		✓ Yes			Yes			✓ Yes			✓ Yes	
7 Pedestrian Timing Required		14			0			14			14	
8 Free Right (y/n)			Yes			☐ Yes			☐ Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	3		3	3	3	3	0	3.5	3.5	3	3.5	3.5
11 Minimum Green	4	4	4	4	4	4	0	10	10	4	10	10
12 Reference Cycle Length	120											
13 Volume Combined	0.0	848.0	194.0	3.0	8.0	0.0	0.0	1213.0	0.0	17.0	917.0	789.0
14 Volume Separate Left	844.0	4.0		3.0	8.0		0.0	1213.0		17.0	917.0	
15 Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	0.908	1.000	1.000	0.952	1.000
16 Turning Factor Adjust	0.950	0.950	0.850	0.950	0.850	0.850	0.950	0.999	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	0.0	3610.9	1615.0	1805.0	1615.0	0.0	0.0	5172.4	0.0	1805.0	3617.6	1615.0
18 Saturated Flow Separate	3610.0	1900.0		1805.0	1615.0		0.0	5172.4		1805.0	3617.6	
19 Pedestrian Interference Time		0.0	1.2		1.2	1.2		0.0	1.2		0.0	1.2
20 Pedestrian Frequency		28.3%			100.0%			28.3%			28.3%	
21 Protected Option Allowed		FALSE			FALSE			TRUE			TRUE	
22 Reference Time	NA	NA	14.4	NA	NA	0.0	0.0	28.1	0.0	1.1	30.4	58.6
23 Adjusted Reference Time	NA	NA	17.4	NA	NA	7.0	0.0	31.6	13.5	7.0	33.9	62.1
Permitted Option												
24 Proportion Lefts	1	1.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	0	848		3	8		0	404		17	459	
26 Proportion Lefts Left	1	1.00		1	0.00		1	0.00		1	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	0.07		0.07	1.00		0.07	1.00		1.07	1.00	
29 Permitted Sat Flow	0.0	241.5		120.3	1615.0		0.0	1724.1		1925.3	1808.8	
30 Reference Time A	0.0	421.5		3.0	1.8		0.0	28.1		1.1	30.4	
31 Adjusted Saturation B		0.0			1615.0			5172.4			3617.6	
32 Reference Time B		36.2			1.8			NA			NA	
33 Reference Time Lefts	36.1			8.2			NA			NA		
34 Reference Time		36.2			3.0			28.1			30.4	
35 Adjusted Reference Time		39.2			7.0			31.6			33.9	
Split Timing												
36 Ref Time Combined		28.2			1.8			28.1			30.4	
37 Ref Time By Movement	28.1	0.3		0.2	1.8		0.0	28.1		1.1	30.4	
38 Reference Time		28.2			1.8			28.1			30.4	
39 Adjusted Reference Time	31.2	31.2		7.0	7.0		31.6	31.6		33.9	33.9	
Summary	East	West	North	South								
40 Protected Option		IA		3.6								
41 Permitted Option		9.2		3.9								
42 Split Option		3.2		5.6								
43 Minimum		3.2		3.9								
44 Combined		72										
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	17.4	7.0	13.5	62.1								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	31.6	33.9	7.0	31.2								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	7.0	31.2	7.0	0.0								
50 Combined	56.1	72.1	27.5	93.3								
51 Intersection Capacity Utiliza		77.8%			l							
52 Level Of Service		77.570 D								Revision	2003 0	
2010. 0. 00. 1100			l								_000.0	

Intersection Location: Pleasant Valley / US 101 SB Ramps
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

1	Movement	Ĵ	EBT	EBR	WBL	◆ WBT	L WBR	NBL	↑	NBR	SBL	SBT	SBR
2	Lanes	1	1	1	1	1	0	0	3	0	1	2	3BK 1
	Shared LT Lane (y/n)	✓ Yes	'	1	Yes	•		Yes			Yes		
	Volume		4	106	11	0	11	0	1275	4	10	1001	523
	Pedestrians			10			10		0	10			10
	Ped Button (y/n)		✓ Yes			Yes			✓ Yes			✓ Yes	
	Pedestrian Timing Required		14			0			14			14	
	Free Right (y/n)			Yes			Yes			Yes			Yes
	Ideal Flow	1900	1900	<u></u> 1900	1900	1900	1900	1900	1900	 1900	1900	1900	 1900
10	Lost Time	3	3	3	3	3	3	0	3.5	3.5	3	3.5	3.5
11	Minimum Green	4	4	4	4	4	4	0	10	10	4	10	10
12	Reference Cycle Length	120											
13	Volume Combined	0.0	857.0	106.0	11.0	11.0	0.0	0.0	1279.0	0.0	10.0	1001.0	523.0
14	Volume Separate Left	853.0	4.0		11.0	11.0		0.0	1279.0		10.0	1001.0	
-	Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	0.908	1.000	1.000	0.952	1.000
	Turning Factor Adjust	0.950	0.950	0.850	0.950	0.850	0.850	0.950	1.000	0.850	0.950	1.000	0.850
	Saturated Flow Combined	0.0	3610.9	1615.0	1805.0	1615.0	0.0	0.0	5173.2	0.0	1805.0	3617.6	1615.0
	Saturated Flow Separate	3610.0	1900.0		1805.0	1615.0		0.0	5173.2		1805.0	3617.6	
	Pedestrian Interference Time		0.0	1.2		1.2	1.2		0.0	1.2		0.0	1.2
	Pedestrian Frequency		28.3%			100.0%			28.3%			28.3%	
	Protected Option Allowed		FALSE			FALSE			TRUE			TRUE	
	Reference Time	NA	NA	7.9	NA	NA	0.0	0.0	29.7	0.0	0.7	33.2	38.9
_	Adjusted Reference Time	NA	NA	10.9	NA	NA	7.0	0.0	33.2	13.5	7.0	36.7	42.4
	Permitted Option												
	Proportion Lefts	1			1	0.00		1	0.00		1	0.00	
	Volume Left Lane	0			11	11		0	426		10	501	
	Proportion Lefts Left	1	1.00		1	0.00		1	0.00		1	0.00	
	Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
	Left turn Factor	0.07	0.07		0.07	1.00		0.07	1.00		1.07	1.00	
_	Permitted Sat Flow	0.0	241.4		120.3	1615.0		0.0	1724.4		1925.3	1808.8	
	Reference Time A	0.0	425.9		11.0	2.1		0.0	29.7		0.7	33.2	
	Adjusted Saturation B Reference Time B		0.0			1615.0			5173.2			3617.6	
		20.4	36.5		0.7	2.1		NIA	NA		NIA	NA	
	Reference Time Lefts Reference Time	36.4	36.5		8.7	8.7		NA	29.7		NA	33.2	
	Adjusted Reference Time		39.5			11.7			33.2			36.7	
	Split Timing		38.3			11.7			33.2			30.7	
	Ref Time Combined		28.5			2.1			29.7			33.2	
	Ref Time By Movement	28.4	0.3		0.7	2.1		0.0	29.7		0.7	33.2	
	Reference Time	20.4	28.5		0.7	2.1		0.0	29.7		0.7	33.2	
	Adjusted Reference Time	31.5	31.5		7.0	7.0		33.2	33.2		36.7	36.7	
_	Summary		West	North		7.0		00.2	00.2		00.1	00.1	
	Protected Option	N		40									
	Permitted Option		9.5	36									
	Split Option		3.5	69									
43	Minimum		3.5	36									
	Combined		75	.2									
	Right Turns	EBR	WBR	NBR	SBR								
45	Adjusted Reference Time	10.9	7.0	13.5	42.4								
	Cross Through Direction	NBT	SBT	WBT	EBT								
	Cross Through Adj Ref Time	33.2	36.7	7.0	31.5								
48	Oncoming Left Direction	WBL	EBL	SBL	NBL								
	Oncoming Left Adj Ref Time	7.0	31.5	7.0	0.0								
	Combined	51.0	75.2	27.5	73.8								
	Intersection Capacity Utilizat	tion	62.7%										
	Level Of Service		В								Revision	2003.0	

Intersection Location: Pleasant Valley / US 101 NB Off Ra
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

								_		_			
1	Movement	J		7	—	+	L		T		5	1	4
·		EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2	Lanes	0	0	0	1	0	2	0	3	1	0	3	0
3	Shared LT Lane (y/n)	Yes			Yes			Yes			Yes		
	Volume	0	0	0	294	0	420	0	1399	130	0	1417	0
5	Pedestrians			0			0			0			0
6	Ped Button (y/n)		Yes			Yes			Yes			Yes	
7	Pedestrian Timing Required		0			0			0			0	
8	Free Right (y/n)			Yes			Yes			✓ Yes			Yes
9	Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10	Lost Time	0	0	0	3	0	3	0	3.5	3.5	0	3.5	0
11	Minimum Green	0	0	0	4	0	4	0	10	10	0	10	0
12	Reference Cycle Length	120											
13	Volume Combined	0.0	0.0	0.0	294.0	0.0	420.0	0.0	1399.0	130.0	0.0	1417.0	0.0
14	Volume Separate Left	0.0	0.0		294.0	0.0		0.0	1399.0		0.0	1417.0	
15	Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	0.885	1.000	0.908	1.000	1.000	0.908	1.000
16	Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
17	Saturated Flow Combined	0.0	0.0	0.0	1805.0	0.0	2858.6	0.0	5175.6	1615.0	0.0	5175.6	0.0
18	Saturated Flow Separate	0.0	0.0		1805.0	0.0		0.0	5175.6		0.0	5175.6	
19	Pedestrian Interference Time		0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0
20	Pedestrian Frequency		0.0%			0.0%			0.0%			0.0%	
21	Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
22	Reference Time	0.0	0.0	0.0	19.5	0.0	17.6	0.0	32.4	9.7	0.0	32.9	0.0
23	Adjusted Reference Time	0.0	0.0	0.0	22.5	0.0	20.6	0.0	35.9	13.5	0.0	36.4	0.0
	Permitted Option												
24	Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
	Volume Left Lane	0	0		294	0		0	466		0	472	
26	Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
	Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0	
	Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		0.07	1.00	
29	Permitted Sat Flow	0.0	0.0		120.3	0.0		0.0	1725.2		0.0	1725.2	
30	Reference Time A	0.0	0.0		293.2	0.0		0.0	32.4		0.0	32.9	
31	Adjusted Saturation B		0.0			0.0			5175.6			5175.6	
32	Reference Time B		0.0			0.0			NA			NA	
33	Reference Time Lefts	0.0			27.5			NA			NA		
34	Reference Time		0.0			27.5			32.4			32.9	
35	Adjusted Reference Time		0.0			27.5			35.9			36.4	
	Split Timing												
	Ref Time Combined		0.0			0.0			32.4			32.9	
37	Ref Time By Movement	0.0	0.0		19.5	0.0		0.0	32.4		0.0	32.9	
38	Reference Time		0.0			19.5			32.4			32.9	
39	Adjusted Reference Time	0.0	0.0		19.5	19.5		35.9	35.9		36.4	36.4	
	Summary	East	West	North	South								
	Protected Option	22	2.5	36	.4								
	Permitted Option		'.5	36									
	Split Option).5	72									
	Minimum	19	9.5	36	.4								
44	Combined		55	.9									
	Right Turns	EBR	WBR	NBR	SBR								
	Adjusted Reference Time	0.0	20.6	13.5	0.0								
	Cross Through Direction	NBT	SBT	WBT	EBT								
	Cross Through Adj Ref Time	35.9	36.4	0.0	0.0								
	Oncoming Left Direction	WBL	EBL	SBL	NBL								
	Oncoming Left Adj Ref Time	19.5	0.0	0.0	0.0								
	Combined	55.5	57.0	13.5	0.0								
51	Intersection Capacity Utiliza	tion	47.5%										
	Level Of Service		A								Revision	2003.0	

Intersection Location: Pleasant Valley / US 101 NB Off Ra
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

1	Movement	1	EBT	EBR	WBL	◆ WBT	L	1	1	NIPP	SBL	₽ CPT	SBR
2	Lanes	EBL 0	0	EBK	VVBL	0	WBR 2	NBL 0	NBT 3	NBR 1	SBL 0	SBT 3	3BK 0
	Shared LT Lane (y/n)	Yes	U	U	Yes	U		Yes	3	I	Yes	3	
	Volume	0	0	0	436	0	594	0	1475	188		1099	0
	Pedestrians	U	U	0	430	U	0	U	1473	0	0	1033	0
	Ped Button (y/n)		Yes	U		Yes	Ü		Yes	U		Yes	
	Pedestrian Timing Required		0			0			0			0	
	Free Right (y/n)			Yes			Yes		-	✓ Yes			Yes
	Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
	Lost Time	0		0	3	0	3	0	3.5	3.5	0	3.5	0
	Minimum Green	0	0	0	4	0	4	0	10	10	0	10	0
	Reference Cycle Length	120											
	Volume Combined	0.0	0.0	0.0	436.0	0.0	594.0	0.0	1475.0	188.0	0.0	1099.0	0.0
	Volume Separate Left	0.0	0.0	0.0	436.0	0.0	00110	0.0	1475.0	10010	0.0	1099.0	0.0
	Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	0.885	1.000	0.908	1.000	1.000	0.908	1.000
	Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
	Saturated Flow Combined	0.0	0.0	0.0	1805.0	0.0	2858.6	0.0	5175.6	1615.0	0.0	5175.6	0.0
	Saturated Flow Separate	0.0	0.0		1805.0	0.0		0.0	5175.6		0.0	5175.6	
	Pedestrian Interference Time		0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0
20	Pedestrian Frequency		0.0%			0.0%			0.0%			0.0%	
	Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
	Reference Time	0.0	0.0	0.0	29.0	0.0	24.9	0.0	34.2	14.0	0.0	25.5	0.0
23	Adjusted Reference Time	0.0	0.0	0.0	32.0	0.0	27.9	0.0	37.7	17.5	0.0	29.0	0.0
	Permitted Option												
24	Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
25	Volume Left Lane	0	0		436	0		0	492		0	366	
26	Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
27	Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0	
28	Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		0.07	1.00	
29	Permitted Sat Flow	0.0	0.0		120.3	0.0		0.0	1725.2		0.0	1725.2	
	Reference Time A	0.0	0.0		434.8	0.0		0.0	34.2		0.0	25.5	
	Adjusted Saturation B		0.0			0.0			5175.6			5175.6	
32	Reference Time B		0.0			0.0			NA			NA	
	Reference Time Lefts	0.0			37.0			NA			NA		
	Reference Time		0.0			37.0			34.2			25.5	
	Adjusted Reference Time		0.0			37.0			37.7			29.0	
	Split Timing												
	Ref Time Combined		0.0			0.0			34.2			25.5	
	Ref Time By Movement	0.0	0.0		29.0	0.0		0.0	34.2		0.0	25.5	
	Reference Time		0.0			29.0			34.2			25.5	
	Adjusted Reference Time	0.0	0.0		29.0	29.0		37.7	37.7		29.0	29.0	
	Summary		West	North									
	Protected Option		2.0	37									
	Permitted Option		7.0	37									
42	Split Option		9.0	66									
	Minimum	29	9.0	37	./								
	Combined		66										
	Right Turns	EBR	WBR	NBR	SBR								
	Adjusted Reference Time	0.0	27.9	17.5	0.0								
	Cross Through Direction	NBT	SBT	WBT	EBT								
	Cross Through Adj Ref Time	37.7	29.0	0.0	0.0								
48	Oncoming Left Direction	WBL	EBL	SBL	NBL								
49	Oncoming Left Adj Ref Time	29.0	0.0	0.0	0.0								
	Combined	66.7	56.9	17.5	0.0								
	Intersection Capacity Utilizat	tion	55.6%								Davidete	2002.2	
52	Level Of Service		В								Revision	∠003.0	

EXISTING PLUS APPROVED/PENDING WORKSHEETS

Intersection Location: Pleasant Valley / Lewis Road
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

City: Ventura County

Alternative: Existing Plus Approved/Pending

Project: Pacific Rock

						A	_		_			
1 Movement	J	\rightarrow	7	—	\leftarrow	L	7	T		6	1	4
. In o volucing	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	2	2	1	2	2	1	1	2	1	1	2	1
3 Shared LT Lane (y/n)	Yes		-	Yes		-	Yes		-	Yes		
4 Volume	215	616	18	414	367	222	50	290	260	131	686	243
5 Pedestrians			10			10			10			10
6 Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes	-		✓ Yes	
7 Pedestrian Timing Required		20			17			– 28			_ 27	
8 Free Right (y/n)			Yes			Yes			Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
10 Lost Time	3	4	4	3	4	4	3	4	4	3	4	4
11 Minimum Green	4	10	10	4	10	10	4	10	10	4	10	10
12 Reference Cycle Length	120											
13 Volume Combined	215.0	616.0	18.0	414.0	367.0	222.0	50.0	290.0	260.0	131.0	686.0	243.0
14 Volume Separate Left	215.0	616.0	10.0	414.0	367.0	ZZZ.0	50.0	290.0	200.0	131.0	686.0	240.0
15 Lane Utilization Factor	0.971	0.952	1.000	0.971	0.952	1.000	1.000	0.952	1.000	1.000	0.952	1.000
16 Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	3505.3	3617.6	1615.0	3505.3	3617.6	1615.0	1805.0	3617.6	1615.0	1805.0	3617.6	1615.0
18 Saturated Flow Separate	3505.3	3617.6	1010.0	3505.3	3617.6	1010.0	1805.0	3617.6	1010.0	1805.0	3617.6	1010.0
19 Pedestrian Interference Time	3303.3	0.0	1.2	3303.3	0.0	1.2	1000.0	0.0	1.2	1000.0	0.0	1.2
20 Pedestrian Frequency		28.3%	1.2		28.3%	1.2		28.3%	1.2		28.3%	1.4
21 Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
21 Protected Option Allowed 22 Reference Time	7.4		1.0	14.0		16.5	2.2		10.2	8.7	22.8	10 1
23 Adjusted Reference Time	10.4	20.4	1.3 14.0	14.2 17.2	12.2 17.5	20.5	3.3 7.0	9.6 19.1	19.3 23.3	11.7	28.0	18.1 22.1
	10.4	24.4	14.0	17.2	17.5	20.5	7.0	19.1	23.3	11.7	20.0	22.1
Permitted Option	1	0.00		4	0.00		4	0.00		4	0.00	
24 Proportion Lefts	1 107.5	0.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	107.5	308		207	184		50	145		131	343	
26 Proportion Lefts Left	1 1 7	0.00		1 1 1 1	0.00		1 1 2	0.00		1	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		1.07	1.00	
29 Permitted Sat Flow	116.8	1808.8		116.8	1808.8		120.3	1808.8		1925.3	1808.8	
30 Reference Time A	110.4	20.4		212.6	12.2		49.9	9.6		8.7	22.8	
31 Adjusted Saturation B		3617.6			3617.6			3617.6			3617.6	
32 Reference Time B		NA			NA			NA			NA	
33 Reference Time Lefts	NA	440.4		NA	040.0		NA	40.0		NA	00.0	
34 Reference Time		110.4			212.6			49.9			22.8	
35 Adjusted Reference Time		114.4			216.6			53.9			28.0	
Split Timing												
36 Ref Time Combined		20.4			12.2			9.6			22.8	
37 Ref Time By Movement	7.4	20.4		14.2	12.2		3.3	9.6		8.7	22.8	
38 Reference Time		20.4			14.2			9.6			22.8	
39 Adjusted Reference Time	24.4	24.4		19.0	19.0		19.1	19.1		28.0	28.0	
Summary	East West		North South									
40 Protected Option	41.6		35.0									
41 Permitted Option		216.6		53.9								
42 Split Option		43.4		47.1								
43 Minimum	41.6		35.0									
44 Combined		76.										
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	14.0	20.5	23.3	22.1								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	19.1	28.0	17.5	24.4								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	17.2	10.4	11.7	7.0								
50 Combined	50.3	58.8	52.6	53.5								
51 Intersection Capacity Utiliza	tion	63.8%			1							
52 Level Of Service	В								Revision	2003.0		

Intersection Location: Pleasant Valley / Lewis Road
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

						A	4					
1 Movement		\rightarrow	7	.	1			T		U		
i illovelilette	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	2	2	1	2	2	1	1	2	1	1	2	1
3 Shared LT Lane (y/n)	Yes	_		Yes	_		Yes	_	•	Yes	_	·
4 Volume	237	417	17	338	803	269	89	559	456	121	426	287
5 Pedestrians	201	717	10		000	10	- 00	000	10	121	420	10
6 Ped Button (y/n)		✓ Yes	10		✓ Yes	10		✓ Yes			✓ Yes	10
7 Pedestrian Timing Required		20			17			28			27	
8 Free Right (y/n)		20	Yes		.,	Yes		20	Yes		21	Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	3	4	4	3	4	4	3	4	4	3	4	4
11 Minimum Green	4	10	10	4	10	10	4	10	10	4	10	10
12 Reference Cycle Length	120			·						<u> </u>	.0	
13 Volume Combined	237.0	417.0	17.0	338.0	803.0	269.0	89.0	559.0	456.0	121.0	426.0	287.0
14 Volume Separate Left	237.0	417.0	17.0	338.0	803.0	209.0	89.0	559.0	430.0	121.0	426.0	207.0
15 Lane Utilization Factor	0.971	0.952	1.000	0.971	0.952	1.000	1.000	0.952	1.000	1.000	0.952	1.000
16 Turning Factor Adjust	0.971	1.000	0.850	0.971	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	3505.3	3617.6		3505.3	3617.6	1615.0	1805.0	3617.6	1615.0	1805.0	3617.6	1615.0
17 Saturated Flow Complined 18 Saturated Flow Separate	3505.3	3617.6	1015.0	3505.3	3617.6	0.01	1805.0	3617.6	1015.0	1805.0	3617.6	1015.0
19 Pedestrian Interference Time	JOU5.3	0.0	1.2	JJU5.3	0.0	1.2	1005.0	0.0	1.2	1005.0	0.0	1.2
		28.3%	1.2		28.3%	1.2		28.3%	1.2		28.3%	1.2
20 Pedestrian Frequency					Z8.3%						Z8.3%	
21 Protected Option Allowed	0.4	TRUE	4.0	44.0		00.0		TRUE	00.0	0.0		04.0
22 Reference Time	8.1	13.8	1.3	11.6	26.6	20.0	5.9	18.5	33.9	8.0	14.1	21.3
23 Adjusted Reference Time	11.1	19.6	14.0	14.6	30.6	24.0	8.9	25.2	37.9	11.0	21.8	25.3
Permitted Option				<u> </u>								
24 Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	118.5	209		169	402		89	280		121	213	
26 Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
27 Left turn Equivalents	15.0			15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		1.07	1.00	
29 Permitted Sat Flow	116.8	1808.8		116.8	1808.8		120.3	1808.8		1925.3	1808.8	
30 Reference Time A	121.7	13.8		173.6	26.6		88.8	18.5		8.0	14.1	
31 Adjusted Saturation B		3617.6			3617.6			3617.6			3617.6	
32 Reference Time B		NA			NA			NA			NA	
33 Reference Time Lefts	NA			NA			NA			NA		
34 Reference Time		121.7			173.6			88.8			14.1	
35 Adjusted Reference Time		125.7			177.6			92.8			21.8	
Split Timing												
36 Ref Time Combined		13.8			26.6			18.5			14.1	
37 Ref Time By Movement	8.1	13.8		11.6	26.6		5.9	18.5		8.0	14.1	
38 Reference Time		13.8			26.6			18.5			14.1	
39 Adjusted Reference Time	19.6	19.6		30.6	30.6		25.2	25.2		21.8	21.8	
Summary		West	North									
40 Protected Option		1.7		5.3								
41 Permitted Option	1	7.6	92									
42 Split Option).2	47									
43 Minimum	41	1.7		5.3								
44 Combined			3.0									
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	14.0	24.0	37.9	25.3								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	25.2	21.8	30.6	19.6								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	14.6	11.1	11.0	8.9								
50 Combined	53.8	56.9	79.6	53.8								
51 Intersection Capacity Utiliza	tion	66.3%			1							
52 Level Of Service		С								Revision	2003.0	

Intersection Location: Pleasant Valley / Pancho
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

				_			_					
1 Movement		\rightarrow	7	•	1	L	7	T		Ĵ	I I	4
- Increment	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	1		0	2	2	0	1	1	1	1	1	0
3 Shared LT Lane (y/n)	Yes			Yes			✓ Yes			Yes		
4 Volume	11	933	157	306	692	10	87	2	84	26	2	48
5 Pedestrians			10		002	10	<u> </u>	_	10			10
6 Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes			Yes	
7 Pedestrian Timing Required		17			17			23				
8 Free Right (y/n)			Yes			Yes			Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	3		4	3	4	4	2	3	3	2	3	3
11 Minimum Green	2	5		2	5	5	3	5	5	3	5	5
12 Reference Cycle Length	120				Ū	-					Ū	
13 Volume Combined	11.0	1090.0	0.0	306.0	702.0	0.0	0.0	89.0	84.0	26.0	50.0	0.0
14 Volume Separate Left	11.0	1090.0	0.0	306.0	702.0	0.0	87.0	2.0	04.0	26.0	50.0	0.0
15 Lane Utilization Factor	1.000	0.952	1.000	0.971	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000
16 Turning Factor Adjust	0.950	0.932	0.850	0.950	0.932	0.850	0.950	0.951	0.850	0.950	0.856	0.850
17 Saturated Flow Combined	1805.0			3505.3	3609.9	0.0	0.930	3614.3	1615.0	1805.0	1626.4	0.0
18 Saturated Flow Separate	1805.0	3539.4	0.0	3505.3	3609.9	0.0	3610.0	1900.0	1013.0	1805.0	1626.4	0.0
19 Pedestrian Interference Time	1003.0	0.2	1.2	3303.3	0.0	1.2	3010.0	0.0	1.2	1003.0	1.2	1.2
20 Pedestrian Frequency		28.3%	1.2		28.3%	1.2		28.3%	1.2		100.0%	1.2
21 Protected Option Allowed		Z8.3%			Z8.3%			FALSE			FALSE	
22 Reference Time	0.7		0.0	40.5		0.0	NIA		0.0	NIA		0.0
	0.7	37.1	0.0	10.5	23.4	9.0	NA NA	NA	6.2 9.2	NA	NA	0.0
23 Adjusted Reference Time	5.0	41.1	9.0	13.5	27.4	9.0	NA	NA	9.2	NA	NA	8.0
Permitted Option		0.00			0.00			0.00			0.00	
24 Proportion Lefts	1	0.00		1	0.00		1	0.98		1	0.00	
25 Volume Left Lane	11	545		153	351		0	89		26	50	
26 Proportion Lefts Left	1			1	0.00		1	0.98		1	0.00	
27 Left turn Equivalents	15.0			15.0	15.0		15.0	15.1		0.9	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	0.07		1.07	1.00	
29 Permitted Sat Flow	120.3	1769.7		116.8	1804.9		0.0	244.5		1925.3	1626.4	
30 Reference Time A	11.0	37.1		157.1	23.4		0.0	43.7		1.7	4.9	
31 Adjusted Saturation B		3539.4			3609.9			0.0			1626.4	
32 Reference Time B		NA			NA			11.0			4.9	
33 Reference Time Lefts	NA			NA			10.9			9.7		
34 Reference Time		37.1			157.1			11.0			4.9	
35 Adjusted Reference Time		41.1			161.1			17.4			8.0	
Split Timing												
36 Ref Time Combined		37.1			23.4			3.0			4.9	
37 Ref Time By Movement	0.7	37.1		10.5	23.4		2.9	0.1		1.7	4.9	
38 Reference Time		37.1			23.4			3.0			4.9	
39 Adjusted Reference Time	41.1	41.1		27.4	27.4		13.1	13.1		8.0	8.0	
Summary		West	North									
40 Protected Option		1.6	N									
41 Permitted Option		1.1		'.4								
42 Split Option		3.5	21									
43 Minimum	54	1.6	17	'.4								
44 Combined		72	2.0									
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	9.0	9.0	9.2	8.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	13.1	8.0	27.4	41.1								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	13.5	5.0	8.0	13.1								
50 Combined	35.6	22.0	44.6	62.2								
51 Intersection Capacity Utiliza		60.0%	-									
52 Level Of Service		В								Revision	2003 0	

Intersection Location: Pleasant Valley / Pancho
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

							_					
1 Movement	J		7	F	←	L		T		4	. ↓	4
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	1	2	0	2	2	0	_ 1	1	1	1	1	0
3 Shared LT Lane (y/n)	Yes			Yes			✓ Yes			Yes		
4 Volume	26	831	83	74	885	24	447	4	391	10	4	28
5 Pedestrians			10			10			10			10
6 Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes			∐ Yes	
7 Pedestrian Timing Required		17			17			23			0	
8 Free Right (y/n)	4000	4000	Yes 1900	1000	1000	Yes	4000	1000	Yes	4000	4000	Yes
9 Ideal Flow 10 Lost Time	1900	1900		1900	1900	1900	1900	1900	1900	1900	1900	1900
11 Minimum Green	3 2	4 5	4 5	2	4 5	4 5	3	3 5	<u>3</u>		3 5	3 5
12 Reference Cycle Length	120	3	3		5	3	3	3	5	3	5	3
, ,		0440	0.0	74.0	000.0	0.0	0.0	454.0	004.0	40.0	00.0	0.0
13 Volume Combined	26.0	914.0	0.0	74.0	909.0	0.0	0.0	451.0	391.0	10.0	32.0	0.0
14 Volume Separate Left	26.0	914.0	4 000	74.0	909.0	4.000	447.0	4.0	4.000	10.0	32.0	4.000
15 Lane Utilization Factor	1.000	0.952	1.000	0.971	0.952	1.000	1.000	1.000	1.000	1.000 0.950	1.000	1.000
16 Turning Factor Adjust 17 Saturated Flow Combined	0.950	0.986	0.850	0.950 3505.3	0.996 3603.3	0.850	0.950	0.950 3611.7	0.850		0.869 1650.6	0.850
	1805.0	3568.3 3568.3	0.0	3505.3	3603.3	0.0	3610.0	1900.0	1615.0	1805.0 1805.0	1650.6	0.0
18 Saturated Flow Separate 19 Pedestrian Interference Time	1805.0	0.1	1.2	3005.3	0.0	1.2	3010.0	0.0	1.2	1005.0	1.1	1.2
20 Pedestrian Frequency		28.3%	1.2		28.3%	1.2		28.3%	1.2		100.0%	1.2
		TRUE			TRUE			FALSE			FALSE	
21 Protected Option Allowed 22 Reference Time	1.7	30.8	0.0	2.5	30.3	0.0	NA	NA	29.1	NA	NA	0.0
23 Adjusted Reference Time	1.7 5.0	34.8	9.0	2.5 5.5	34.3	9.0	NA NA	NA NA	32.1	NA NA	NA NA	0.0 8.0
	5.0	34.0	9.0	5.5	34.3	9.0	INA	INA	32.1	INA	INA	0.0
Permitted Option 24 Proportion Lefts	1	0.00		1	0.00		1	0.99		1	0.00	
25 Volume Left Lane	26	457		37	455		0	451		10	32	
26 Proportion Lefts Left	1	0.00		1	0.00		1	0.99		10	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	0.07		1.07	1.00	
29 Permitted Sat Flow	120.3	1784.2		116.8	1801.6		0.0	242.1		1925.3	1650.6	
30 Reference Time A	25.9	30.8		38.0	30.3		0.0	223.5		0.7	3.4	
31 Adjusted Saturation B	20.0	3568.3		00.0	3603.3		0.0	0.0		0.7	1650.6	
32 Reference Time B		NA			NA			23.0			3.4	
33 Reference Time Lefts	NA			NA			22.9	20.0		8.7	011	
34 Reference Time		30.8		101	38.0			23.0		0	3.4	
35 Adjusted Reference Time		34.8			42.0			26.0			8.0	
Split Timing												
36 Ref Time Combined		30.8			30.3			15.0			3.4	
37 Ref Time By Movement	1.7	30.8		2.5	30.3		14.9	0.3		0.7	3.4	
38 Reference Time		30.8			30.3			15.0			3.4	
39 Adjusted Reference Time	34.8	34.8		34.3	34.3		20.3	20.3		8.0	8.0	
Summary	East	West	North	South								
40 Protected Option	40).4	N	Α								
41 Permitted Option	42	2.0	26	6.0								
42 Split Option		9.2		3.3								
43 Minimum	40).4	26	5.0								
44 Combined		66	5.4									
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	9.0	9.0	32.1	8.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	20.3	8.0	34.3	34.8								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	5.5	5.0	8.0	20.3								
50 Combined	34.8	22.0	74.4	63.1								
51 Intersection Capacity Utiliza	tion	62.0%										
52 Level Of Service		В								Revision	2003.0	

Intersection Location: Pleasant Valley / US 101 SB Ramps
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

							_					
1 Movement	J	ightharpoons	7	F	+	L	7	T		4	↓	4
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	1	1	1	1	1	0	0	3	0	1	2	1
3 Shared LT Lane (y/n)	✓ Yes			Yes			Yes Yes			Yes		
4 Volume	935	4	179	3	0	8	0	1193	5	17	896	889
5 Pedestrians			10			10			10			10
6 Ped Button (y/n)		✓ Yes			Yes			✓ Yes			✓ Yes	
7 Pedestrian Timing Required		14			0	_		14			14	_
8 Free Right (y/n)			☐ Yes			Yes			Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	3	3	3	3	3	3	0	3.5	3.5	3	3.5	3.5
11 Minimum Green	4	4	4	4	4	4	0	10	10	4	10	10
12 Reference Cycle Length	120											
13 Volume Combined	0.0	939.0	179.0	3.0	8.0	0.0	0.0	1198.0	0.0	17.0	896.0	889.0
14 Volume Separate Left	935.0	4.0		3.0	8.0		0.0	1198.0		17.0	896.0	
15 Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	0.908	1.000	1.000	0.952	1.000
16 Turning Factor Adjust	0.950	0.950	0.850	0.950	0.850	0.850	0.950	0.999	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	0.0	3610.8	1615.0	1805.0	1615.0	0.0	0.0	5172.4	0.0	1805.0	3617.6	1615.0
18 Saturated Flow Separate	3610.0	1900.0		1805.0	1615.0		0.0	5172.4		1805.0	3617.6	
19 Pedestrian Interference Time		0.0	1.2		1.2	1.2		0.0	1.2		0.0	1.2
20 Pedestrian Frequency		28.3%			100.0%			28.3%			28.3%	
21 Protected Option Allowed		FALSE			FALSE			TRUE			TRUE	
22 Reference Time	NA	NA	13.3	NA	NA	0.0	0.0	27.8	0.0	1.1	29.7	66.1
23 Adjusted Reference Time	NA	NA	16.3	NA	NA	7.0	0.0	31.3	13.5	7.0	33.2	69.6
Permitted Option												
24 Proportion Lefts	1	1.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	0	939		3	8		0	399		17	448	
26 Proportion Lefts Left	1	1.00		1	0.00		1	0.00		1	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	0.07		0.07	1.00		0.07	1.00		1.07	1.00	
29 Permitted Sat Flow	0.0	241.4		120.3	1615.0		0.0	1724.1		1925.3	1808.8	
30 Reference Time A	0.0	466.8		3.0	1.8		0.0	27.8		1.1	29.7	
31 Adjusted Saturation B		0.0			1615.0			5172.4			3617.6	
32 Reference Time B	00.4	39.2			1.8			NA			NA	
33 Reference Time Lefts	39.1	00.0		8.2	0.0		NA	07.0		NA	00.7	
34 Reference Time		39.2			3.0			27.8			29.7	
35 Adjusted Reference Time		42.2			7.0			31.3			33.2	
Split Timing		0.1.0			4.0							
36 Ref Time Combined	04.4	31.2			1.8		0.0	27.8		1.1	29.7	
37 Ref Time By Movement	31.1	0.3		0.2	1.8		0.0	27.8		1.1	29.7	
38 Reference Time 39 Adjusted Reference Time	34.2	31.2 34.2		7.0	1.8 7.0		24.2	27.8 31.3		22.0	29.7 33.2	
,			NI41-	7.0	7.0		31.3	31.3		33.2	33.2	
Summary		West	North									
40 Protected Option	N		38									
41 Permitted Option		2.2		3.2								
42 Split Option 43 Minimum		.2		.5 3.2								
	4).Z								
44 Combined		74		000								
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	16.3	7.0	13.5	69.6								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	31.3	33.2	7.0	34.2								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time50 Combined	7.0	34.2	7.0	0.0								
	54.6	74.4	27.5	103.8								
51 Intersection Capacity Utiliza	tion	86.5%								Davists	2002.0	
52 Level Of Service		Е								Revision	∠003.0	

Intersection Location: Pleasant Valley / US 101 SB Ramps
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

	1	-	J		4	1	1	1	A	L		
1 Movement	5 DI		TDD	₩D!	WDT	WDD	NDI	■ NDT	NDD	ODI	◆	000
21.5755	EBL 1	EBT	EBR 1	WBL	WBT 1	WBR 0	NBL	NBT	NBR 0	SBL	SBT	SBR
2 Lanes	✓ Yes	1	1	1	1	U	0 ☐ Yes	3	U	1	2	1
3 Shared LT Lane (y/n)	928	4	400	Yes	0	4.4	0 res	4070	4	Yes 10	4000	600
4 Volume 5 Pedestrians	928	4	106 10	11	U	11 10	U	1272	<u>4</u> 10	10	1020	623 10
		[/ Vaa	10		□ Vaa	10		[/ Vee	10		/ Vaa	10
6 Ped Button (y/n) 7 Pedestrian Timing Required		✓ Yes			Yes 0			✓ Yes			✓ Yes	
		14	Yes		U	Yes		14	Yes		14	Yes
8 Free Right (y/n) 9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time												
11 Minimum Green	3	3	3	3	3	3	0	3.5 10	3.5 10		3.5 10	3.5 10
_		4	4	4	4	4	U	10	10	4	10	10
12 Reference Cycle Length	120											
13 Volume Combined	0.0	932.0	106.0	11.0	11.0	0.0	0.0	1276.0	0.0	10.0	1020.0	623.0
14 Volume Separate Left	928.0	4.0		11.0	11.0		0.0	1276.0		10.0	1020.0	
15 Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	0.908	1.000	1.000	0.952	1.000
16 Turning Factor Adjust	0.950	0.950	0.850	0.950	0.850	0.850	0.950	1.000	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	0.0	3610.8	1615.0	1805.0	1615.0	0.0	0.0	5173.2	0.0	1805.0	3617.6	1615.0
18 Saturated Flow Separate	3610.0	1900.0		1805.0	1615.0		0.0	5173.2		1805.0	3617.6	
19 Pedestrian Interference Time		0.0	1.2		1.2	1.2		0.0	1.2		0.0	1.2
20 Pedestrian Frequency		28.3%			100.0%			28.3%			28.3%	
21 Protected Option Allowed		FALSE			FALSE			TRUE			TRUE	
22 Reference Time	NA	NA	7.9	NA	NA	0.0	0.0	29.6	0.0	0.7	33.8	46.3
23 Adjusted Reference Time	NA	NA	10.9	NA	NA	7.0	0.0	33.1	13.5	7.0	37.3	49.8
Permitted Option												
24 Proportion Lefts	1	1.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	0	932		11	11		0	425		10	510	
26 Proportion Lefts Left	1	1.00		1	0.00		1	0.00		1	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	0.07		0.07	1.00		0.07	1.00		1.07	1.00	
29 Permitted Sat Flow	0.0	241.4		120.3	1615.0		0.0	1724.4		1925.3	1808.8	
30 Reference Time A	0.0	463.3		11.0	2.1		0.0	29.6		0.7	33.8	
31 Adjusted Saturation B		0.0			1615.0			5173.2			3617.6	
32 Reference Time B		39.0			2.1			NA			NA	
33 Reference Time Lefts	38.8			8.7			NA			NA		
34 Reference Time		39.0			8.7			29.6			33.8	
35 Adjusted Reference Time		42.0			11.7			33.1			37.3	
Split Timing												
36 Ref Time Combined		31.0			2.1			29.6			33.8	
37 Ref Time By Movement	30.8	0.3		0.7	2.1		0.0	29.6		0.7	33.8	
38 Reference Time		31.0			2.1			29.6			33.8	
39 Adjusted Reference Time	34.0	34.0		7.0	7.0		33.1	33.1		37.3	37.3	
Summary		West	North				30.1	30.1		30	50	
40 Protected Option		A	40									
41 Permitted Option		2.0		'.3								
42 Split Option		1.0		0.4								
43 Minimum		1.0		'.3								
44 Combined	71	78		.0								
	EPD	WBR		SBR								
Right Turns	EBR		NBR									
45 Adjusted Reference Time	10.9	7.0	13.5	49.8								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	33.1	37.3	7.0	34.0								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	7.0	34.0	7.0	0.0								
50 Combined	51.0	78.3	27.5	83.8								
51 Intersection Capacity Utiliza	tion	69.8%										
52 Level Of Service		С								Revision	2003.0	

Intersection Location: Pleasant Valley / US 101 NB Off Ra
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

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		1	_			4	1		1				
1	Movement	EBL	EBT	EBR	WBL	WBT	WBR	■ NBL	■ NBT	■ NBR	SBL	SBT	SBR
2	Lanes	0	0	EBK 0	VVDL 1	0	2	NBL 0	3	NDK	SBL 0	3	36K 0
	Shared LT Lane (y/n)	Yes	U	0	Yes	U		Yes		1	Yes	J	
	Volume	0	0	0	244	0	513	0	1525	122	0	1546	0
	Pedestrians			0			0			0			0
_	Ped Button (y/n)		Yes			Yes			Yes			Yes	
	Pedestrian Timing Required		0			0			0			0	
8	Free Right (y/n)			Yes			Yes			✓ Yes			Yes
	Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
	Lost Time	0	-	0	3	0	3	0	3.5	3.5	0		0
	Minimum Green	0	0	0	4	0	4	0	10	10	0	10	0
	Reference Cycle Length	120											
	Volume Combined	0.0	0.0	0.0	244.0	0.0	513.0	0.0	1525.0	122.0	0.0	1546.0	0.0
	Volume Separate Left	0.0	0.0		244.0	0.0		0.0	1525.0		0.0	1546.0	
_	Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	0.885	1.000	0.908	1.000	1.000	0.908	1.000
	Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
	Saturated Flow Combined	0.0	0.0	0.0	1805.0	0.0	2858.6	0.0	5175.6	1615.0	0.0	5175.6	0.0
	Saturated Flow Separate	0.0		0.0	1805.0		0.0	0.0	5175.6	0.0	0.0	5175.6	0.0
	Pedestrian Interference Time		0.0%	0.0		0.0%	0.0		0.0%	0.0		0.0%	0.0
	Pedestrian Frequency Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
	Reference Time	0.0	0.0	0.0	16.2	0.0	21.5	0.0	35.4	9.1	0.0	35.8	0.0
	Adjusted Reference Time	0.0	0.0	0.0	19.2	0.0	24.5	0.0	38.9	13.5	0.0	39.3	0.0
	Permitted Option	0.0	0.0	0.0	19.2	0.0	24.0	0.0	50.9	10.0	0.0	39.5	0.0
	Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
	Volume Left Lane	0	0.00		244	0.00		0	508		0	515	
	Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
_	Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0	
	Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		0.07	1.00	
29	Permitted Sat Flow	0.0	0.0		120.3	0.0		0.0	1725.2		0.0	1725.2	
30	Reference Time A	0.0	0.0		243.3	0.0		0.0	35.4		0.0	35.8	
31	Adjusted Saturation B		0.0			0.0			5175.6			5175.6	
32	Reference Time B		0.0			0.0			NA			NA	
	Reference Time Lefts	0.0			24.2			NA			NA		
_	Reference Time		0.0			24.2			35.4			35.8	
	Adjusted Reference Time		0.0			24.2			38.9			39.3	
	Split Timing												
	Ref Time Combined	0.0	0.0		40.0	0.0		0.0	35.4			35.8	
	Ref Time By Movement	0.0	0.0		16.2	0.0		0.0	35.4		0.0	35.8	
	Reference Time	0.0	0.0		16.2	16.2 16.2		38.9	35.4 38.9		39.3	35.8 39.3	
_	Adjusted Reference Time			North		10.2		ატ.9	აგ.გ		აყ.პ	39.3	
	Summary Protected Option		West 0.2	North 39									
	Permitted Option		1.2	39									
	Split Option		3.2	78									
	Minimum		5.2	39									
	Combined	<u> </u>	55 55		. •								
	Right Turns	EBR	WBR	NBR	SBR								
	Adjusted Reference Time	0.0	24.5	13.5	0.0								
	Cross Through Direction	NBT	SBT	WBT	EBT								
	Cross Through Adj Ref Time	38.9	39.3	0.0	0.0								
48	Oncoming Left Direction	WBL	EBL	SBL	NBL								
49	Oncoming Left Adj Ref Time	16.2	0.0	0.0	0.0								
	Combined	55.1	63.9	13.5	0.0								
	Intersection Capacity Utiliza	tion	53.2%			-							
52	Level Of Service		Α								Revision	2003.0	
				•									

Intersection Location: Pleasant Valley / US 101 NB Off Ra
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

2 Lanes 0 0 0 1 0 2 0 3 1 3 Shared LT Lane (y/n) Yes Yes Yes Yes Yes 4 Volume 0 0 0 4 0 677 0 1577 183 5 Pedestrians 0	SBL SBT 0 3 Yes 0 1235 ———————————————————————————————————	0 0 0 Yes
EBL EBT EBR WBL WBT WBR NBL NBT NBR S 2 Lanes 0 0 0 1 0 2 0 3 1 3 Shared LT Lane (y/n) Yes Yes Yes Yes Yes Yes Yes 0	0 3 Yes 0 1235 U Yes (1900 1900 1900 1900 1900 1900 1900 190	3 0 5 0 0 0
2 Lanes 0 0 0 1 0 2 0 3 1 3 Shared LT Lane (y/n) Yes Yes Yes Yes Yes □ 4 Volume 0 0 0 419 0 677 0 1577 183 5 Pedestrians 0 <th>0 3 Yes 0 1235 U Yes (1900 1900 1900 1900 1900 1900 1900 190</th> <th>3 0 5 0 0 0</th>	0 3 Yes 0 1235 U Yes (1900 1900 1900 1900 1900 1900 1900 190	3 0 5 0 0 0
3 Shared LT Lane (y/n) Yes Yes Yes ✓ 4 Volume 0 0 0 419 0 677 0 1577 183 0 0 0 5 Pedestrians 0 0 0 0 0 0 6 Ped Button (y/n) Yes Yes Yes Yes 7 Pedestrian Timing Required 0 0 0 0 0 0 0 8 Free Right (y/n) Yes Yes Yes Yes Yes Yes Yes 9 Ideal Flow 1900 1900 1900 1900 1900 1900 1900 1900	0 1235	0 Ves
4 Volume 0 0 419 0 677 0 1577 183 5 Pedestrians 0 0 0 0 0 6 Ped Button (y/n) Yes Yes Yes Yes 7 Pedestrian Timing Required 0 0 0 0 8 Free Right (y/n) Yes Yes Yes Yes 9 Ideal Flow 1900 19	Yes (0 Ves
5 Pedestrians 0 0 0 6 Ped Button (y/n) Yes Yes Yes 7 Pedestrian Timing Required 0 0 0 8 Free Right (y/n) Yes Yes Yes 9 Ideal Flow 1900	1900 1900	Yes
7 Pedestrian Timing Required 0 1900	1900 1900	Yes
8 Free Right (y/n) Yes Yes Yes 9 Ideal Flow 1900<	1900 1900	Yes
8 Free Right (y/n) Yes ✓ Yes 9 Ideal Flow 1900 1		
9 Ideal Flow 1900		4000
11 Minimum Green 0 0 0 4 0 4 0 10 10 12 Reference Cycle Length 120 13 Volume Combined 0.0 0.0 0.0 419.0 0.0 677.0 0.0 1577.0 183.0 14 Volume Separate Left 0.0 0.0 419.0 0.0 0.0 1577.0 15 Lane Utilization Factor 1.000 1.000 1.000 1.000 0.885 1.000 0.908 1.000	0 2 5	1900
12 Reference Cycle Length 120 13 Volume Combined 0.0 0.0 0.0 419.0 0.0 677.0 0.0 1577.0 183.0 14 Volume Separate Left 0.0 0.0 419.0 0.0 0.0 1577.0 15 Lane Utilization Factor 1.000 1.000 1.000 0.085 1.000 0.908 1.000	U ₁ 3.5	5 0
13 Volume Combined 0.0 0.0 0.0 419.0 0.0 677.0 0.0 1577.0 183.0 14 Volume Separate Left 0.0 0.0 419.0 0.0 0.0 1577.0 15 Lane Utilization Factor 1.000 1.000 1.000 0.085 1.000 0.908 1.000	0 10	0
14 Volume Separate Left 0.0 0.0 419.0 0.0 0.0 1577.0 15 Lane Utilization Factor 1.000 1.000 1.000 1.000 0.885 1.000 0.908 1.000		
14 Volume Separate Left 0.0 0.0 419.0 0.0 0.0 1577.0 15 Lane Utilization Factor 1.000 1.000 1.000 1.000 0.885 1.000 0.908 1.000	0.0 1235.0	0.0
15 Lane Utilization Factor 1.000 1.000 1.000 1.000 0.885 1.000 0.908 1.000	0.0 1235.0	
	1.000 0.908	
	0.950 1.000	
17 Saturated Flow Combined 0.0 0.0 1805.0 0.0 2858.6 0.0 5175.6 1615.0	0.0 5175.6	
18 Saturated Flow Separate 0.0 0.0 1805.0 0.0 0.0 5175.6	0.0 5175.6	
19 Pedestrian Interference Time 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	0.0	
20 Pedestrian Frequency 0.0% 0.0% 0.0%	0.0%	
21 Protected Option Allowed TRUE TRUE TRUE	TRUE	
22 Reference Time 0.0 0.0 0.0 27.9 0.0 28.4 0.0 36.6 13.6	0.0 28.6	
23 Adjusted Reference Time 0.0 0.0 0.0 30.9 0.0 31.4 0.0 40.1 17.1	0.0 32.1	
Permitted Option	0.0 02.	0.0
24 Proportion Lefts 1 0.00 1 0.00 1 0.00	1 0.00	1
25 Volume Left Lane 0 0 419 0 0 526	0 412	
26 Proportion Lefts Left 1 0.00 1 0.00 1 0.00	1 0.00	
27 Left turn Equivalents 15.0 15.0 15.0 15.0 15.0 15.0	15.0 15.0	
28 Left turn Factor 0.07 1.00 0.07 1.00 0.07 1.00	0.07 1.00	
29 Permitted Sat Flow 0.0 0.0 120.3 0.0 0.0 1725.2	0.0 1725.2	
30 Reference Time A 0.0 0.0 417.8 0.0 0.0 36.6	0.0 1723.2	
31 Adjusted Saturation B 0.0 0.0 5175.6	5175.6	
32 Reference Time B 0.0 0.0 NA	N/	
33 Reference Time Lefts 0.0 35.9 NA	NA	
34 Reference Time 0.0 35.9 36.6	28.6	
35 Adjusted Reference Time 0.0 35.9 40.1	32.1	
Split Timing	52.	
36 Ref Time Combined 0.0 0.0 36.6	28.6	
36 Ref Time Combined 0.0 0.0 27.9 0.0 36.6 36.6	0.0 28.6	
38 Reference Time 0.0 0.0 27.9 36.6 36.6	28.6	
39 Adjusted Reference Time 0.0 0.0 27.9 40.1 40.1	32.1 32.1	
	32.1 32.	
Summary East West North South		
40 Protected Option 30.9 40.1 41 Permitted Option 35.9 40.1		
43 Minimum 27.9 40.1		
44 Combined 67.9		
Right Turns EBR WBR NBR SBR		
45 Adjusted Reference Time 0.0 31.4 17.1 0.0		
46 Cross Through Direction NBT SBT WBT EBT		
47 Cross Through Adj Ref Time 40.1 32.1 0.0 0.0		
48 Oncoming Left Direction WBL EBL SBL NBL		
49 Oncoming Left Adj Ref Time 27.9 0.0 0.0 0.0		
50 Combined 67.9 63.6 17.1 0.0		
51 Intersection Capacity Utilization 56.6%		
52 Level Of Service B	evision 2003.0	

EXISTING PLUS APPROVED/PENDING PLUS PROJECT WORKSHEETS

Intersection Location: Pleasant Valley / Lewis Road
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

1 Movement	1	\rightarrow	7	•	+	t	1	1	A	J		J
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	■ NBT	NBR	SBL	SBT	SBR
2 Lanes	2	2	1	2	2	1	1	2	1	1	2	1
3 Shared LT Lane (y/n)	Yes			Yes			Yes			Yes		
4 Volume	215	625	18	418	371	226	50	290	264	135	686	243
5 Pedestrians			10			10			10			10
6 Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes			✓ Yes	
7 Pedestrian Timing Required					- 17						27	
8 Free Right (y/n)			Yes			Yes			Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	— 1900
10 Lost Time	3	4	4	3	4	4	3	4	4	3	4	4
11 Minimum Green	4	10	10	4	10	10	4	10	10	4	10	10
12 Reference Cycle Length	120											
13 Volume Combined	215.0	625.0	18.0	418.0	371.0	226.0	50.0	290.0	264.0	135.0	686.0	243.0
14 Volume Separate Left	215.0	625.0	10.0	418.0	371.0	220.0	50.0	290.0	204.0	135.0	686.0	240.0
15 Lane Utilization Factor	0.971	0.952	1.000	0.971	0.952	1.000	1.000	0.952	1.000	1.000	0.952	1.000
16 Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	3505.3	3617.6	1615.0	3505.3	3617.6	1615.0	1805.0	3617.6	1615.0	1805.0	3617.6	1615.0
18 Saturated Flow Separate	3505.3	3617.6	1010.0	3505.3	3617.6	1010.0	1805.0	3617.6	1010.0	1805.0	3617.6	1010.0
19 Pedestrian Interference Time	3303.3	0.0	1.2	3303.3	0.0	1.2	1005.0	0.0	1.2	1003.0	0.0	1.2
20 Pedestrian Frequency		28.3%	1.2		28.3%	1.2		28.3%	1.2		28.3%	1.2
21 Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
22 Reference Time	7.4		4.0	44.0		40.0	2.2		40.0	0 0		40.4
	7.4 10.4	20.7	1.3 14.0	14.3	12.3	16.8	3.3 7.0	9.6 19.1	19.6	9.0 12.0	22.8	18.1
23 Adjusted Reference Time	10.4	24.7	14.0	17.3	17.6	20.8	7.0	19.1	23.6	12.0	28.0	22.1
Permitted Option		0.00			0.00			0.00		1	0.00	
24 Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	107.5	313		209	186		50	145		135	343	
26 Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
27 Left turn Equivalents	15.0			15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		1.07	1.00	
29 Permitted Sat Flow	116.8	1808.8		116.8	1808.8		120.3	1808.8		1925.3	1808.8	
30 Reference Time A	110.4	20.7		214.6	12.3		49.9	9.6		9.0	22.8	
31 Adjusted Saturation B		3617.6			3617.6			3617.6			3617.6	
32 Reference Time B		NA			NA			NA			NA	
33 Reference Time Lefts	NA			NA			NA			NA		
34 Reference Time		110.4			214.6			49.9			22.8	
35 Adjusted Reference Time		114.4			218.6			53.9			28.0	
Split Timing												
36 Ref Time Combined		20.7			12.3			9.6			22.8	
37 Ref Time By Movement	7.4	20.7		14.3	12.3		3.3	9.6		9.0	22.8	
38 Reference Time		20.7			14.3			9.6			22.8	
39 Adjusted Reference Time	24.7	24.7		19.1	19.1		19.1	19.1		28.0	28.0	
Summary	East	West	North	South								
40 Protected Option	42	2.0	35	5.0								
41 Permitted Option	21	8.6	53	3.9								
42 Split Option	43	3.8	47									
43 Minimum	42	2.0	35	5.0								
44 Combined		77										
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	14.0	20.8	23.6	22.1								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	19.1	28.0	17.6	24.7								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	17.3	10.4	12.0	7.0								
50 Combined	50.4	59.1	53.2	53.8								
51 Intersection Capacity Utiliza		64.2%	55.E	55.0								
52 Level Of Service	uon	04.2 % C								Revision	2003 U	
OF FOAD OF OUR AICE										CONSIGN	2000.0	

Intersection Location: Pleasant Valley / Lewis Road
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

1	Movement	1	\rightarrow	7	•	+	t	1	1		L		J
'	ino venicine	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2	Lanes	2	2	1	2	2	1	1	2	1	1	2	1
3	Shared LT Lane (y/n)	Yes			Yes			Yes			Yes		
_	Volume	237	419	17	340	810	271	89	559	458	123	426	287
5	Pedestrians			10			10			10			10
6	Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes			✓ Yes	
7	Pedestrian Timing Required		20			_ 17						27	
	Free Right (y/n)			Yes			Yes			Yes			Yes
	Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10	Lost Time	3	4	4	3	4	4	3	4	4	3	4	4
11	Minimum Green	4	10	10	4	10	10	4	10	10	4	10	10
12	Reference Cycle Length	120											
_	Volume Combined	237.0	419.0	17.0	340.0	810.0	271.0	89.0	559.0	458.0	123.0	426.0	287.0
	Volume Separate Left	237.0	419.0		340.0	810.0		89.0	559.0		123.0	426.0	
	Lane Utilization Factor	0.971	0.952	1.000	0.971	0.952	1.000	1.000	0.952	1.000	1.000	0.952	1.000
	Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
	Saturated Flow Combined	3505.3	3617.6	1615.0	3505.3	3617.6	1615.0	1805.0	3617.6	1615.0	1805.0	3617.6	1615.0
	Saturated Flow Separate	3505.3	3617.6		3505.3	3617.6		1805.0	3617.6		1805.0	3617.6	
	Pedestrian Interference Time	1100.0	0.0	1.2		0.0	1.2	120.0	0.0	1.2	1230.0	0.0	1.2
	Pedestrian Frequency		28.3%			28.3%			28.3%			28.3%	
	Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
	Reference Time	8.1	13.9	1.3	11.6	26.9	20.1	5.9	18.5	34.0	8.2	14.1	21.3
	Adjusted Reference Time	11.1	19.6	14.0	14.6	30.9	24.1	8.9	25.2	38.0	11.2	21.8	25.3
20	Permitted Option	1111	10.0	14.0	14.0	00.0	27.1	0.0	20.2	00.0	11.2	21.0	20.0
24	Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
	Volume Left Lane	118.5	210		170	405		89	280		123	213	
	Proportion Lefts Left	110.3	0.00		170	0.00		1	0.00		123	0.00	
	Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
	Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		1.07	1.00	
	Permitted Sat Flow	116.8	1808.8		116.8	1808.8		120.3	1808.8		1925.3	1808.8	
_	Reference Time A	121.7	13.9		174.6	26.9		88.8	18.5		8.2	14.1	
	Adjusted Saturation B	121.7	3617.6		174.0	3617.6		00.0	3617.6		0.2	3617.6	
	Reference Time B		3017.0 NA			3017.0 NA			NA			NA	
_	Reference Time Lefts	NA	INA		NA	INA		NA	INA		NA	INA	
	Reference Time	IVA	121.7		INA	174.6		INA	88.8		IVA	14.1	
	Adjusted Reference Time		125.7			174.6			92.8			21.8	
33	Split Timing		123.7			170.0			92.0			21.0	
26	Ref Time Combined		13.9			26.9			18.5			14.1	
37	Ref Time By Movement	8.1	13.9		11.6	26.9		5.9	18.5		8.2	14.1	
	Reference Time	0.1	13.9		11.0	26.9		5.8	18.5		0.2	14.1	
	Adjusted Reference Time	19.6	19.6		30.9	30.9		25.2	25.2		21.8	21.8	
39				North		30.9		23.2	25.2		21.0	21.0	
40	Summary Protected Option		West										
	Protected Option		2.0	36									
	Permitted Option		8.6	92 47									
	Split Option Minimum).5	36									
		42	2.0		.4								
44	Combined	FFF	78		000								
45	Right Turns	EBR	WBR	NBR	SBR								
	Adjusted Reference Time	14.0	24.1	38.0	25.3								
	Cross Through Direction	NBT	SBT	WBT	EBT								
	Cross Through Adj Ref Time	25.2	21.8	30.9	19.6								
	Oncoming Left Direction	WBL	EBL	SBL	NBL								
	Oncoming Left Adj Ref Time	14.6	11.1	11.2	8.9								
	Combined	53.9	57.0	80.1	53.9								
	Intersection Capacity Utiliza	tion	66.7%										
52	Level Of Service		С								Revision	2003.0	

Intersection Location: Pleasant Valley / Pancho
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

							_					
1 Movement	J	\rightarrow	7	5	+	L	7	T		Ĵ	Ţ	4
. In o voin one	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	1	2	0	2	2	0	1	1	1	1	1	0
3 Shared LT Lane (y/n)	Yes	_		Yes	_	Ů	✓ Yes		•	Yes		
4 Volume	11	933	173	377	692	10	98	2	148	26	2	48
5 Pedestrians	- ' '	300	10	311	002	10	30	L	10	20		10
6 Ped Button (y/n)		✓ Yes	10		✓ Yes	10		✓ Yes	10		Yes	10
7 Pedestrian Timing Required		17			17			23			<u> </u>	
8 Free Right (y/n)		17	Yes		17	Yes		23	Yes		U	Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	3	1900	1900	3	4	4	2	3	3	1900	3	3
11 Minimum Green	2	5	5	2	5	5	3	5	<u>5</u>	3	5	5
	120	J	J		J	J	J	J	J	3	J	J
12 Reference Cycle Length		44000						400.0				
13 Volume Combined	11.0	1106.0	0.0	377.0	702.0	0.0	0.0	100.0	148.0	26.0	50.0	0.0
14 Volume Separate Left	11.0	1106.0	,	377.0	702.0		98.0	2.0	,	26.0	50.0	
15 Lane Utilization Factor	1.000	0.952	1.000	0.971	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000
16 Turning Factor Adjust	0.950	0.977	0.850	0.950	0.998	0.850	0.950	0.951	0.850	0.950	0.856	0.850
17 Saturated Flow Combined	1805.0	3532.7	0.0	3505.3	3609.9	0.0	0.0	3613.8	1615.0	1805.0	1626.4	0.0
18 Saturated Flow Separate	1805.0	3532.7		3505.3	3609.9		3610.0	1900.0		1805.0	1626.4	
19 Pedestrian Interference Time		0.2	1.2		0.0	1.2		0.0	1.2		1.2	1.2
20 Pedestrian Frequency		28.3%			28.3%			28.3%			100.0%	
21 Protected Option Allowed		TRUE			TRUE			FALSE			FALSE	
22 Reference Time	0.7	37.8	0.0	12.9	23.4	0.0	NA	NA	11.0	NA	NA	0.0
23 Adjusted Reference Time	5.0	41.8	9.0	15.9	27.4	9.0	NA	NA	14.0	NA	NA	8.0
Permitted Option												
24 Proportion Lefts	1	0.00		1	0.00		1	0.98		1	0.00	
25 Volume Left Lane	11	553		188.5	351		0	100		26	50	
26 Proportion Lefts Left	1	0.00		1	0.00		1	0.98		1	0.00	
27 Left turn Equivalents	15.0			15.0	15.0		15.0	15.1		0.9	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	0.07		1.07	1.00	
29 Permitted Sat Flow	120.3	1766.4		116.8	1804.9		0.0	244.1		1925.3	1626.4	
30 Reference Time A	11.0	37.8		193.6	23.4		0.0	49.2		1.7	4.9	
31 Adjusted Saturation B		3532.7			3609.9			0.0			1626.4	
32 Reference Time B		NA			NA			11.3			4.9	
33 Reference Time Lefts	NA			NA			11.3			9.7		
34 Reference Time		37.8		101	193.6		1110	11.3		0.1	4.9	
35 Adjusted Reference Time		41.8			197.6			17.6			8.0	
Split Timing					10110						0.0	
36 Ref Time Combined		37.8			23.4			3.3			4.9	
37 Ref Time By Movement	0.7	37.8		12.9	23.4		3.3	0.1		1.7	4.9	
38 Reference Time	0.7	37.8		12.3	23.4		0.0	3.3		1.7	4.9	
39 Adjusted Reference Time	41.8	41.8		27.4	27.4		13.1	13.1		8.0	8.0	
Summary		West	North		21.4		10.1	10.1		0.0	0.0	
40 Protected Option		7.7	North									
41 Permitted Option		7.6		'.6								
42 Split Option		7.6 9.1	21									
43 Minimum		7.7		'.6								
	3/			.υ								
44 Combined		75		000								
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	9.0	9.0	14.0	8.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	13.1	8.0	27.4	41.8								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	15.9	5.0	8.0	13.1								
50 Combined	38.0	22.0	49.4	62.9								
51 Intersection Capacity Utiliza	tion	62.8%										
52 Level Of Service		В								Revision	2003.0	

Intersection Location: Pleasant Valley / Pancho
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak City: Ventura County

Alternative: Existing Plus App/Pen Plus Project

Project: Pacific Rock

	•		7				4	1		Ι.		
1 Movement	5 DI	- FDT	TDD	₩DI	WDT	WDD	ND!	■	NDD	CDI	↓	CDD
2 Lanes	EBL 1	EBT 2	EBR 0	WBL 2	WBT 2	WBR 0	NBL 1	NBT 1	NBR 1	SBL 1	SBT 1	SBR 0
3 Shared LT Lane (y/n)	Yes		U	☐ Yes		U	✓ Yes	ı	1	☐ Yes	<u> </u>	0
4 Volume	26	831	89	106	885	24	458	4	430	10	4	28
5 Pedestrians	20	001	10	100	000	10	100		10	10		10
6 Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes			Yes	
7 Pedestrian Timing Required		17			17			23			_ 0	
8 Free Right (y/n)			Yes			Yes			Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	 1900	1900	1900	1900
10 Lost Time	3	4	4	3	4	4	2	3	3	2	3	3
11 Minimum Green	2	5	5	2	5	5	3	5	5	3	5	5
12 Reference Cycle Length	120											
13 Volume Combined	26.0	920.0	0.0	106.0	909.0	0.0	0.0	462.0	430.0	10.0	32.0	0.0
14 Volume Separate Left	26.0	920.0		106.0	909.0		458.0	4.0		10.0	32.0	
15 Lane Utilization Factor	1.000	0.952	1.000	0.971	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000
16 Turning Factor Adjust	0.950	0.985	0.850	0.950	0.996	0.850	0.950	0.950	0.850	0.950	0.869	0.850
17 Saturated Flow Combined	1805.0	3565.1	0.0	3505.3	3603.3	0.0	0.0	3611.6	1615.0	1805.0	1650.6	0.0
18 Saturated Flow Separate	1805.0	3565.1		3505.3	3603.3		3610.0	1900.0		1805.0	1650.6	
19 Pedestrian Interference Time		0.1	1.2		0.0	1.2		0.0	1.2		1.1	1.2
20 Pedestrian Frequency		28.3%			28.3%			28.3%			100.0%	
21 Protected Option Allowed		TRUE			TRUE			FALSE			FALSE	
22 Reference Time	1.7	31.1	0.0	3.6	30.3	0.0	NA	NA	32.0	NA	NA	0.0
23 Adjusted Reference Time	5.0	35.1	9.0	6.6	34.3	9.0	NA	NA	35.0	NA	NA	8.0
Permitted Option												
24 Proportion Lefts	1	0.00		1	0.00		1	0.99		1	0.00	
25 Volume Left Lane	26	460		53	455		0	462		10	32	
26 Proportion Lefts Left	1	0.00		1	0.00		1	0.99		1	0.00	
27 Left turn Equivalents	15.0			15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	0.07		1.07	1.00	
29 Permitted Sat Flow	120.3	1782.6		116.8	1801.6		0.0	242.1		1925.3	1650.6	
30 Reference Time A	25.9	31.1		54.4	30.3		0.0	229.0		0.7	3.4	
31 Adjusted Saturation B		3565.1			3603.3			0.0			1650.6	
32 Reference Time B		NA			NA			23.4			3.4	
33 Reference Time Lefts	NA			NA			23.2			8.7		
34 Reference Time		31.1			54.4			23.4			3.4	
35 Adjusted Reference Time		35.1			58.4			26.4			8.0	
Split Timing												
36 Ref Time Combined		31.1			30.3		1	15.4			3.4	
37 Ref Time By Movement	1.7	31.1		3.6	30.3		15.2	0.3		0.7	3.4	
38 Reference Time	05.1	31.1		04.0	30.3		00.5	15.4			3.4	
39 Adjusted Reference Time	35.1	35.1	N	34.3	34.3		20.5	20.5		8.0	8.0	
Summary		West	North									
40 Protected Option		1.7	N									
41 Permitted Option		3.4	26									
42 Split Option 43 Minimum		9.4	28 26									
	4	1.7		0.4								
44 Combined	EDD.	68		000								
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	9.0	9.0	35.0	8.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time48 Oncoming Left Direction	20.5	8.0	34.3 SBL	35.1								
	WBL	EBL		NBL 20.5								
49 Oncoming Left Adj Ref Time50 Combined	6.6 36.1	5.0	8.0									
		22.0	77.3	63.6								
51 Intersection Capacity Utiliz	ation	64.4%								Davisles	2002.0	
52 Level Of Service		С								Revision	∠∪∪3.U	

Intersection Location: Pleasant Valley / US 101 SB Ramps
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

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1 Movement	J	\rightarrow	7	•	1	T	7			Ĵ	1	4
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	1	1	1	1	1	0	0	3	0	1	2	1
3 Shared LT Lane (y/n)	✓ Yes			Yes			Yes			Yes		
4 Volume	935	4	196	3	0	8	0	1257	5	17	950	889
5 Pedestrians			10			10			10			10
6 Ped Button (y/n)		✓ Yes			Yes			✓ Yes			✓ Yes	
7 Pedestrian Timing Required		14			_ 0			- 14			14	
8 Free Right (y/n)			Yes			Yes			Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	3	3	3	3	3	3	0	3.5	3.5	3	3.5	3.5
11 Minimum Green	4	4	4	4	4	4	0	10	10	4	10	10
12 Reference Cycle Length	120											
13 Volume Combined	0.0	939.0	196.0	3.0	8.0	0.0	0.0	1262.0	0.0	17.0	950.0	889.0
14 Volume Separate Left	935.0	4.0	100.0	3.0	8.0	0.0	0.0	1262.0	0.0	17.0	950.0	000.0
15 Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	0.908	1.000	1.000	0.952	1.000
16 Turning Factor Adjust	0.950	0.950	0.850	0.950	0.850	0.850	0.950	0.999	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	0.0	3610.8		1805.0	1615.0	0.0	0.0	5172.5	0.0	1805.0	3617.6	1615.0
18 Saturated Flow Separate	3610.0	1900.0	12.0.0	1805.0	1615.0	3.3	0.0	5172.5	J.0	1805.0	3617.6	
19 Pedestrian Interference Time	3310.0	0.0	1.2		1.2	1.2	<u> </u>	0.0	1.2		0.0	1.2
20 Pedestrian Frequency		28.3%			100.0%			28.3%	···-		28.3%	
21 Protected Option Allowed		FALSE			FALSE			TRUE			TRUE	
22 Reference Time	NA	NA	14.6	NA	NA	0.0	0.0	29.3	0.0	1.1	31.5	66.1
23 Adjusted Reference Time	NA	NA	17.6	NA	NA	7.0	0.0	32.8	13.5	7.0	35.0	69.6
Permitted Option	1471	14/1	17.0	1471	1471	7.0	0.0	02.0	10.0	7.0	00.0	00.0
24 Proportion Lefts	1	1.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	0			3	8		0	421		17	475	
26 Proportion Lefts Left	1	1.00		1	0.00		1	0.00		1	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	0.07		0.07	1.00		0.07	1.00		1.07	1.00	
29 Permitted Sat Flow	0.07	241.4		120.3	1615.0		0.0	1724.2		1925.3	1808.8	
30 Reference Time A	0.0	466.8		3.0	1.8		0.0	29.3		1.1	31.5	
31 Adjusted Saturation B	0.0	0.0		0.0	1615.0		0.0	5172.5		1.1	3617.6	
32 Reference Time B		39.2			1.8			NA			NA	
33 Reference Time Lefts	39.1	00.2		8.2	1.0		NA	1471		NA	1471	
34 Reference Time	00.1	39.2		0.2	3.0		1071	29.3		147 (31.5	
35 Adjusted Reference Time		42.2			7.0			32.8			35.0	
Split Timing		72.2			7.0			32.0			33.0	
36 Ref Time Combined		31.2			1.8			29.3			31.5	
37 Ref Time By Movement	31.1	0.3		0.2	1.8		0.0	29.3		1.1	31.5	
38 Reference Time	01.1	31.2		0.2	1.8		0.0	29.3		1.1	31.5	
39 Adjusted Reference Time	34.2	34.2		7.0	7.0		32.8	32.8		35.0	35.0	
Summary		West	North		7.0		52.0	JZ.0		55.0	55.0	
40 Protected Option		A).8								
41 Permitted Option		2.2	35									
42 Split Option		1.2	67									
43 Minimum		1.2		5.0								
44 Combined			5.2									
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	17.6	7.0	13.5	69.6								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	32.8	35.0	7.0	34.2								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Direction 49 Oncoming Left Adj Ref Time												
50 Combined	7.0 57.3	34.2 76.2	7.0 27.5	0.0 103.8								
			21.5	103.0								
51 Intersection Capacity Utiliza	ition	86.5%								Dovisies	2002.0	
52 Level Of Service		Е								Revision	2003.0	

Intersection Location: Pleasant Valley / US 101 SB Ramps
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

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1 Movement	J	\rightarrow	7	•	1	T	7			Ĵ	1	4
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	1	1	1	1	1	0	0	3	0	1	2	1
3 Shared LT Lane (y/n)	✓ Yes			Yes			Yes			Yes		
4 Volume	928	4	114	11	0	11	0	1311	4	10	1045	623
5 Pedestrians			10			10			10			10
6 Ped Button (y/n)		✓ Yes			Yes			✓ Yes			✓ Yes	
7 Pedestrian Timing Required		14			_ 0			_ 14			14	
8 Free Right (y/n)			Yes			Yes			Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	 1900
10 Lost Time	3	3	3	3	3	3	0	3.5	3.5	3	3.5	3.5
11 Minimum Green	4	4	4	4	4	4	0	10	10	4	10	10
12 Reference Cycle Length	120											
13 Volume Combined	0.0	932.0	114.0	11.0	11.0	0.0	0.0	1315.0	0.0	10.0	1045.0	623.0
14 Volume Separate Left	928.0	4.0	111.0	11.0	11.0	0.0	0.0	1315.0	0.0	10.0	1045.0	020.0
15 Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	0.908	1.000	1.000	0.952	1.000
16 Turning Factor Adjust	0.950	0.950	0.850	0.950	0.850	0.850	0.950	1.000	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	0.00	3610.8	1615.0	1805.0	1615.0	0.0	0.0	5173.2	0.00	1805.0	3617.6	1615.0
18 Saturated Flow Separate	3610.0	1900.0	1010.0	1805.0	1615.0	0.0	0.0	5173.2	0.0	1805.0	3617.6	.010.0
19 Pedestrian Interference Time	3010.0	0.0	1.2	1500.0	1.2	1.2	5.0	0.0	1.2	.555.0	0.0	1.2
20 Pedestrian Frequency		28.3%	1.2		100.0%	1.2		28.3%	1.2		28.3%	1.2
21 Protected Option Allowed		FALSE			FALSE			TRUE			TRUE	
22 Reference Time	NA	NA	8.5	NA	NA	0.0	0.0	30.5	0.0	0.7	34.7	46.3
23 Adjusted Reference Time	NA NA	NA	11.5	NA NA	NA NA	7.0	0.0	34.0	13.5	7.0	38.2	49.8
Permitted Option	INA	IVA	11.5	INA	INA	7.0	0.0	34.0	10.0	7.0	30.2	49.0
24 Proportion Lefts	4	1.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	1 0			<u></u>	11		0	438		10	523	
26 Proportion Lefts Left	1	1.00		1	0.00		1	0.00		10	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	0.07		0.07	1.00		0.07	1.00		1.07	1.00	
29 Permitted Sat Flow	0.07	241.4		120.3	1615.0		0.07	1724.4		1925.3	1808.8	
30 Reference Time A	0.0	463.3		11.0	2.1		0.0	30.5		0.7	34.7	
31 Adjusted Saturation B	0.0	0.0		11.0	1615.0		0.0	5173.2		0.7	3617.6	
32 Reference Time B		39.0			2.1			0173.2 NA			3017.0 NA	
33 Reference Time Lefts	38.8	39.0		8.7	2.1		NA	INA		NA	INA	
34 Reference Time	30.0	39.0		0.7	8.7		INA	30.5		IVA	34.7	
35 Adjusted Reference Time		42.0			11.7			34.0			38.2	
-		42.0			11.7			34.0			30.2	
Split Timing 36 Ref Time Combined		24.0			0.4			20.5			24.7	
37 Ref Time Combined 37 Ref Time By Movement	30.8	31.0 0.3		0.7	2.1 2.1		0.0	30.5		0.7	34.7 34.7	
38 Reference Time	30.8	31.0		0.7			0.0	30.5		0.7		
39 Adjusted Reference Time	34.0	34.0		7.0	2.1 7.0		34.0	34.0		38.2	34.7 38.2	
			Month		1.0		34.0	34.0		30.2	30.2	
Summary 40 Protected Option		West	North									
40 Protected Option		IA	41									
41 Permitted Option		2.0	38									
42 Split Option		1.0	72									
43 Minimum	41	1.0	38									
44 Combined	EDD	79		000								
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	11.5	7.0	13.5	49.8								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	34.0	38.2	7.0	34.0								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	7.0	34.0	7.0	0.0								
50 Combined	52.5	79.1	27.5	83.8								
51 Intersection Capacity Utiliza	tion	69.8%										
52 Level Of Service		С								Revision	2003.0	

Intersection Location: Pleasant Valley / US 101 NB Off Ra
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

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1 Movement	J	\rightarrow	7	—	←	T	7)	↓	4
1	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	0	0	0	1	0	2	0	3	1	0	3	0
3 Shared LT Lane (y/n)	Yes			Yes			Yes			Yes		-
4 Volume	0	0	0	296	0	513	0	1525	137	0	1548	0
5 Pedestrians			0			0			0			0
6 Ped Button (y/n)		Yes			Yes			Yes			Yes	
7 Pedestrian Timing Required		0			0			0			0	
8 Free Right (y/n)			Yes			Yes			✓ Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	 1900
10 Lost Time	0	0	0	3	0	3	0	3.5	3.5	0	3.5	0
11 Minimum Green	0	0	0	4	0	4	0	10	10	0	10	0
12 Reference Cycle Length	120											
13 Volume Combined	0.0	0.0	0.0	296.0	0.0	513.0	0.0	1525.0	137.0	0.0	1548.0	0.0
14 Volume Separate Left	0.0	0.0		296.0	0.0		0.0	1525.0		0.0	1548.0	
15 Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	0.885	1.000	0.908	1.000	1.000	0.908	1.000
16 Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	0.0	0.0	0.0	1805.0	0.0	2858.6	0.0	5175.6	1615.0	0.0	5175.6	0.0
18 Saturated Flow Separate	0.0	0.0		1805.0	0.0		0.0	5175.6		0.0	5175.6	
19 Pedestrian Interference Time		0.0	0.0		0.0	0.0	5.5	0.0	0.0	3.3	0.0	0.0
20 Pedestrian Frequency		0.0%			0.0%			0.0%			0.0%	
21 Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
22 Reference Time	0.0	0.0	0.0	19.7	0.0	21.5	0.0	35.4	10.2	0.0	35.9	0.0
23 Adjusted Reference Time	0.0	0.0	0.0	22.7	0.0	24.5	0.0	38.9	13.7	0.0	39.4	0.0
Permitted Option	0.0	0.0	0.0		0.0	2	0.0	00.0		0.0	00.1	0.0
24 Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	0			296	0.00		0	508		0	516	
26 Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		0.07	1.00	
29 Permitted Sat Flow	0.0	0.0		120.3	0.0		0.0	1725.2		0.0	1725.2	
30 Reference Time A	0.0	0.0		295.2	0.0		0.0	35.4		0.0	35.9	
31 Adjusted Saturation B	0.0	0.0		200.2	0.0		0.0	5175.6		0.0	5175.6	
32 Reference Time B		0.0			0.0			NA			NA	
33 Reference Time Lefts	0.0	0.0		27.7	0.0		NA	147.		NA	1.47.1	
34 Reference Time	0.0	0.0		21.1	27.7		147.	35.4		147.	35.9	
35 Adjusted Reference Time		0.0			27.7			38.9			39.4	
Split Timing		0.0			21.1			30.5			JJT	
36 Ref Time Combined		0.0			0.0			35.4			35.9	
37 Ref Time By Movement	0.0	0.0		19.7	0.0		0.0	35.4		0.0	35.9	
38 Reference Time	0.0	0.0		10.7	19.7		0.0	35.4		0.0	35.9	
39 Adjusted Reference Time	0.0	0.0		19.7	19.7		38.9	38.9		39.4	39.4	
Summary		West	North		10.7		50.9	50.9		55.4	55.4	
40 Protected Option		2.7).4								
41 Permitted Option		<i>r</i> 7.7).4								
42 Split Option		9.7		3.2								
43 Minimum		9.7		0.4								
44 Combined	18	59 59		. т								
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	0.0	24.5	13.7	0.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time48 Oncoming Left Direction	38.9 WBL	39.4 EBL	0.0 SBL	0.0 NBL								
			0.0									
49 Oncoming Left Adj Ref Time 50 Combined	19.7 58.5	0.0 63.9	13.7	0.0								
			13.1	U.U								
51 Intersection Capacity Utiliza	ition	53.3%								Davisles	2002.0	
52 Level Of Service		Α								Revision	∠003.0	

Intersection Location: Pleasant Valley / US 101 NB Off Ra
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

1 Movement	J	\rightarrow	7	F	1	L				4	↓	4
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	0	0	0	1	0	2	0	3	1	_ 0	3	0
3 Shared LT Lane (y/n)	☐ Yes			Yes			Yes	4555	400	Yes	4005	
4 Volume	0	0	0	444	0	677	0	1577	193	0	1235	0
5 Pedestrians			0			0			0			0
6 Ped Button (y/n)		Yes			Yes 0			Yes 0			∐ Yes 0	
7 Pedestrian Timing Required 8 Free Right (y/n)		0	Yes		U	Yes		U	✓ Yes		U	Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	1900	0	1900	3	0	3	0	3.5	3.5	1900	3.5	0
11 Minimum Green	0	0	0	4	0	4	0	10	10	-	10	0
12 Reference Cycle Length	120		Ū		Ū			10				
13 Volume Combined	0.0	0.0	0.0	444.0	0.0	677.0	0.0	1577.0	193.0	0.0	1235.0	0.0
14 Volume Separate Left	0.0	0.0		444.0	0.0	011.0	0.0	1577.0	130.0	0.0	1235.0	0.0
15 Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	0.885	1.000	0.908	1.000	1.000	0.908	1.000
16 Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	0.0	0.0	0.0	1805.0	0.0	2858.6	0.0	5175.6	1615.0	0.0	5175.6	0.0
18 Saturated Flow Separate	0.0	0.0		1805.0	0.0	. ,	0.0	5175.6		0.0	5175.6	
19 Pedestrian Interference Time	J.,	0.0	0.0		0.0	0.0	3.3	0.0	0.0	J.J	0.0	0.0
20 Pedestrian Frequency		0.0%			0.0%			0.0%			0.0%	
21 Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
22 Reference Time	0.0	0.0	0.0	29.5	0.0	28.4	0.0	36.6	14.3	0.0	28.6	0.0
23 Adjusted Reference Time	0.0	0.0	0.0	32.5	0.0	31.4	0.0	40.1	17.8	0.0	32.1	0.0
Permitted Option												
24 Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	0	0		444	0		0	526		0	412	
26 Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		0.07	1.00	
29 Permitted Sat Flow	0.0	0.0		120.3	0.0		0.0	1725.2		0.0	1725.2	
30 Reference Time A	0.0	0.0		442.8	0.0		0.0	36.6		0.0	28.6	
31 Adjusted Saturation B		0.0			0.0			5175.6			5175.6	
32 Reference Time B		0.0			0.0			NA			NA	
33 Reference Time Lefts	0.0	0.0		37.5	07.5		NA	00.0		NA	00.0	
34 Reference Time		0.0			37.5			36.6			28.6	
35 Adjusted Reference Time		0.0			37.5			40.1			32.1	
Split Timing 36 Ref Time Combined		0.0			0.0			36.6			28.6	
37 Ref Time By Movement	0.0	0.0		29.5	0.0		0.0	36.6		0.0	28.6	
38 Reference Time	0.0	0.0		29.5	29.5		0.0	36.6		0.0	28.6	
39 Adjusted Reference Time	0.0	0.0		29.5	29.5		40.1	40.1		32.1	32.1	
Summary		West	North		20.0		10.1	10.1		02.1	02.1	
40 Protected Option		2.5	40									
41 Permitted Option		7.5	40									
42 Split Option		9.5		2								
43 Minimum		9.5	40									
44 Combined		69	9.6									
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	0.0	31.4	17.8	0.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	40.1	32.1	0.0	0.0								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	29.5	0.0	0.0	0.0								
50 Combined	69.6	63.6	17.8	0.0								
51 Intersection Capacity Utiliza	tion	58.0%										
52 Level Of Service		В								Revision	2003.0	

CUMULATIVE YEAR 2030 WORKSHEETS

Intersection Location: Pleasant Valley / Lewis Road
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

								4		_			
1	Movement	J		7	F	1	L	7	T		4	↓	→
		EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2	Lanes	2	2	1	2	2	1	1	2	1	1	2	1
3	Shared LT Lane (y/n)	Yes			Yes			Yes			Yes		
4	Volume	236	636	30	638	401	244	64	298	363	221	974	250
5	Pedestrians			10			10			10			10
6	Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes			√ Yes	
7	Pedestrian Timing Required		20			17			28			27	
8	Free Right (y/n)			Yes			Yes			Yes			Yes
9	Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10	Lost Time	3	4	4	3	4	4	3	4	4	3	4	4
11	Minimum Green	4	10	10	4	10	10	4	10	10	4	10	10
12	Reference Cycle Length	120											
13	Volume Combined	236.0	636.0	30.0	638.0	401.0	244.0	64.0	298.0	363.0	221.0	974.0	250.0
14	Volume Separate Left	236.0	636.0		638.0	401.0		64.0	298.0		221.0	974.0	
15	Lane Utilization Factor	0.971	0.952	1.000	0.971	0.952	1.000	1.000	0.952	1.000	1.000	0.952	1.000
_	Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
	Saturated Flow Combined	3505.3	3617.6	1615.0	3505.3	3617.6	1615.0	1805.0	3617.6	1615.0	1805.0	3617.6	1615.0
	Saturated Flow Separate	3505.3	3617.6	2.0.0	3505.3	3617.6	2.5.5	1805.0	3617.6	2.5.5	1805.0	3617.6	
	Pedestrian Interference Time	000010	0.0	1.2	0000.0	0.0	1.2	100010	0.0	1.2	100010	0.0	1.2
_	Pedestrian Frequency		28.3%			28.3%			28.3%			28.3%	
_	Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
	Reference Time	8.1	21.1	2.2	21.8		18.1	4.3	9.9	27.0	14.7	32.3	18.6
	Adjusted Reference Time	11.1	25.1	14.0	24.8	18.4	22.1	7.3	19.1	31.0	17.7	36.3	22.6
23	Permitted Option	11.1	20.1	14.0	24.0	10.4	22.1	7.5	19.1	31.0	17.7	30.5	22.0
24	Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
	Volume Left Lane	118	318		319	201		64			221	487	
	Proportion Lefts Left	1 10	0.00		1	0.00		1	0.00		1	0.00	
		15.0									0.9		
27 28	Left turn Equivalents Left turn Factor	0.07	15.0 1.00		15.0 0.07	15.0 1.00		15.0 0.07	15.0 1.00		1.07	15.0 1.00	
		116.8											
	Permitted Sat Flow Reference Time A	121.2	1808.8		116.8 327.6	1808.8		120.3	1808.8		1925.3	1808.8	
		121.2	21.1		327.0			63.8	9.9		14.7	32.3	
_	Adjusted Saturation B Reference Time B		3617.6 NA			3617.6 NA			3617.6 NA			3617.6 NA	
	Reference Time Lefts	NIA	INA		NIA	NA		NIA	INA		NIA	INA	
		NA	404.0		NA	207.0		NA	60.0		NA	20.0	
-	Reference Time		121.2			327.6			63.8			32.3	
35	Adjusted Reference Time		125.2			331.6			67.8			36.3	
00	Split Timing		04.1			40.0			0.0			00.0	
	Ref Time Combined	2 4	21.1		04.0	13.3		4.0	9.9		44-	32.3	
	Ref Time By Movement	8.1	21.1		21.8	13.3		4.3	9.9		14.7	32.3	
	Reference Time	05.4	21.1		05.0	21.8		40.4	9.9		00.0	32.3	
39	Adjusted Reference Time	25.1	25.1		25.8	25.8		19.1	19.1		36.3	36.3	
	Summary		West	North									
	Protected Option	49		43									
	Permitted Option		1.6	67									
	Split Option).9	55									
	Minimum	49	9.9	43	.6								
44	Combined		93										
	Right Turns	EBR	WBR	NBR	SBR								
	Adjusted Reference Time	14.0	22.1	31.0	22.6								
	Cross Through Direction	NBT	SBT	WBT	EBT								
47	Cross Through Adj Ref Time	19.1	36.3	18.4	25.1								
	Oncoming Left Direction	WBL	EBL	SBL	NBL								
	Oncoming Left Adj Ref Time	24.8	11.1	17.7	7.3								
	Combined	57.9	69.5	67.0	54.9								
	Intersection Capacity Utiliza		77.9%	-	-								
	Level Of Service		D								Revision	2003 0	
J2											. 101101011	_000.0	

Intersection Location: Pleasant Valley / Lewis Road
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

1 Movement	1		7	←	+	t	1	1		L		J
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	2	2	1	2	2	1	1	2	1	1	2	1
3 Shared LT Lane (y/n)	Yes			Yes			Yes			Yes		
4 Volume	258	457	31	370	851	296	107	746	648	133	468	314
5 Pedestrians			10			10			10			10
6 Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes			✓ Yes	
7 Pedestrian Timing Required		20			17			28			27	
8 Free Right (y/n)			Yes Yes			Yes Yes			Yes Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	3	4	4	3	4	4	3	4	4	3	4	4
11 Minimum Green	4	10	10	4	10	10	4	10	10	4	10	10
12 Reference Cycle Length	120											
13 Volume Combined	258.0	457.0	31.0	370.0	851.0	296.0	107.0	746.0	648.0	133.0	468.0	314.0
14 Volume Separate Left	258.0	457.0		370.0	851.0		107.0	746.0		133.0	468.0	
15 Lane Utilization Factor	0.971	0.952	1.000	0.971	0.952	1.000	1.000	0.952	1.000	1.000	0.952	1.000
16 Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	3505.3	3617.6	1615.0	3505.3	3617.6	1615.0	1805.0	3617.6	1615.0	1805.0	3617.6	1615.0
18 Saturated Flow Separate	3505.3	3617.6	12.0.0	3505.3	3617.6	12.0.0	1805.0	3617.6	12.0.0	1805.0	3617.6	12.0.0
19 Pedestrian Interference Time	000010	0.0	1.2	0000.0	0.0	1.2	100010	0.0	1.2	100010	0.0	1.2
20 Pedestrian Frequency		28.3%			28.3%			28.3%			28.3%	
21 Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
22 Reference Time	8.8	15.2	2.3	12.7	28.2	22.0	7.1	24.7	48.1	8.8	15.5	23.3
23 Adjusted Reference Time	11.8	20.5	14.0	15.7	32.2	26.0	10.1	29.7	52.1	11.8	22.8	27.3
Permitted Option	11.0	20.5	14.0	10.7	52.2	20.0	10.1	23.1	JZ. 1	11.0	22.0	21.0
24 Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	129	229		185	426		107	373		133	234	
26 Proportion Lefts Left				100			107	0.00		133	0.00	
	1 1 0	0.00			0.00 15.0					0.9		
27 Left turn Equivalents 28 Left turn Factor	15.0 0.07	15.0 1.00		15.0 0.07	1.00		15.0 0.07	15.0 1.00		1.07	15.0 1.00	
29 Permitted Sat Flow					1808.8						1808.8	
30 Reference Time A	116.8 132.5	1808.8 15.2		116.8 190.0	28.2		120.3 106.7	1808.8 24.7		1925.3 8.8	15.5	
31 Adjusted Saturation B	132.3	3617.6		190.0	3617.6		100.7	3617.6		0.0	3617.6	
32 Reference Time B		3617.6 NA			3617.6 NA			3617.6 NA			3617.6 NA	
33 Reference Time B	NA	INA		NA	INA		NA	NA		NA	INA	
34 Reference Time	INA	132.5		INA	190.0		INA	106.7		INA	15.5	
		136.5			194.0			110.7			22.8	
35 Adjusted Reference Time		130.3			194.0			110.7			22.0	
Split Timing		45.0			00.0			04.7			45.5	
36 Ref Time Combined	0.0	15.2		40.7	28.2		7.4	24.7		0.0	15.5	
37 Ref Time By Movement	8.8	15.2		12.7	28.2		7.1	24.7		8.8	15.5	
38 Reference Time	00.5	15.2		20.0	28.2		00 7	24.7		00.0	15.5	
39 Adjusted Reference Time	20.5	20.5	N	32.2	32.2		29.7	29.7		22.8	22.8	
Summary		West	North									
40 Protected Option	44		41									
41 Permitted Option		4.0	110									
42 Split Option		2.8	52									
43 Minimum	44		41	.5								
44 Combined		85										
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	14.0	26.0	52.1	27.3								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	29.7	22.8	32.2	20.5								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	15.7	11.8	11.8	10.1								
50 Combined	59.3	60.6	96.2	58.0								
51 Intersection Capacity Utiliza	tion	80.2%		<u>-</u> -								
52 Level Of Service		D								Revision	2003.0	

Intersection Location: Pleasant Valley / Pancho
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

1 Movement	1	\rightarrow	7	•	—	t	1	1		L		J
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	■ NBT	NBR	SBL	SBT	SBR
2 Lanes	1	2	0	2	2	0	1	1	1	1	1	0
3 Shared LT Lane (y/n)	Yes			Yes		J	✓ Yes	'		☐ Yes	•	
4 Volume		1018	278	388	831	11	132	2	116	32	2	53
5 Pedestrians	.,	10.0	10			10		_	10		_	10
6 Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes			Yes	
7 Pedestrian Timing Required		17			17			23			0	
8 Free Right (y/n)			Yes			Yes			Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	3	4	4	3	4	4	2	3	3	2	3	3
11 Minimum Green	2	5	5	2	5	5	3	5	5	3	5	5
12 Reference Cycle Length	120											
13 Volume Combined	17.0	1296.0	0.0	388.0	842.0	0.0	0.0	134.0	116.0	32.0	55.0	0.0
14 Volume Separate Left	17.0	1296.0	0.0	388.0	842.0	0.0	132.0	2.0	110.0	32.0	55.0	0.0
15 Lane Utilization Factor	1.000	0.952	1.000	0.971	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000
16 Turning Factor Adjust	0.950	0.968	0.850	0.950	0.998	0.850	0.950	0.951	0.850	0.950	0.855	0.850
17 Saturated Flow Combined	1805.0	3501.2	0.030	3505.3	3610.5	0.030	0.930	3612.8	1615.0	1805.0	1625.4	0.030
18 Saturated Flow Separate	1805.0	3501.2	0.0	3505.3	3610.5	0.0	3610.0	1900.0	10.0	1805.0	1625.4	0.0
19 Pedestrian Interference Time	1000.0	0.3	1.2	0000.0	0.0	1.2	3310.0	0.0	1.2	1000.0	1.2	1.2
20 Pedestrian Frequency		28.3%	1.2		28.3%	1.2		28.3%	1.2		100.0%	1.2
21 Protected Option Allowed		TRUE			TRUE			FALSE			FALSE	
22 Reference Time	1.1	44.7	0.0	13.3	28.0	0.0	NA	NA	8.6	NA	NA	0.0
23 Adjusted Reference Time	5.0	48.7	9.0	16.3	32.0	9.0	NA NA	NA NA	11.6	NA NA	NA NA	8.0
Permitted Option	5.0	40.7	9.0	10.5	32.0	9.0	INA	INA	11.0	IVA	INA	0.0
24 Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	17	648		194	421		0	0.99 134		32	55	
26 Proportion Lefts Left	17	0.00		194	0.00		1	0.99		1	0.00	
27 Left turn Equivalents	15.0			15.0	15.0		15.0	15.1		0.9	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	0.07		1.07	1.00	
29 Permitted Sat Flow	120.3	1750.6		116.8	1805.3		0.07	243.2		1925.3	1625.4	
30 Reference Time A	17.0	44.7		199.2	28.0		0.0	66.1		2.1	5.3	
31 Adjusted Saturation B	17.0	3501.2		199.2	3610.5		0.0	0.0		2.1	1625.4	
32 Reference Time B		NA			3010.5 NA			12.5			5.3	
33 Reference Time B	NA	IVA		NA	INA		12.4	12.5		10.1	5.5	
34 Reference Time	INA	44.7		IVA	199.2		12.4	12.5		10.1	5.3	
35 Adjusted Reference Time		48.7			203.2			18.4			8.3	
Split Timing		40.7			200.2			10.4			0.3	
36 Ref Time Combined		44.7			28.0			4.5			5.3	
37 Ref Time Combined 37 Ref Time By Movement	1.1	44.7		13.3	28.0		4.4	0.1		2.1	5.3	
38 Reference Time	1.1	44.7		13.3	28.0		4.4	4.5		2.1	5.3	
39 Adjusted Reference Time	48.7	44.7		32.0	32.0		13.1	13.1		8.3	8.3	
Summary		West	North		32.0		13.1	13.1		0.3	0.3	
40 Protected Option 41 Permitted Option		3.2	N 19	A 3.4								
· ·												
42 Split Option 43 Minimum).7 5.0		.4 3.4								
	00			0.4								
44 Combined		83		000								
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	9.0	9.0	11.6	8.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	13.1	8.3	32.0	48.7								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	16.3	5.0	8.3	13.1								
50 Combined	38.4	22.3	51.9	69.8								
51 Intersection Capacity Utiliza	tion	69.5%								_		
52 Level Of Service		С								Revision	2003.0	

Intersection Location: Pleasant Valley / Pancho

Analyzed by: VRPA Technologies, Inc

Date and Time of Data: PM Peak

Alternative: Cumulative Year 2030 W/out Project

Project: Pacific Rock

City: Ventura County

1 Movement **EBL** WBL **WBT** WBR NBL NBT SBL SBT SBR **EBT EBR NBR** 2 Lanes 2 0 0 0 3 Shared LT Lane (y/n) Yes ✓ Yes Yes Yes Yes 4 Volume 891 970 430 34 173 26 472 6 14 31 5 Pedestrians 10 10 10 10 6 Ped Button (y/n) ✓ Yes ✓ Yes Yes ✓ Yes 7 Pedestrian Timing Required 17 17 23 Yes Yes 8 Free Right (y/n) Yes Yes 1900 9 Ideal Flow 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 10 Lost Time 4 4 3 11 Minimum Green 2 5 5 2 5 5 3 5 5 3 5 5 12 Reference Cycle Length 120 13 Volume Combined 34.0 1064.0 0.0 996.0 0.0 478.0 430.0 14.0 0.0 91.0 0.0 35.0 14 Volume Separate Left 34.0 1064.0 91.0 996.0 472.0 14.0 6.0 35.0 15 Lane Utilization Factor 1.000 0.952 1.000 0.971 0.952 1.000 1.000 1.000 1.000 1.000 1.000 1.000 16 Turning Factor Adjust 0.950 0.976 0.850 0.950 0.996 0.850 0.950 0.951 0.850 0.950 0.867 0.850 17 Saturated Flow Combined 1805.0 3529.4 0.0 3603.4 0.0 1615.0 3505.3 0.0 3612.4 1805.0 1647.6 0.0 18 Saturated Flow Separate 1805.0 3529.4 3505.3 3603.4 3610.0 1900.0 1805.0 1647.6 19 Pedestrian Interference Time 0.2 1.2 0.0 1.2 0.0 1.2 1.1 1.2 20 Pedestrian Frequency 28.3% 28.3% 28.3% 100.0% 21 Protected Option Allowed **TRUE** TRUE **FALSE FALSE** 22 Reference Time 2.3 0.0 3.1 0.0 NΑ 32.0 NA 36.4 33.2 NA NA 0.0 23 Adjusted Reference Time 5.3 40.4 9.0 6.1 37.2 9.0 NA 35.0 NA 8.0 NA NA Permitted Option 24 Proportion Lefts 0.00 0.00 1 0.99 0.00 25 Volume Left Lane 34 532 45.5 498 0 478 14 35 26 Proportion Lefts Left 0.00 0.00 1 0.99 0.00 15.0 15.0 15.0 27 Left turn Equivalents 15.0 15.0 15.1 0.9 15.0 28 Left turn Factor 0.07 1.00 0.07 1.00 0.07 0.07 1.07 1.00 29 Permitted Sat Flow 1801.7 1647.6 120.3 1764.7 116.8 0.0 242.8 1925.3 30 Reference Time A 33.9 36.4 46.7 33.2 0.0 236.3 3.6 3529.4 31 Adjusted Saturation B 3603.4 1647.6 0.0 32 Reference Time B NA NA 23.9 3.6 33 Reference Time Lefts NA NA 23.7 8.9 34 Reference Time 36.4 46.7 23.9 3.6 35 Adjusted Reference Time 40.4 50.7 26.9 8.0 Split Timing 36 Ref Time Combined 36.4 33.2 15.9 3.6 37 Ref Time By Movement 2.3 36.4 3.1 33.2 15.7 0.4 0.9 3.6 38 Reference Time 36.4 33.2 15.9 3.6 39 Adjusted Reference Time 40.4 40.4 37.2 37.2 20.9 20.9 8.0 8.0 Summary East West North South 40 Protected Option 46.5 NA 41 Permitted Option 50.7 26.9 42 Split Option 28.9 77.6 43 Minimum 46.5 26.9 44 Combined 73.4 Right Turns **EBR** WBR **NBR** SBR 45 Adjusted Reference Time 9.0 9.0 35.0 8.0 46 Cross Through Direction NBT **SBT WBT EBT** 47 Cross Through Adj Ref Time 20.9 8.0 37.2 40.4 48 Oncoming Left Direction WBL **EBL** SBL **NBL** 49 Oncoming Left Adj Ref Time 5.3 6.1 8.0 20.9 36.0 22.3 80.2 69.3 50 Combined 51 Intersection Capacity Utilization 66.8% 52 Level Of Service C Revision 2003.0

Intersection Location: Pleasant Valley / US 101 SB Ramps
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

City: Ventura County
Alternative: Cumulative Year 2030 W/out Projec

Project: Pacific Rock

							4		_			
1 Movement	J	\rightarrow	7	₽	←	L	7	T		4	↓	4
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	1	1	1	1	1	0	0	3	0	1	2	1
3 Shared LT Lane (y/n)	✓ Yes			Yes			Yes			Yes		
4 Volume	1092	4	213	3	0	9	0	1261	5	21	1033	1083
5 Pedestrians			10			10			10			10
6 Ped Button (y/n)		✓ Yes			Yes			✓ Yes			√ Yes	
7 Pedestrian Timing Required		14			0			14			14	
8 Free Right (y/n)			Yes Yes			Yes			Yes Yes			☐ Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	3		3	3	3	3	0	3.5	3.5	3	3.5	3.5
11 Minimum Green	4	4	4	4	4	4	0	10	10	4	10	10
12 Reference Cycle Length	120											
13 Volume Combined	0.0	1096.0	213.0	3.0	9.0	0.0	0.0	1266.0	0.0	21.0	1033.0	1083.0
14 Volume Separate Left	1092.0	4.0		3.0	9.0		0.0	1266.0		21.0	1033.0	
15 Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	0.908	1.000	1.000	0.952	1.000
16 Turning Factor Adjust	0.950	0.950	0.850	0.950	0.850	0.850	0.950	0.999	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	0.0	3610.7	1615.0	1805.0	1615.0	0.0	0.0	5172.5	0.0	1805.0	3617.6	1615.0
18 Saturated Flow Separate	3610.0	1900.0		1805.0	1615.0		0.0	5172.5		1805.0	3617.6	
19 Pedestrian Interference Time		0.0	1.2		1.2	1.2		0.0	1.2		0.0	1.2
20 Pedestrian Frequency		28.3%			100.0%			28.3%			28.3%	
21 Protected Option Allowed		FALSE			FALSE			TRUE			TRUE	
22 Reference Time	NA	NA	15.8	NA	NA	0.0	0.0	29.4	0.0	1.4	34.3	80.5
23 Adjusted Reference Time	NA	NA	18.8	NA	NA	7.0	0.0	32.9	13.5	7.0	37.8	84.0
Permitted Option												
24 Proportion Lefts	1	1.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	0	1096		3	9		0	422		21	517	
26 Proportion Lefts Left	1	1.00		1	0.00		1	0.00		1	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	0.07		0.07	1.00		0.07	1.00		1.07	1.00	
29 Permitted Sat Flow	0.0	241.3		120.3	1615.0		0.0	1724.2		1925.3	1808.8	
30 Reference Time A	0.0	545.1		3.0	1.9		0.0	29.4		1.4	34.3	
31 Adjusted Saturation B		0.0			1615.0			5172.5			3617.6	
32 Reference Time B		44.4			1.9			NA			NA	
33 Reference Time Lefts	44.3			8.2			NA			NA		
34 Reference Time		44.4			3.0			29.4			34.3	
35 Adjusted Reference Time		47.4			7.0			32.9			37.8	
Split Timing												
36 Ref Time Combined		36.4			1.9			29.4			34.3	
37 Ref Time By Movement	36.3	0.3		0.2	1.9		0.0	29.4		1.4	34.3	
38 Reference Time		36.4			1.9			29.4			34.3	
39 Adjusted Reference Time	39.4	39.4		7.0	7.0		32.9	32.9		37.8	37.8	
Summary		West	North									
40 Protected Option		IA	39									
41 Permitted Option		7.4		' .8								
42 Split Option		6.4).6								
43 Minimum	46	6.4	37	7.8								
44 Combined		84	.2									
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	18.8	7.0	13.5	84.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	32.9	37.8	7.0	39.4								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	7.0	39.4	7.0	0.0								
50 Combined	58.7	84.2	27.5	123.4								
51 Intersection Capacity Utiliza	ition	102.8%			•							
52 Level Of Service		G								Revision	2003.0	
			1									

Intersection Location: Pleasant Valley / US 101 SB Ramps
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

				_			_					
1 Movement	J	\rightarrow	7	•	1	L	7	T)	1	4
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	1	1	1	1	1	0	0	3	0	1	2	1
3 Shared LT Lane (y/n)	✓ Yes			Yes			Yes			Yes		
4 Volume	1228	4	116	12	0	12	0	1372	5	25	1117	945
5 Pedestrians			10			10			10			10
6 Ped Button (y/n)		✓ Yes			Yes			✓ Yes			✓ Yes	
7 Pedestrian Timing Required		14			_ 0			- 14			14	
8 Free Right (y/n)			Yes			Yes			Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	 1900
10 Lost Time	3	3	3	3	3	3	0	3.5	3.5	3	3.5	3.5
11 Minimum Green	4	4	4	4	4	4	0	10	10	4	10	10
12 Reference Cycle Length	120											
13 Volume Combined	0.0	1232.0	116.0	12.0	12.0	0.0	0.0	1377.0	0.0	25.0	1117.0	945.0
14 Volume Separate Left	1228.0	4.0	110.0	12.0	12.0	0.0	0.0	1377.0	0.0	25.0	1117.0	0 10.0
15 Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	0.908	1.000	1.000	0.952	1.000
16 Turning Factor Adjust	0.950	0.950	0.850	0.950	0.850	0.850	0.950	0.999	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	0.0	3610.6		1805.0	1615.0	0.0	0.0	5172.8	0.0	1805.0	3617.6	1615.0
18 Saturated Flow Separate	3610.0	1900.0	10.0	1805.0	1615.0	0.0	0.0	5172.8	0.0	1805.0	3617.6	1010.0
19 Pedestrian Interference Time	5510.0	0.0	1.2	.555.0	1.2	1.2	0.0	0.0	1.2	.555.0	0.0	1.2
20 Pedestrian Frequency		28.3%	1.2		100.0%	1.2		28.3%	1.2		28.3%	1.2
21 Protected Option Allowed		FALSE			FALSE			TRUE			TRUE	
22 Reference Time	NA	NA	8.6	NA	NA	0.0	0.0	31.9	0.0	1.7	37.1	70.2
23 Adjusted Reference Time	NA NA	NA	11.6	NA NA	NA	7.0	0.0	35.4	13.5	7.0	40.6	73.7
Permitted Option	IVA	IVA	11.0	11/1	INA	7.0	0.0	55.4	10.0	7.0	40.0	10.1
24 Proportion Lefts	1	1.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	0			12	12		0	459		25	559	
26 Proportion Lefts Left	1	1.00		1	0.00		1	0.00		1	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	0.07		0.07	1.00		0.07	1.00		1.07	1.00	
29 Permitted Sat Flow	0.07	241.2		120.3	1615.0		0.07	1724.3		1925.3	1808.8	
30 Reference Time A	0.0	612.9		120.3	2.1		0.0	31.9		1923.3	37.1	
31 Adjusted Saturation B	0.0	0.0		12.0	1615.0		0.0	5172.8		1.7	3617.6	
32 Reference Time B		48.9			2.1			3172.0 NA			3017.0 NA	
33 Reference Time Lefts	48.8	40.9		8.8	2.1		NA	INA		NA	INA	
34 Reference Time	40.0	48.9		0.0	8.8		INA	31.9		IVA	37.1	
35 Adjusted Reference Time		51.9			11.8			35.4			40.6	
Split Timing		31.8			11.0			33.4			40.0	
36 Ref Time Combined		40.9			2.1			31.9			37.1	
37 Ref Time Combined 37 Ref Time By Movement	40.8	0.3		0.8	2.1		0.0	31.9		1.7	37.1	
38 Reference Time	40.0	40.9		0.0	2.1		0.0	31.9		1.7	37.1	
39 Adjusted Reference Time	43.9	43.9		7.0	7.0		35.4	35.4		40.6	40.6	
Summary		West	North		1.0		33.4	55.4		70.0	+0.0	
40 Protected Option		A	42									
41 Permitted Option		I.9	42									
42 Split Option).9		5.0								
43 Minimum).9		0.0								
44 Combined	30		.5									
	EPP	WBR		SDD								
Right Turns 45 Adjusted Reference Time	EBR 11.6		NBR	SBR								
46 Cross Through Direction	11.6	7.0 SBT	13.5	73.7 EBT								
<u> </u>	NBT		WBT									
47 Cross Through Adj Ref Time48 Oncoming Left Direction	35.4 WBL	40.6 EBL	7.0 SBL	43.9								
				NBL								
49 Oncoming Left Adj Ref Time 50 Combined	7.0 54.1	43.9	7.0	0.0								
		91.5	27.5	117.7								
51 Intersection Capacity Utiliza	ition	98.1%								Dovinin	2002.0	
52 Level Of Service		F								Revision	∠∪∪3.U	

Intersection Location: Pleasant Valley / US 101 NB Off Ra
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

City: Ventura County
Alternative: Cumulative Year 2030 W/out Projec

Project: Pacific Rock

1	Movement	Ĵ	EBT	EBR	WBL	◆ WBT	WBR	NBL	1 NBT	NBR	SBL	SBT	SBR
2	Lanes	0	0	0	VVDL	0	2	0	3	NDK	SBL 0	3	36K
	Shared LT Lane (y/n)	Yes	U	U	Yes	U		Yes	3	I	Yes	3	
	Volume	0	0	0	385	0	708	0	1604	151	0	2028	0
	Pedestrians			0	000	J	00		100-1	0		2020	0
	Ped Button (y/n)		Yes			Yes			Yes			Yes	
	Pedestrian Timing Required		0			0			0			1 0	
	Free Right (y/n)			Yes			Yes		-	✓ Yes			Yes
	Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
	Lost Time	0		0	3	0	3	0	3.5	3.5	0	3.5	0
	Minimum Green	0	0	0	4	0	4	0	10	10	0	10	0
	Reference Cycle Length	120											
	Volume Combined	0.0	0.0	0.0	385.0	0.0	708.0	0.0	1604.0	151.0	0.0	2028.0	0.0
	Volume Separate Left	0.0	0.0	0.0	385.0	0.0	1 0010	0.0	1604.0	10110	0.0	2028.0	0.0
	Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	0.885	1.000	0.908	1.000	1.000	0.908	1.000
	Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
	Saturated Flow Combined	0.0	0.0	0.0	1805.0	0.0	2858.6	0.0	5175.6	1615.0	0.0	5175.6	0.0
	Saturated Flow Separate	0.0	0.0		1805.0	0.0		0.0	5175.6		0.0	5175.6	
	Pedestrian Interference Time		0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0
20	Pedestrian Frequency		0.0%			0.0%			0.0%			0.0%	
	Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
	Reference Time	0.0	0.0	0.0	25.6	0.0	29.7	0.0	37.2	11.2	0.0	47.0	0.0
23	Adjusted Reference Time	0.0	0.0	0.0	28.6	0.0	32.7	0.0	40.7	14.7	0.0	50.5	0.0
	Permitted Option												
24	Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
	Volume Left Lane	0	0		385	0		0	535		0	676	
26	Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
	Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0	
	Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		0.07	1.00	
29	Permitted Sat Flow	0.0	0.0		120.3	0.0		0.0	1725.2		0.0	1725.2	
30	Reference Time A	0.0	0.0		383.9	0.0		0.0	37.2		0.0	47.0	
31	Adjusted Saturation B		0.0			0.0			5175.6			5175.6	
32	Reference Time B		0.0			0.0			NA			NA	
33	Reference Time Lefts	0.0			33.6			NA			NA		
	Reference Time		0.0			33.6			37.2			47.0	
	Adjusted Reference Time		0.0			33.6			40.7			50.5	
	Split Timing												
	Ref Time Combined		0.0			0.0			37.2			47.0	
	Ref Time By Movement	0.0	0.0		25.6	0.0		0.0	37.2		0.0	47.0	
	Reference Time		0.0			25.6			37.2			47.0	
-	Adjusted Reference Time	0.0	0.0		25.6	25.6		40.7	40.7		50.5	50.5	
	Summary		West	North									
	Protected Option		3.6	50									
	Permitted Option		3.6	50									
42	Split Option		5.6	91									
	Minimum	25	5.6	50	.5								
	Combined		76										
	Right Turns	EBR	WBR	NBR	SBR								
	Adjusted Reference Time	0.0	32.7	14.7	0.0								
	Cross Through Direction	NBT	SBT	WBT	EBT								
	Cross Through Adj Ref Time	40.7	50.5	0.0	0.0								
48	Oncoming Left Direction	WBL	EBL	SBL	NBL								
49	Oncoming Left Adj Ref Time	25.6	0.0	0.0	0.0								
	Combined	66.3	83.2	14.7	0.0								
	Intersection Capacity Utilizat	tion	69.4%		_								
52	Level Of Service		С								Revision	2003.0	

Intersection Location: Pleasant Valley / US 101 NB Off Ra
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

1 Movement	1	\rightarrow	7	ſ	—	t	1	1	A	L		J
I Wovement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	■ NBT	NBR	SBL	SBT	SBR
2 Lanes	0			1	0	2	0	3	1	0	3	0
3 Shared LT Lane (y/n)	Yes			Yes		_	Yes	Ü		Yes		
4 Volume	0	0	0	460	0	757	0	1855	245	0	1946	0
5 Pedestrians	Ů		0	100		0		1000	0		1010	0
6 Ped Button (y/n)		Yes			Yes			Yes			Yes	Ŭ
7 Pedestrian Timing Required		0			0			0			0	
8 Free Right (y/n)			Yes			Yes		U	✓ Yes		J	Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	0	0	0	3	0	3	0	3.5	3.5	0	3.5	0
11 Minimum Green	0	0	0	4	0	4	0	10	10	0	10	0
12 Reference Cycle Length	120			·	Ū	•						
13 Volume Combined	0.0	0.0	0.0	460.0	0.0	757.0	0.0	1855.0	245.0	0.0	1946.0	0.0
14 Volume Separate Left	0.0	0.0	0.0	460.0	0.0	707.0	0.0	1855.0	240.0	0.0	1946.0	0.0
15 Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	0.885	1.000	0.908	1.000	1.000	0.908	1.000
16 Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	0.930	0.0	0.00	1805.0	0.0	2858.6	0.950	5175.6	1615.0	0.950	5175.6	0.00
18 Saturated Flow Separate	0.0	0.0	0.0	1805.0	0.0	2000.0	0.0	5175.6	1010.0	0.0	5175.6	0.0
19 Pedestrian Interference Time	0.0	0.0	0.0	1000.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
20 Pedestrian Frequency		0.0%	0.0		0.0%	0.0		0.0%	0.0		0.0%	0.0
21 Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
22 Reference Time	0.0	0.0	0.0	30.6	0.0	31.8	0.0	43.0	18.2	0.0	45.1	0.0
23 Adjusted Reference Time	0.0	0.0	0.0	33.6	0.0	34.8	0.0	46.5	21.7	0.0	48.6	0.0
	0.0	0.0	0.0	33.0	0.0	34.0	0.0	40.5	21.7	0.0	40.0	0.0
Permitted Option	1	0.00			0.00		1	0.00		4	0.00	
24 Proportion Lefts 25 Volume Left Lane	1	0.00		1	0.00		0	0.00		1 0	0.00	
	0	_		460	0			618			649	
26 Proportion Lefts Left	1 1 1 0	0.00		1 1 1 0	0.00		1 1 1 1	0.00		1	0.00	
27 Left turn Equivalents	15.0			15.0	15.0		15.0	15.0		15.0	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		0.07	1.00	
29 Permitted Sat Flow	0.0	0.0		120.3	0.0		0.0	1725.2		0.0	1725.2	
30 Reference Time A	0.0	0.0		458.7	0.0		0.0	43.0		0.0	45.1	
31 Adjusted Saturation B		0.0			0.0			5175.6			5175.6	
32 Reference Time B	0.0	0.0		00.0	0.0		NIA	NA		N.I.A	NA	
33 Reference Time Lefts	0.0	0.0		38.6	20.0		NA	42.0		NA	45.4	
34 Reference Time		0.0			38.6			43.0			45.1	
35 Adjusted Reference Time		0.0			38.6			46.5			48.6	
Split Timing		^ ^						40.0			45.4	
36 Ref Time Combined	^ ^	0.0		00.0	0.0			43.0			45.1	
37 Ref Time By Movement	0.0	0.0		30.6	0.0		0.0	43.0		0.0	45.1	
38 Reference Time	0.0	0.0		20.0	30.6		40.5	43.0		40.0	45.1	
39 Adjusted Reference Time	0.0	0.0	N	30.6	30.6		46.5	46.5		48.6	48.6	
Summary 40 Protected Option		West	North									
40 Protected Option		3.6	48									
41 Permitted Option		3.6	48									
42 Split Option		0.6	95									
43 Minimum	30).6	48	0.0								
44 Combined).2									
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	0.0	34.8	21.7	0.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	46.5	48.6	0.0	0.0								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	30.6	0.0	0.0	0.0								
50 Combined	77.1	83.4	21.7	0.0								
51 Intersection Capacity Utiliza	tion	69.5%								_		
52 Level Of Service		С								Revision	2003.0	

Intersection Location: Pleasant Valley / Lewis Road
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

1	Movement	1	\rightarrow	7	-	—	t	1	1		4		J
		EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2	Lanes	2	2	1	2	2	1	1	2	1	1	2	1
3	Shared LT Lane (y/n)	Yes			Yes			Yes			Yes		
4	Volume	236	645	30	642	405	248	64	298	367	225	974	250
5	Pedestrians			10			10			10			10
6	Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes			✓ Yes	
7	Pedestrian Timing Required		20			17			28			27	
8	Free Right (y/n)			Yes			Yes			Yes			Yes
9	Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10	Lost Time	3	4	4	3	4	4	3	4	4	3	4	4
11	Minimum Green	4	10	10	4	10	10	4	10	10	4	10	10
12	Reference Cycle Length	120											
13	Volume Combined	236.0	645.0	30.0	642.0	405.0	248.0	64.0	298.0	367.0	225.0	974.0	250.0
14	Volume Separate Left	236.0	645.0		642.0	405.0		64.0	298.0		225.0	974.0	
15	Lane Utilization Factor	0.971	0.952	1.000	0.971	0.952	1.000	1.000	0.952	1.000	1.000	0.952	1.000
16	Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
17	Saturated Flow Combined	3505.3	3617.6	1615.0	3505.3	3617.6	1615.0	1805.0	3617.6	1615.0	1805.0	3617.6	1615.0
18	Saturated Flow Separate	3505.3	3617.6		3505.3	3617.6		1805.0	3617.6		1805.0	3617.6	
19	Pedestrian Interference Time		0.0	1.2		0.0	1.2		0.0	1.2		0.0	1.2
20	Pedestrian Frequency		28.3%			28.3%			28.3%			28.3%	
21	Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
	Reference Time	8.1	21.4	2.2	22.0	13.4	18.4	4.3	9.9	27.3	15.0	32.3	18.6
23	Adjusted Reference Time	11.1	25.4	14.0	25.0	18.4	22.4	7.3	19.1	31.3	18.0	36.3	22.6
H	Permitted Option							_					
24	Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
	Volume Left Lane	118	323		321	203		64	149		225	487	
	Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
	Left turn Equivalents	15.0			15.0	15.0		15.0	15.0		0.9	15.0	
	Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		1.07	1.00	
	Permitted Sat Flow	116.8	1808.8		116.8	1808.8		120.3	1808.8		1925.3	1808.8	
	Reference Time A	121.2	21.4		329.7	13.4		63.8	9.9		15.0	32.3	
	Adjusted Saturation B	1 - 11-	3617.6		02011	3617.6		00.0	3617.6		1010	3617.6	
	Reference Time B		NA			NA			NA			NA	
	Reference Time Lefts	NA			NA			NA			NA		
	Reference Time		121.2			329.7			63.8			32.3	
	Adjusted Reference Time		125.2			333.7			67.8			36.3	
_	Split Timing								00				
	Ref Time Combined		21.4			13.4			9.9			32.3	
	Ref Time By Movement	8.1	21.4		22.0	13.4		4.3	9.9		15.0	32.3	
	Reference Time		21.4			22.0			9.9			32.3	
	Adjusted Reference Time	25.4	25.4		26.0	26.0		19.1	19.1		36.3	36.3	
-	Summary		West	North	South								
40	Protected Option).4		3.6								
	Permitted Option		3.7		'.8								
_	Split Option		.4		5.4								
	Minimum).4		3.6								
	Combined	- 30	93										
	Right Turns	EBR	WBR	NBR	SBR								
	Adjusted Reference Time	14.0	22.4	31.3	22.6								
	Cross Through Direction	NBT	SBT	WBT	EBT								
	Cross Through Adj Ref Time	19.1	36.3	18.4	25.4								
	Oncoming Left Direction	WBL	EBL	SBL	NBL								
	Oncoming Left Adj Ref Time	25.0	11.1	18.0	7.3								
	Combined	58.1	69.8	67.7	55.2								
	Intersection Capacity Utiliza		78.3 %	01.1	JJ.Z								
	Level Of Service	uon									Revision	2002.0	
52	Level Of Service		D								REVISION	2003.0	

Intersection Location: Pleasant Valley / Lewis Road
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

						A	4					
1 Movement	J	\rightarrow	7	.	—	L	7	T		J		4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	2	2	1	2	2	1	1	2	1	1	2	1
3 Shared LT Lane (y/n)	Yes		-	Yes	_	-	Yes		-	Yes		
4 Volume	258	459	31	372	858	298	107	746	650	135	468	314
5 Pedestrians			10			10			10			10
6 Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes			✓ Yes	
7 Pedestrian Timing Required					17			– 28			27	
8 Free Right (y/n)			Yes			Yes			Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	3	4	4	3	4	4	3	4	4	3	4	4
11 Minimum Green	4	10	10	4	10	10	4	10	10	4	10	10
12 Reference Cycle Length	120											
13 Volume Combined	258.0	459.0	31.0	372.0	858.0	298.0	107.0	746.0	650.0	135.0	468.0	314.0
14 Volume Separate Left	258.0	459.0	01.0	372.0	858.0	200.0	107.0	746.0	000.0	135.0	468.0	014.0
15 Lane Utilization Factor	0.971	0.952	1.000	0.971	0.952	1.000	1.000	0.952	1.000	1.000	0.952	1.000
16 Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	3505.3	3617.6	1615.0	3505.3	3617.6	1615.0	1805.0	3617.6	1615.0	1805.0	3617.6	1615.0
18 Saturated Flow Separate	3505.3	3617.6	1010.0	3505.3	3617.6	1010.0	1805.0	3617.6	1010.0	1805.0	3617.6	1010.0
19 Pedestrian Interference Time	3303.3	0.0	1.2	3303.3	0.0	1.2	1000.0	0.0	1.2	1000.0	0.0	1.2
20 Pedestrian Frequency		28.3%	1.2		28.3%	1.2		28.3%	1.2		28.3%	1.2
21 Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
22 Reference Time	0.0		2.2	40.7		22.4	7.1		40.2	0.0		22.2
	8.8	15.2	2.3	12.7	28.5	22.1	7.1 10.1	24.7	48.3	9.0	15.5	23.3
23 Adjusted Reference Time	11.8	20.6	14.0	15.7	32.5	26.1	10.1	29.7	52.3	12.0	22.8	27.3
Permitted Option	4	0.00		4	0.00			0.00			0.00	
24 Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	129	230		186	429		107	373		135	234	
26 Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		1.07	1.00	
29 Permitted Sat Flow	116.8	1808.8		116.8	1808.8		120.3	1808.8		1925.3	1808.8	
30 Reference Time A	132.5	15.2		191.0	28.5		106.7	24.7		9.0	15.5	
31 Adjusted Saturation B		3617.6			3617.6			3617.6			3617.6	
32 Reference Time B		NA			NA			NA			NA	
33 Reference Time Lefts	NA			NA			NA			NA		
34 Reference Time		132.5			191.0			106.7			15.5	
35 Adjusted Reference Time		136.5			195.0			110.7			22.8	
Split Timing												
36 Ref Time Combined		15.2			28.5			24.7			15.5	
37 Ref Time By Movement	8.8	15.2		12.7	28.5		7.1	24.7		9.0	15.5	
38 Reference Time		15.2			28.5			24.7			15.5	
39 Adjusted Reference Time	20.6	20.6		32.5	32.5		29.7	29.7		22.8	22.8	
Summary		West	North					<u>-</u>				
40 Protected Option		1.3	41									
41 Permitted Option		5.0	110									
42 Split Option		3.0	52									
43 Minimum	44	1.3	41	.6								
44 Combined		85	.9									
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	14.0	26.1	52.3	27.3								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	29.7	22.8	32.5	20.6								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	15.7	11.8	12.0	10.1								
50 Combined	59.4	60.8	96.7	58.0								
51 Intersection Capacity Utiliza	tion	80.6%			1							
52 Level Of Service		D								Revision	2003.0	

Intersection Location: Pleasant Valley / Pancho
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

						A	_					
1 Movement	J	\rightarrow	7	5	1			T				
i iliovement	EBL	EBT	EBR	₩BL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	1		0	2	2	0	1	1	1	1	1	0
3 Shared LT Lane (y/n)	Yes		Ü	Yes			✓ Yes			Yes		
4 Volume	17	1018	294	459	831	11	143	2	180	32	2	53
5 Pedestrians	17	1010	10	400	001	10	140		100	52		10
6 Ped Button (y/n)		✓ Yes	10		✓ Yes	10		✓ Yes	10		Yes	10
7 Pedestrian Timing Required		17			17			23			0	
		17	Yes		17	Yes		23	Yes		U	Yes
8 Free Right (y/n)	4000	4000	_	4000	4000	_	4000	4000		4000	1000	_
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
10 Lost Time	3		4	3	4	4	2	3	3	2	3	3
11 Minimum Green	2	5	5	2	5	5	3	5	5	3	5	5
12 Reference Cycle Length	120											
13 Volume Combined	17.0	1312.0	0.0	459.0	842.0	0.0	0.0	145.0	180.0	32.0	55.0	0.0
14 Volume Separate Left	17.0	1312.0		459.0	842.0		143.0	2.0		32.0	55.0	
15 Lane Utilization Factor	1.000	0.952	1.000	0.971	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000
16 Turning Factor Adjust	0.950	0.966	0.850	0.950	0.998	0.850	0.950	0.951	0.850	0.950	0.855	0.850
17 Saturated Flow Combined	1805.0		0.0	3505.3	3610.5	0.0	0.0	3612.6	1615.0	1805.0	1625.4	0.0
18 Saturated Flow Separate	1805.0	3496.0		3505.3	3610.5		3610.0	1900.0		1805.0	1625.4	
19 Pedestrian Interference Time		0.3	1.2		0.0	1.2		0.0	1.2		1.2	1.2
20 Pedestrian Frequency		28.3%			28.3%			28.3%			100.0%	
21 Protected Option Allowed		TRUE			TRUE			FALSE			FALSE	
22 Reference Time	1.1	45.3	0.0	15.7	28.0	0.0	NA	NA	13.4	NA	NA	0.0
23 Adjusted Reference Time	5.0	49.3	9.0	18.7	32.0	9.0	NA	NA	16.4	NA	NA	8.0
Permitted Option												
24 Proportion Lefts	1	0.00		1	0.00		1	0.99		1	0.00	
25 Volume Left Lane	17	656		229.5	421		0	145		32	55	
26 Proportion Lefts Left	1			1	0.00		1	0.99		1	0.00	
27 Left turn Equivalents	15.0			15.0	15.0		15.0	15.1		0.9	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	0.07		1.07	1.00	
29 Permitted Sat Flow	120.3	1748.0		116.8	1805.3		0.07	243.0		1925.3	1625.4	
30 Reference Time A	17.0	45.3		235.7	28.0		0.0	71.6		2.1	5.3	
31 Adjusted Saturation B	17.0	3496.0		233.1	3610.5		0.0	0.0		2.1	1625.4	
32 Reference Time B		3490.0 NA			3010.5 NA			12.8			5.3	
33 Reference Time Lefts	NA	INA		NA	INA		12.8	12.0		10.1	3.3	
34 Reference Time	INA	45.3		INA	235.7		12.0	12.8		10.1	5.3	
					239.7							
35 Adjusted Reference Time		49.3			239.7			18.7			8.3	
Split Timing		45.0			00.0			4.0				
36 Ref Time Combined	4.4	45.3		45-	28.0		4.0	4.8		2 1	5.3	
37 Ref Time By Movement	1.1	45.3		15.7	28.0		4.8	0.1		2.1	5.3	
38 Reference Time	40.0	45.3		00.0	28.0		40.4	4.8			5.3	
39 Adjusted Reference Time	49.3			32.0	32.0		13.1	13.1		8.3	8.3	
Summary		West	North									
40 Protected Option		3.0	N									
41 Permitted Option		9.7		3.7								
42 Split Option		1.3		.4								
43 Minimum	68	3.0		3.7								
44 Combined		86	6.7									
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	9.0	9.0	16.4	8.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	13.1	8.3	32.0	49.3								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	18.7	5.0	8.3	13.1								
50 Combined	40.8	22.3	56.6	70.4								
51 Intersection Capacity Utiliza		72.3%	23.0	. 5								
52 Level Of Service		72.576 C								Revision	2003.0	
											_000.0	

Intersection Location: Pleasant Valley / Pancho
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

						A	4					
1 Movement		\rightarrow	7	5	←			T		L		
i illovelilette	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	1	2	0	2	2	0	1	1	1	1	1	0
3 Shared LT Lane (y/n)	Yes			Yes	_		✓ Yes	-	-	Yes		
4 Volume	34	891	179	123	970	26	483	6	469	14	4	31
5 Pedestrians			10			10			10			10
6 Ped Button (y/n)		✓ Yes			✓ Yes			✓ Yes			Yes	
7 Pedestrian Timing Required		17			17			23			0	
8 Free Right (y/n)			Yes			Yes			Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	 1900
10 Lost Time	3		4	3	4	4	2	3	3	2	3	3
11 Minimum Green	2	5	5	2	5	5	3	5	5	3	5	5
12 Reference Cycle Length	120						-		-			
13 Volume Combined	34.0	1070.0	0.0	123.0	996.0	0.0	0.0	489.0	469.0	14.0	35.0	0.0
14 Volume Separate Left	34.0	1070.0	0.0	123.0	996.0	0.0	483.0	6.0	400.0	14.0	35.0	0.0
15 Lane Utilization Factor	1.000	0.952	1.000	0.971	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000
16 Turning Factor Adjust	0.950	0.932	0.850	0.950	0.996	0.850	0.950	0.951	0.850	0.950	0.867	0.850
17 Saturated Flow Combined	1805.0	3526.8	0.030	3505.3	3603.4	0.030	0.930	3612.3	1615.0	1805.0	1647.6	0.030
18 Saturated Flow Separate	1805.0	3526.8	0.0	3505.3	3603.4	0.0	3610.0	1900.0	1010.0	1805.0	1647.6	0.0
19 Pedestrian Interference Time	1003.0	0.2	1.2	3303.3	0.0	1.2	3010.0	0.0	1.2	1000.0	1.1	1.2
20 Pedestrian Frequency		28.3%	1.2		28.3%	1.2		28.3%	1.2		100.0%	1.2
21 Protected Option Allowed		TRUE			TRUE			FALSE			FALSE	
22 Reference Time	2.2	36.6	0.0	4.2		0.0	NΙΛ		24.0	NA	NA	0.0
23 Adjusted Reference Time	2.3 5.3	40.6	9.0	4.2 7.2	33.2 37.2	9.0	NA NA	NA NA	34.8 37.8	NA NA	NA NA	0.0 8.0
	5.3	40.0	9.0	1.2	31.2	9.0	INA	INA	31.0	INA	INA	0.0
Permitted Option	4	0.00		4	0.00		4	0.00		4	0.00	
24 Proportion Lefts	1	0.00		1	0.00		1	0.99		1	0.00	
25 Volume Left Lane	34	535		61.5	498		0	489		14	35	
26 Proportion Lefts Left	1	0.00		1 1 1 1	0.00		1	0.99		1	0.00	
27 Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.1		0.9	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	0.07		1.07	1.00	
29 Permitted Sat Flow	120.3	1763.4		116.8	1801.7		0.0	242.7		1925.3	1647.6	
30 Reference Time A	33.9	36.6		63.2	33.2		0.0	241.8		0.9	3.6	
31 Adjusted Saturation B		3526.8			3603.4			0.0			1647.6	
32 Reference Time B		NA			NA			24.2			3.6	
33 Reference Time Lefts	NA	00.0		NA			24.1	0.4.0		8.9		
34 Reference Time		36.6			63.2			24.2			3.6	
35 Adjusted Reference Time		40.6			67.2			27.2			8.0	
Split Timing												
36 Ref Time Combined		36.6			33.2			16.2			3.6	
37 Ref Time By Movement	2.3	36.6		4.2	33.2		16.1	0.4		0.9	3.6	
38 Reference Time		36.6			33.2			16.2			3.6	
39 Adjusted Reference Time	40.6	40.6		37.2	37.2		21.2	21.2		8.0	8.0	
Summary		West	North									
40 Protected Option		7.8	N									
41 Permitted Option		7.2	27									
42 Split Option		7.8	29									
43 Minimum	47	7.8		'.2								
44 Combined		75										
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	9.0	9.0	37.8	8.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	21.2	8.0	37.2	40.6								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	7.2	5.3	8.0	21.2								
50 Combined	37.4	22.3	83.0	69.8								
51 Intersection Capacity Utiliza	tion	69.2%			•							
52 Level Of Service		С								Revision	2003.0	
			l .									

Intersection Location: Pleasant Valley / US 101 SB Ramps
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

1 Movement	1	\rightarrow	7	ſ	←	t	1	1		J	I	J
l limit of the limit	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	1	1	1	1	1	0	0	3	0	1	2	1
3 Shared LT Lane (y/n)	✓ Yes			Yes			Yes			Yes		
4 Volume	1092	4	230	3	0	9	0	1325	5	21	1087	1083
5 Pedestrians			10			10			10			10
6 Ped Button (y/n)		✓ Yes			Yes			✓ Yes			✓ Yes	
7 Pedestrian Timing Required		14			_ 0			-			14	
8 Free Right (y/n)			Yes			Yes			Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	<u></u>
10 Lost Time	3		3	3	3	3	0	3.5	3.5	3	3.5	3.5
11 Minimum Green	4		4	4	4	4	0	10	10	4	10	10
12 Reference Cycle Length	120											
13 Volume Combined	0.0	1096.0	230.0	3.0	9.0	0.0	0.0	1330.0	0.0	21.0	1087.0	1083.0
14 Volume Separate Left	1092.0		200.0	3.0	9.0	0.0	0.0	1330.0	0.0	21.0	1087.0	1000.0
15 Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	0.908	1.000	1.000	0.952	1.000
16 Turning Factor Adjust	0.950	0.950	0.850	0.950	0.850	0.850	0.950	0.999	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	0.930	3610.7	1615.0	1805.0	1615.0	0.030	0.930	5172.7	0.030	1805.0	3617.6	1615.0
18 Saturated Flow Separate	3610.0	1900.0	1010.0	1805.0	1615.0	0.0	0.0	5172.7	0.0	1805.0	3617.6	1010.0
19 Pedestrian Interference Time	3010.0	0.0	1.2	1005.0	1.2	1.2	0.0	0.0	1.2	1003.0	0.0	1.2
20 Pedestrian Frequency		28.3%	1.2		100.0%	1.2		28.3%	1.2		28.3%	1.2
21 Protected Option Allowed		FALSE			FALSE			TRUE			TRUE	
22 Reference Time	NA		47.4	NIA		0.0	0.0	30.9	0.0	4.4	36.1	00.5
		NA	17.1 20.1	NA NA	NA NA	0.0 7.0	0.0		0.0	1.4 7.0	39.6	80.5
23 Adjusted Reference Time	NA	NA	20.1	NA	NA	7.0	0.0	34.4	13.5	7.0	39.0	84.0
Permitted Option		4.00			0.00			0.00		1	0.00	
24 Proportion Lefts	1	1.00		1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	0			3			0	443		21	544	
26 Proportion Lefts Left	1	1.00		1	0.00		1	0.00		1	0.00	
27 Left turn Equivalents	15.0			15.0	15.0		15.0	15.0		0.9	15.0	
28 Left turn Factor	0.07	0.07		0.07	1.00		0.07	1.00		1.07	1.00	
29 Permitted Sat Flow	0.0	241.3		120.3	1615.0		0.0	1724.2		1925.3	1808.8	
30 Reference Time A	0.0	545.1		3.0	1.9		0.0	30.9		1.4	36.1	
31 Adjusted Saturation B		0.0			1615.0			5172.7			3617.6	
32 Reference Time B		44.4			1.9			NA			NA	
33 Reference Time Lefts	44.3			8.2			NA			NA		
34 Reference Time		44.4			3.0			30.9			36.1	
35 Adjusted Reference Time		47.4			7.0			34.4			39.6	
Split Timing												
36 Ref Time Combined		36.4			1.9			30.9			36.1	
37 Ref Time By Movement	36.3	0.3		0.2	1.9		0.0	30.9		1.4	36.1	
38 Reference Time		36.4			1.9			30.9			36.1	
39 Adjusted Reference Time	39.4	39.4		7.0	7.0		34.4	34.4		39.6	39.6	
Summary	East	West	North	South								
40 Protected Option	N	IA	41	.4								
41 Permitted Option		7.4		0.6								
42 Split Option		5.4		3.9								
43 Minimum		5.4		0.6								
44 Combined			3.0									
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	20.1	7.0	13.5	84.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	34.4	39.6	7.0	39.4								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	7.0	39.4	7.0	0.0								
50 Combined	61.4	86.0	27.5	123.4								
51 Intersection Capacity Utiliza		102.8%	21.0	120.7								
52 Level Of Service	uon									Revision	2002.0	
JZ Level Of Service		G								KEVISION	∠003.0	

Intersection Location: Pleasant Valley / US 101 SB Ramps
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

						4		4	•				
1	Movement			1	•	—	L	7			4	♣	4
		EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
_	Lanes	1	1	1	1	1	0	0	3	0	1	2	1
3	Shared LT Lane (y/n)	✓ Yes			Yes			Yes			Yes		
_	Volume	1228	4	124	12	0	12	0	1411	5	25	1142	945
	Pedestrians			10			10			10		_	10
	Ped Button (y/n)		✓ Yes			Yes			✓ Yes			✓ Yes	
	Pedestrian Timing Required		14			0			14			14	
	Free Right (y/n)			Yes			☐ Yes			Yes			☐ Yes
	Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
	Lost Time	3	3	3	3	3	3	0	3.5	3.5	3	3.5	3.5
	Minimum Green	4	4	4	4	4	4	0	10	10	4	10	10
	Reference Cycle Length	120											
	Volume Combined	0.0	1232.0	124.0	12.0	12.0	0.0	0.0	1416.0	0.0	25.0	1142.0	945.0
_	Volume Separate Left	1228.0	4.0		12.0	12.0		0.0	1416.0		25.0	1142.0	
_	Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	0.908	1.000	1.000	0.952	1.000
	Turning Factor Adjust	0.950	0.950	0.850	0.950	0.850	0.850	0.950	0.999	0.850	0.950	1.000	0.850
	Saturated Flow Combined	0.0	3610.6	1615.0	1805.0	1615.0	0.0	0.0	5172.9	0.0	1805.0	3617.6	1615.0
	Saturated Flow Separate	3610.0	1900.0		1805.0	1615.0		0.0	5172.9		1805.0	3617.6	
_	Pedestrian Interference Time		0.0	1.2		1.2	1.2		0.0	1.2		0.0	1.2
	Pedestrian Frequency		28.3%			100.0%			28.3%			28.3%	
	Protected Option Allowed		FALSE			FALSE			TRUE			TRUE	
_	Reference Time	NA	NA	9.2	NA	NA	0.0	0.0	32.9	0.0	1.7	37.9	70.2
23	Adjusted Reference Time	NA	NA	12.2	NA	NA	7.0	0.0	36.4	13.5	7.0	41.4	73.7
	Permitted Option												
	Proportion Lefts	1	1.00		1	0.00		1	0.00		1	0.00	
	Volume Left Lane	0	1232		12	12		0	472		25	571	
26	Proportion Lefts Left	1	1.00		1	0.00		1	0.00		1	0.00	
27	Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		0.9	15.0	
28	Left turn Factor	0.07	0.07		0.07	1.00		0.07	1.00		1.07	1.00	
29	Permitted Sat Flow	0.0	241.2		120.3	1615.0		0.0	1724.3		1925.3	1808.8	
30	Reference Time A	0.0	612.9		12.0	2.1		0.0	32.9		1.7	37.9	
	Adjusted Saturation B		0.0			1615.0			5172.9			3617.6	
	Reference Time B		48.9			2.1			NA			NA	
33	Reference Time Lefts	48.8			8.8			NA			NA		
	Reference Time		48.9			8.8			32.9			37.9	
35	Adjusted Reference Time		51.9			11.8			36.4			41.4	
	Split Timing												
	Ref Time Combined		40.9			2.1			32.9			37.9	
	Ref Time By Movement	40.8	0.3		8.0	2.1		0.0	32.9		1.7	37.9	
	Reference Time		40.9			2.1			32.9			37.9	
39	Adjusted Reference Time	43.9	43.9		7.0	7.0		36.4	36.4		41.4	41.4	
	Summary		West	North									
	Protected Option	N		43									
	Permitted Option		.9	41									
	Split Option	50		77									
	Minimum	50		41	.4								
44	Combined		92	2.3									
	Right Turns	EBR	WBR	NBR	SBR								
	Adjusted Reference Time	12.2	7.0	13.5	73.7								
	Cross Through Direction	NBT	SBT	WBT	EBT								
	Cross Through Adj Ref Time	36.4	41.4	7.0	43.9								
	Oncoming Left Direction	WBL	EBL	SBL	NBL								
	Oncoming Left Adj Ref Time	7.0	43.9	7.0	0.0								
	Combined	55.6	92.3	27.5	117.7								
	Intersection Capacity Utiliza	tion	98.1%			•							
52	Level Of Service		F								Revision	2003.0	

Intersection Location: Pleasant Valley / US 101 NB Off Ra
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: AM Peak

City: Ventura County
Alternative: Cumulative Year 2030 Plus Project

Project: Pacific Rock

								4					
1	Movement	J		7	—	+	L		T		5	1	4
		EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2	Lanes	0	0	0	1	0	2	0	3	1	0	3	0
3	Shared LT Lane (y/n)	Yes			Yes			Yes			Yes		
	Volume	0	0	0	437	0	708	0	1604	166	0	2030	0
5	Pedestrians			0			0			0			0
6	Ped Button (y/n)		Yes			Yes			Yes			Yes	
7	Pedestrian Timing Required		0			0			0			0	
	Free Right (y/n)			Yes			Yes			✓ Yes			Yes
	Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	 1900
10	Lost Time	0	0	0	3	0	3	0	3.5	3.5	0	3.5	0
11	Minimum Green	0	0	0	4	0	4	0	10	10	0	10	0
12	Reference Cycle Length	120											
	Volume Combined	0.0	0.0	0.0	437.0	0.0	708.0	0.0	1604.0	166.0	0.0	2030.0	0.0
	Volume Separate Left	0.0	0.0	0.0	437.0	0.0	700.0	0.0	1604.0	100.0	0.0	2030.0	0.0
	Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	0.885	1.000	0.908	1.000	1.000	0.908	1.000
	Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
	Saturated Flow Combined	0.930	0.0	0.0	1805.0	0.0	2858.6	0.950	5175.6	1615.0	0.9	5175.6	0.00
	Saturated Flow Separate	0.0	0.0	0.0	1805.0	0.0	2000.0	0.0	5175.6	1010.0	0.0	5175.6	0.0
	Pedestrian Interference Time	0.0	0.0	0.0	1005.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Pedestrian Frequency		0.0%	0.0		0.0%	0.0		0.0%	0.0		0.0%	0.0
	Protected Option Allowed		TRUE			TRUE			TRUE			TRUE	
	Reference Time	0.0		0.0	20.4		20.7	0.0		40.0	0.0		0.0
		0.0	0.0	0.0	29.1	0.0	29.7	0.0	37.2	12.3	0.0	47.1	0.0
	Adjusted Reference Time	0.0	0.0	0.0	32.1	0.0	32.7	0.0	40.7	15.8	0.0	50.6	0.0
	Permitted Option	4	0.00			0.00		4	0.00			0.00	
	Proportion Lefts	1	0.00		1	0.00		1	0.00		1	0.00	
	Volume Left Lane	0	0		437	0		0	535		0	677	
	Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
	Left turn Equivalents	15.0	15.0		15.0	15.0		15.0	15.0		15.0	15.0	
	Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		0.07	1.00	
_	Permitted Sat Flow	0.0	0.0		120.3	0.0		0.0	1725.2		0.0	1725.2	
	Reference Time A	0.0	0.0		435.8	0.0		0.0	37.2		0.0	47.1	
	Adjusted Saturation B		0.0			0.0			5175.6			5175.6	
	Reference Time B		0.0			0.0			NA			NA	
	Reference Time Lefts	0.0			37.1			NA			NA		
	Reference Time		0.0			37.1			37.2			47.1	
	Adjusted Reference Time		0.0			37.1			40.7			50.6	
	Split Timing												
	Ref Time Combined		0.0			0.0			37.2			47.1	
	Ref Time By Movement	0.0	0.0		29.1	0.0		0.0	37.2		0.0	47.1	
	Reference Time		0.0			29.1			37.2			47.1	
39	Adjusted Reference Time	0.0	0.0		29.1	29.1		40.7	40.7		50.6	50.6	
	Summary		West	North									
	Protected Option		2.1	50	.6								
41	Permitted Option	37	'.1	50									
	Split Option		9.1	91	.3								
	Minimum	29	9.1	50	.6								
44	Combined		79	.6									
	Right Turns	EBR	WBR	NBR	SBR								
	Adjusted Reference Time	0.0	32.7	15.8	0.0								
	Cross Through Direction	NBT	SBT	WBT	EBT								
	Cross Through Adj Ref Time	40.7	50.6	0.0	0.0								
	Oncoming Left Direction	WBL	EBL	SBL	NBL								
	Oncoming Left Adj Ref Time	29.1	0.0	0.0	0.0								
	Combined	69.7	83.3	15.8	0.0								
	Intersection Capacity Utiliza		69.4%			l							
	Level Of Service		C C								Revision	2003.0	
02	20.01 01 001 1100											_000.0	

Intersection Location: Pleasant Valley / US 101 NB Off Ra
Analyzed by: VRPA Technologies, Inc
Date and Time of Data: PM Peak

				_			_					
1 Movement	J	\rightarrow	7	•	←	L	7	T)	I I	4
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
2 Lanes	0	0	0	1	0	2	0	3	1	0	3	0
3 Shared LT Lane (y/n)	Yes			Yes			Yes			Yes		-
4 Volume	0	0	0	485	0	757	0	1855	255	0	1946	0
5 Pedestrians			0			0			0			0
6 Ped Button (y/n)		Yes			Yes			Yes			Yes	
7 Pedestrian Timing Required		0			0							
8 Free Right (y/n)			Yes			Yes			✓ Yes			Yes
9 Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	— 1900
10 Lost Time	0		0	3	0	3	0	3.5	3.5	0	3.5	0
11 Minimum Green	0		0	4	0	4	0	10	10	0	10	0
12 Reference Cycle Length	120				-	-	-					
13 Volume Combined	0.0		0.0	485.0	0.0	757.0	0.0	1855.0	255.0	0.0	1946.0	0.0
14 Volume Separate Left	0.0	0.0	0.0	485.0	0.0	131.0	0.0	1855.0	233.0	0.0	1946.0	0.0
15 Lane Utilization Factor	1.000	1.000	1.000	1.000	1.000	0.885	1.000	0.908	1.000	1.000	0.908	1.000
16 Turning Factor Adjust	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850	0.950	1.000	0.850
17 Saturated Flow Combined	0.950	0.0	0.00	1805.0	0.0	2858.6	0.950	5175.6	1615.0	0.950	5175.6	0.0
18 Saturated Flow Separate	0.0	0.0	0.0	1805.0	0.0	2000.0	0.0	5175.6	1010.0	0.0	5175.6	0.0
19 Pedestrian Interference Time	0.0	0.0	0.0	1605.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
20 Pedestrian Frequency		0.0%	0.0		0.0%	0.0		0.0%	0.0		0.0%	0.0
		TRUE			TRUE						TRUE	
21 Protected Option Allowed	0.0		0 0	20.0		24.0	0.0	TRUE	40.0	0.0		0.0
22 Reference Time	0.0		0.0	32.2	0.0	31.8	0.0	43.0	18.9	0.0	45.1	0.0
23 Adjusted Reference Time	0.0	0.0	0.0	35.2	0.0	34.8	0.0	46.5	22.4	0.0	48.6	0.0
Permitted Option		0.00			0.00			0.00		1	0.00	
24 Proportion Lefts	1			1	0.00		1	0.00		1	0.00	
25 Volume Left Lane	0			485	0		0	618		0	649	
26 Proportion Lefts Left	1	0.00		1	0.00		1	0.00		1	0.00	
27 Left turn Equivalents	15.0			15.0	15.0		15.0	15.0		15.0	15.0	
28 Left turn Factor	0.07	1.00		0.07	1.00		0.07	1.00		0.07	1.00	
29 Permitted Sat Flow	0.0	0.0		120.3	0.0		0.0	1725.2		0.0	1725.2	
30 Reference Time A	0.0	0.0		483.7	0.0		0.0	43.0		0.0	45.1	
31 Adjusted Saturation B		0.0			0.0			5175.6			5175.6	
32 Reference Time B		0.0			0.0			NA			NA	
33 Reference Time Lefts	0.0			40.2			NA			NA		
34 Reference Time		0.0			40.2			43.0			45.1	
35 Adjusted Reference Time		0.0			40.2			46.5			48.6	
Split Timing												
36 Ref Time Combined		0.0			0.0			43.0			45.1	
37 Ref Time By Movement	0.0	0.0		32.2	0.0		0.0	43.0		0.0	45.1	
38 Reference Time		0.0			32.2			43.0			45.1	
39 Adjusted Reference Time	0.0	0.0		32.2	32.2		46.5	46.5		48.6	48.6	
Summary		West	North									
40 Protected Option	35	5.2	48									
41 Permitted Option).2	48									
42 Split Option	32	2.2	95	5.1								
43 Minimum	32	2.2	48	3.6								
44 Combined		80	.9									
Right Turns	EBR	WBR	NBR	SBR								
45 Adjusted Reference Time	0.0	34.8	22.4	0.0								
46 Cross Through Direction	NBT	SBT	WBT	EBT								
47 Cross Through Adj Ref Time	46.5	48.6	0.0	0.0								
48 Oncoming Left Direction	WBL	EBL	SBL	NBL								
49 Oncoming Left Adj Ref Time	32.2	0.0	0.0	0.0								
50 Combined	78.8	83.4	22.4	0.0								
51 Intersection Capacity Utiliza		69.5%			l							
52 Level Of Service		03.3 % C								Revision	2003.0	
											_000.0	

APPENDIX F-2 VCAPCD DATA FOR PACIFIC ROCK QUARRY, EXTEC USAGE 2015—2016



VCAPCD Data for Pacific Rock Quarry EXTEC Usage 2015 - 2016

PACIFIC ROCK EQUIPMENT USAGE LOG - 2012/2015
DESCRIPTION OF EQUIPMENT EXTEC MODEL 5-5, SERVALINO, 9542, STAREN NO. 776-55-0501 WAXWELL NO. AA0031
STATE PORTABLE EQUIPMENT REGISTRATION NO.: 120144

EQUIPMENT OPERATOR AND CONTROLLER OF LOG SHEET:

DATE	TIME ON	TIME OFF	BREAKDOWNS	METER	FUEL	TOMS PROCESSED	NOTES
7-14-15	9:00	3,00			6	270	
7-15-15	*7 435	\$ (30	Y		1.2	540	
7-16-15	7.60	2.000	V			5,60	
7-17-15	200	THE RESERVE OF THE PARTY OF THE			THE STATE OF THE S	555	
7-21-15	1200	200/8	X		11	520	
7-22-15	River	3.00	X		a	470	
7-74-15	7.00	1.00	W .			550	
7-24-15	7:00	3.00	X		Lin	570	
7-28-15	7.00	3.05	K		I II	555	
	7.00		-		6	290	G Screen broken
7-30-15	800	100	N N		IL	530	
7-11-15	700	3:00	The second secon			560	
3 - 5 - 15	7 00	3000			- 44	558	
9 4-15	**************************************	300	X		12	550	
3-5-15		100	X		Li_	545	0.0
9-6-15	1500	3:00	X		M	480	Rollas a required
8.76	7.00		X		11	530	Major service required
8-10-15		3:00	X		12	540	char pump (MEDRINGE)
B 11-15	7:00		X			590	
8 11.15	7.00	3100	X,		13	550	ELIVEY - NE FO
9 15-15	7 (C)	Midd	~		7	260	clockical problem has
			Screen TOP SO	1			
1-815	800	100	X		11	560	
9-15	7'00		Maria Maria		12_	560 540	
1-10-15		3'00	White the same of		12	560	Finished Change to So
1 28-15	7.00	3:00	X X			440	Makerial Good Too wet
7-29-15	7 00		X		13	460	
1-30-15	8:00	3 00	X		975 ₃	355	是有一种的一种。 第一种种种种种种种种种种种种种种种种种种种种种种种种种种种种种种种种种种

EXTEU S-5

Pacific Rock Equipment Log Year: 2015.

Description of Equipment: EXTEC, MODEL S-5, SERIAL NO. 9542, STABEN NO. 778-55-0501, MAXWELL NO. AA0001

State Portable Equipment Registration No.: 130144

Equipment Opperator and Controller of Log Sheet:

Date	Time On	Time Off	Breakdowns	Meter	Fuel	Tons Processed	Notes
10-1-15	7:00 AL	300 PM	X		12	430	Material still art slow
10-2-15	7:00 AM	2:00 PM	x		q	370	How some or about on it to
10-5-15	10:00 04	3'00 PM	Х		12=	280	
10-645	7:00 AM	3:00 PM	×			450	One more dox of motorial
16-7-15	7 CO AM	3 00 PM	Y	anne dallande innovembre demini	12	400	Nervove moterial
11-6-15	9 SO AM	3 00 PM	K		10	495	Mix this material for tree
11-9-15	7:00 AM	3 00 PM	X		13	520	
11-10-15	2'00 AM	5:00 PM	X		12	515	
11-11-15	7:00 AM	JOO PM			12	520	
11-12-15	7:00 AM	500 PM	· ×		13	520	
11-0-15	7:00 AM	Ilino AH	X		6	260	starter fault.
11-18-15	Biog AM	3100 AM	×			500	
1-14-15	7:00 AM	3100 PM	X		13	520	Move to basement home
11-20 15	7:00 AM	12300 BM	X		8	240	
12-09-15	8:00 AM	300 PM	A			500	
2-10-15	7.00 AM	3:00 PM	Х		13	540	
12-11-15	7:00 AM	H4 00:E	X		12	525	
				quantum de la companya de la company			
					despublicangly receiptions and many receives selected		
					printed to the printed in the state of the s	7585	

EXTEU S-5

Pacific Rock Equipment Log Year: 2016

Description of Equipment: EXTEC, MODEL S-5, SERIAL NO. 9542, STABEN NO. 776-55-0501, MAXWELL NO. AA0001

State Portable Equipment Registration No.: 130144

Equipment Opoerator and Controller of Log Sheet:

Date	Time On	Time Off	Breakdowns	Meter	Fuel	Tons Processed	Notes
1-12-16	9.00	3:00	X		li.	490	
1-13-16	7 00	3.00	X	MATERIAL CONTROL OF THE PARTY O	13	520	
1-14:16	7.00	3.00	X		13	510	Move to Polm trees!
1-28-16	7100	300	X		12	500	
1-29-16	7.00	3 90	×		13	530	Major clean up under st
2-3-16	7/66	3:00	V V	\$	12	500	
2-4-16	7:00	3.00	x		13	520	
2-5-16	7:00	3.00	X		12	490	tooket material
2-10-16	9:00	3.00	X		10	400	still art material less
2-18-16	7:00	3100	X X		13	52.0	
2-19-16	7:00	3:00	X		13	500	collactionic left to to
2-21-16	7:00	3:00	X		12	520	
2-23 -K	7:00	3:00	X		13	520	bearing is gove shirt do
3-3-16	8:00	3:00	X		12	500	
3-4-16	7:00	3:00	X		13	520	
3.916	2500	3300	X		12	500	
3-10-16	7:00	3300	K		17	500	material for 3 downers
3-19-16	8:00	3:00	X	Material script in the Copy of the	12	460	
3-15-16	7:00	3:00	X		1.1	730	
3-16-16	7.00	1:00	X		10	390	material is gove, move ex
4-20-16	9:00	3:00	Yes		9	340	fold it & clean it, forms
4-25-16	800	3.00	X		10_	460	
1-26-16	7100	3:00	K		13	480	

EXTEC 3-5

Pacific Rock Equipment Log Year: 2016

Description of Equipment EXTEC, MODEL S-5, SERIAL NO. 9542, STABEN NO. 776-55-0501, MAXWELL NO. AA0001

State Portable Equipment Registration No.: 130144

Equipment Opperator and Controller of Log Sheet:

Date	Time On	Time Off	Breakdowns	Meter	Fuel	Tons Processed	Notes
5-3-16	7:00	3:00	K		12	48.0	
5-4-16	7.00	3'00	X		12	500	The state of the s
5-5-16	700	3.00	X		13	500	A STATE OF THE PARTY OF THE PAR
5-17-16	8:00	3.00	X		13	480	
5-18-16	7:00	3:00	X		13	520	
5-19-16	7.00	3.00	X		13	520	move extent to new pole
5.26%	7.00	3.00	Y		13	520	
5-17-16	7:00	3:00	X		12	520	
5 30 16	7.00	3.0 0	Y		12	500	-
5-31-10	7.00	200	YES		10	380	electrical problems he
6-8-16	9.00	3:00	X		8	420	
6-10-16	7.00	2:00	×	MAN COLUMN TO COLUMN TO THE CASE	12	480	
6-21-16	7.00	3'00	X		13	500	
6-22-16	7100	3:00	X		12	500	Cr.
6-23-16	7:00	3:00	*		12	490	
7:5:16	10:00	3:00			12	340	
7-7-16	7:00	\$100	K		12	490	
78 16	7:00	3:00	1		12	500	Change scients and week
7-19-16	7:00	3:00	X		13	520	
7-20-16	700	3.00	X		13	900	
7-21-16	7.05	3:00	X		13	500	
7-27-16	7:00	3:00	X		13	500	
7-28-16	7:00	3.00	X		13	520	

Total: 37,345 Tons

APPENDIX G

WATER QUALITY IMPACT ASSESSMENT, STORAGE AND USE OF BLASTING AGENTS, PACIFIC ROCK QUARRY





MEMORANDUM

374 Poli Street, Suite 200 • Ventura, California 93001

Date: March 8, 2019

To: Mr. Brian McCarthy, Ventura County Planning Division

From: Mr. Brian Anderson, P.G., Sespe Consulting, Inc.

Cc:

Re: Water Quality Impact Assessment, Storage and Use of Blasting Agents, Pacific Rock Quarry,

Camarillo, CA

Background and Objectives

The Pacific Rock Quarry is a hard rock quarry located at the end of Pancho Road in Camarillo, California. The quarry site constitutes a 111 acre property, which slopes towards the west from an elevation of 940 feet above mean sea level (amsl) to 165 feet amsl, along the southwest side of Conejo Mountain. Mining of rock at and in the vicinity of the quarry dates back to the late 1800s, with the current quarry footprint having been initiated sometime during the 1950s.

As part of ongoing and future quarrying operations, the stone mined at the site is initially retrieved and sized using blasting agents. Blasting is a common mining method used at hard rock quarries to access the material and initially size the rock for further processing.

The purpose of this technical memo is to evaluate the relationship between the blasting conducted at the Pacific Rock Quarry and the groundwater quality, using information presented in prior site-specific studies and review of available literature pertaining to the geologic and hydrogeologic setting, and types of blasting agents used at the site.

Geologic Setting

The following description of the Pacific Rock Quarry geology is taken from an engineering geologic report prepared by Gold Coast Geoservices, Inc. (GCG) in 2010, the quarry produces a variety of construction materials rock products from the Canejo Volcanics, specifically a dacitic breccia. The Canejo Volcanics are estimated to be as much as 3,000 meters thick (Yerkes and Campbell, 1979), and consist of three volcanic units: 1) dark extrusive basaltic rocks; 2) light gray to pinkish gray dacitic breccia; and 3) dark intrusive basaltic rocks. The light gray dacitic breccia is reportedly the principal rock type mined at the quarry, which is described as consisting of unsorted angular fragments of hard, fine-grained dacite to andesite within a detrital matrix of the same composition. Intrusive, generally

Pacific Rock, Inc.

vertically orientated basaltic dikes 10 to 20 feet in width occur in the northeast and southeast areas of the guarry.

Based on geologic mapping of quarry exposures completed by GCG (2010), along with map information prepared by Dibblee and Ehrenspeck (1990), the underlying igneous rocks are characterized as massive and unstratified. However, the Canejo Volcanics reportedly exhibit high angle, intersecting joint sets. Gold Coast Geoservices, Inc. (2010) indicates that the joint surfaces strike north 20 to 45 degrees east, dipping at 55 to 85 degrees to the northwest or southeast; and strike north 35 to 70 degrees west, and dip 80 to 90 degrees southwest. The vertical dikes strike approximately north 45 to 60 degrees west. While this structural fabric has been determined by GCG (2010) as having created conditions for wedge failure and shear zones within several areas of the quarry, the pervasiveness of the structural fabric is unknown at depth. However, these features are reportedly limited in extent and are associated with the basaltic dikes (GCG, 2010). Additionally, jointing can tend to be more pronounced at the margins of intrusive rock bodies, as magma emplacement and cooling at the edges (Balk, 1937). Thus, the rock competency is expected to generally increase, with less joint surfaces at depth.

Groundwater Conditions

Given the type and nature of the site geologic setting, groundwater is anticipated to be primarily attributed to a fracture flow system, hosted within the intrusive igneous rocks. A well completion report prepared by Valley well Drilling for a well located onsite indicates that beyond about 15 feet below ground surface (bgs), varying competency rock occurs to a depth of approximately 200 feet bgs. From 200 feet to 278 feet bgs, the driller reported hard to total depth. Based on this log, the surface lithologies are consistent with the interpreted hydrogeologic conditions; that is, the indurated rock would not be considered to produce appreciable qualities of groundwater, and is likely limited to the fracture water at depth. According to a well completion report prepared by Hopkins Groundwater Consultants dated April 2004, following well completion and development, static groundwater was measured at 84.4 feet bgs. Based on a pump test, the well was estimated to produce about 10 gallons per minute (gpm). This relatively low yield is typical of a heterogenous/anisotropic fractured rock, with relatively low permeability and marginal conductivity. Consequently, this type of hydrogeologic regime would not be expected to provide significant recharge capacity, nor readily communicate with other aquifers, except in instances where there are preferential flow paths.

Environmental Characteristics of ANFO

For the Pacific Rock quarry, blasting agents are used to size the rock so that it can be processed using onsite equipment. The primary blasting agent is ammonium nitrate fuel oil (ANFO), which is typically used at mine sites. Other ancillary materials used at the quarry include detonator sensitive emulsion and nitroglycerine based explosives, detonating cord, DC cast boosters (primers), detonators, delays, relays, starters, lead-in-lines, shock tubes.

At mine sites, bulk ANFO is placed into blasting holes, which upon detonation break apart the rock to initially size the material. Compositionally, ANFO products typically consist of ammonium nitrate

Pacific Rock, Inc.

(NH⁴NO³) and fuel oil. Environmental risks associated with ANFO are site-specific and are related to characteristics such as the type of soil, the depth of the groundwater, presence of surface water, and the amount and infiltration rates of precipitation (Degnan et al., 2016). With respect to environmental impact, ANFO can pose a significant risk to groundwater. Specifically, in groundwater, ANFO can be a source of nitrogen as ammonium (NH⁴) and nitrate (NO3⁻) contamination. These constituents are the direct products of NH⁴NO³, which constitutes about 90% of commonly used commercial explosives by weight (Degnan et al., 2016). Additionally, the ammonia (NH⁴) can also affect groundwater quality.

According to Forsyth et al. (1995), the following mechanisms for the release of nitrates to the environment from blasting agents are:

- 1) Spillage during transport;
- 2) Dissolution (leaching) of explosives agents in "wet" blast holes; and
- 3) Undetonated explosives agents remaining in the rock following the blast.

A study by Defence R&D Canada (2010) found that the detonation of ANFO in saturated conditions is often incomplete. Consequently, due to its high solubility in wet environments ANFO can be lost directly due to dissolution. However, the relative potential risk to water quality can vary based on the type of ANFO product. For example, a study by Revey (1996) evaluated the leachability of several types of ANFO, including gels and emulsions, which found that NO3⁻ releases from emulsions and gels are considerably lower than ANFO; however even these products will leach over time (Cameron et al., 2007; Golder Associates, 2014), resulting in contamination.

Mitigation Approaches

Considering the hydrogeologic setting at the Pacific Rock quarry and relative depth of groundwater, with the proper storage, handling and use of ANFO, the potential for impacts to groundwater quality can be mitigated. In order to reduce the risk of release to groundwater, the practices and procedures listed below are to be implemented at the Pacific Rock quarry site:

- 1) Handling of all blasting agents shall be limited to qualified and licensed blasting contractors at all times.
- 2) All blasting products shall be stored only in approved containers, specifically designed for the sake keeping of explosives.
- 3) Any spillage of ANFO or other explosives shall be immediately cleaned up, and properly disposed of in strict accordance with applicable state and federal regulations.
- 4) The type of ANFO agent selected shall be appropriate for the specific environmental conditions.
- 5) Inspect the blast holes prior to placement of the ANFO to determine water is present. In cases where the boreholes have standing water or are moist, no material shall be placed into the holes until dry conditions are observed.

Pacific Rock, Inc.

- 6) Blast designs and loading controls shall be reviewed to minimize the length of explosive columns, select proper stemming and to ensure to optimize complete detonation.
- 7) A current inventory of the types and quantities, along with Material Safety Data Sheets, shall be maintained onsite by qualified personnel. Relevant information shall be included in the site's pollution prevention plans, including the Hazardous Materials Business Plan and Stormwater Pollution Prevention Plan.

References

Balk, R., 1937. Structural behavior of igneous rocks: Geological Society of America Memoir 5, 177 p.

Cameron, A., Corkey, D., MacDonald, G., Forsyth, B., and Gong, T., 2007. An investigation of ammonium nitrate loss to mine discharge water at Davik Diamond Mines, EXPLO Conference, Wollongong, NSW, pp. 3-4.

Defence R&D Canada, 2010. Assessment of ANFO on the environment, Technical Investigation 09-01, DRDC Valcartier TM-2009-195, 52 p.

Degnan, J. R., Bohlke, J. K., Pelham, K., Langlais, D. M., and Walsh, G. J., 2016. Identification of groundwater nitrate contamination from explosives used in road construction: isotopic, chemical and hydrologic evidence, Environmental Science and Technology, pp. 593-603.

Dibblee, T. W. Jr., and Ehrenspeck, H. E., 1990. Geologic Map of the Camarillo and Newbury Park Quadrangles, Ventura County, California, Dibblee Foundation Map DF-28, scale 1:24000.

Forsyth, W., Cameron, A., and Miller, S., 1995. Explosives and water quality, Sudbury '95 Proceedings of the Conference on Mining and the Environment, Sudbury, Ontario, vol. 2, pp 795-803.

Gold Coat Geoservices, Inc., 2010. Engineering Geologic Report, Modification to Conditional Use Permit (CUP #3817-3), Pacific Rock Quarry, 185 p.

Golder Associates, 2014., Technical Memorandum, Amulsar gold project: estimate of nitrate and ammonia concentrations in mine water as a product of blasting, 14 p.

Hopkins Groundwater Consultants, Inc., 2004. Summary of operations report, water supply well construction project, Canejo Mountain Memorial Park, Ventura County, California, 14 p.

Revey, G. F., 1996. Practical methods to control explosives losses and reduce ammonia and nitrate levels in mine water, Mining Engineering, vol. 48, p. 61-65.

APPENDIX H

RESPONSE TO PACIFIC ROCK QUARRY: LU10-0003 UPDATED STATUS OF OUTSTANDING INVOICES AND ENVIRONMENTAL IMPACT REPORT INFORMATION DELAYS DATED MARCH 12, 2019, PACIFIC ROCK QUARRY EXPANSION





374 Poli Street, Suite 200 • Ventura, CA 93001 Office (805) 275-1515 • Fax (805) 667-8104

April 1, 2019

Mr. Brian McCarthy Mining Program Manager **Ventura County Resource Management Agency** 800 South Victoria Avenue Ventura, California 93009

Re: Response to Pacific Rock Quarry: LU10-0003 Updated Status of Outstanding Invoices and Environmental Impact Report Information Delays dated March 12, 2019, Pacific Rock Quarry Expansion

Dear Mr. McCarthy,

Sespe Consulting, Inc. (Sespe) is pleased to provide this response on behalf of Pacific Rock, Inc. (Pacific Rock) to address the comments received on March 12, 2019 pertaining to the Pacific Rock Quarry Conditional Use Permit (CUP) modification. We have organized this response letter to include the comments in italics, followed by our response. In addition to our responses, we have included a revised Project Description, Reclamation Plan, and select Weight Tickets as attachments.

Environmental Setting Comments

Comment 1: Annual Production Environmental Setting

Based on the County's review of the data provided (from the Mining Operation Annual Reports) and scenarios outlined by our EIR Consultant, Benchmark Resources, the annual production baseline environmental setting will be based on an average production over the previous 10 years, 2008 through 2017. Data provided in the Operator Annual Reports show an average annual production of 20,900 tons.

No new information was provided to support the assertion that 2005 production levels as provided in the Project Operating Parameters document submitted February 12, 2019 represents an appropriate baseline under CEQA.

According to the current Air Protection Control District's Permit to Operate (No. 00489), Pacific Rock is authorized to produce up to 500,000 tons per year for combined material throughput. Recent and historical production values have been provided in the revised Project Description dated April 1, 2019. Pacific Rock is proposing maximum annual production of 468,000. Please refer to the revised Project Description for additional details regarding Annual Production.

Comment 2: Daily Production and Traffic Environmental Setting

To establish the daily production and traffic environmental setting, you submitted a series of weigh ticket summaries and weigh tickets, which document daily loads (truck tips). This data shows the maximum daily truck trips achieved was 30 loads (60 one-way truck trips), which occurred on March 31, 2017. As such, 60 one-way truck trips will be used as the daily truck traffic baseline environmental setting.

Pacific Rock, Inc. April 1, 2019

The assertion that the environmental setting for truck traffic is 120 one-way truck trips (60 truckloads) as provided in the Project Operating Parameters document is not supported by the weight tickets and data that has been submitted.

Pacific Rock has provided job and weight tickets that demonstrate the achievement of 60 loads a day or 120 one way trips a day. Pacific Rock continues to find and review data and we expect to be able to provide additional backup soon. Please refer to the below table for a summation of days and loads.

Da	Daily Loads						
Date	Number of Loads						
1/13/2005	60						
1/16/2005	60						
1/21/2005	60						
1/22/2005	60						
1/25/2005	60						
1/27/2005	60						
1/29/2005	60						
2/19/2005	60						
2/20/2005	60						
2/21/2005	60						
2/22/2005	60						
2/25/2005	60						
5/20/2005	60						
5/23/2005	60						
3/10/2016	60						

Most recently, Pacific Rock has achieved the maximum 60 loads (120 one-way trips) on March 10, 2016, one year before the Notice of Preparation (NOP) was published on August 23, 2017. Select weight tickets have been attached for reference. The remaining weight tickets can be furnished upon request.

Comment 3: Peak Hour Traffic Environmental Setting

In the Project Operating Parameters document, you've asserted that the environmental setting for truck traffic is 30 loads per hour between 7 AM and 9 AM (total of 60 loads, 120 one-way truck trips), and that 15 loads occurred per hour between 4 PM and 6 PM (total of 30 loads, 60 one-way truck trips). First, the maximum level reached for any one day was 30 loads for the entire day on March 31, 2017. Based on the evidence provided, the AM truck trip volume you are claiming to have achieved could not have been possible. Second, your assertions regarding your existing PM truck traffic generation represent a violation of your Conditional Use Permit conditions of approval, which stipulate a daily 4 PM closing time.

Over the course of this application processing, necessary documentation has not been submitted to support your assertions of a baseline environmental setting. Absent any further evidence, the baseline environmental setting for AM and PM peak hour truck trips are assumed to be zero.

The environmental setting for truck traffic that was proposed in the Project Operating Parameters document reflected the maximum trips that Pacific Rock could generate during the AM peak hours. Pacific Rock has the ability to load 1 truck every 30 seconds and generate 120 loads per hour. They are not currently required to keep

Pacific Rock, Inc. April 1, 2019

records of when the trucks depart the site, however for the purpose of this analysis, the existing and future AM peak hour trips should be 30 loads or 60 one-way trips per hour. In the proposed PM peak hour, Pacific Rock expects to generate 15 loads or 30 one-way trips per hour.

Currently, Pacific Rock does not generate any truck trips during the PM peak hours. However, due to the proposed extended hours of operation, there is potential to increase truck trips during the PM peak hours. The extended hours will be utilized on an as-needed basis and daily operations will continue to cease at 4:00 pm unless there is a demand for "after-hours" or post 4:00 pm shipping.

Pacific Rock is not proposing any changes to the maximum daily number of trips.

Comment 4: Water and Energy

With respect to annual and daily water use, evidence provided on February 12, 2019 does not support your assertion that the usage factors should be calculated based on the single year 2005 production data.

As with the annual production describe above, the Annual Reports show that over the previous 10 years, the existing operation has averaged approximately 20,911 tons annually as the baseline environmental setting.

No evidence has been submitted to inform the EIR baseline environmental setting for electrical use of the existing operation.

Water

Pacific Rock primarily utilizes recycled water from an irrigation pond for onsite operations. The tertiary water is supplied by the Camarillo Sanitary District. Currently, it is estimated that approximately 27.9-acre feet per year (AFY) of recycled water is utilized onsite. The proposed modification and expansion will consume roughly 83.5 AFY of recycled water. The non-potable water is drawn from the irrigation pond and is held in a 12,000-gallon tank. An onsite well is proposed to provide potable water for the 24-hour security trailer and will not provide resources for mining and reclamation operations.

Fuel

Diesel fuel invoices from 2016 have been reviewed and compiled into the below table. Please refer below for details regarding the annual and average daily fuel use.

Energy - Diesel Fuel Consumption						
Date	Diesel (gal)	Monthly Total (gal)				
1/13/2016	3,148.6	4 402 0				
1/19/2016	1,043.4	4,192.0				
2/5/2016	4,198.9	4,198.9				
3/3/2016	1,000.5					
3/4/2016	4,020.8	7,425.5				
3/21/2016	2,404.2					
4/6/2016	4,402.9					
4/21/2016	4,915.6	14,229.0				
4/26/2016	4,910.5					
5/18/2016	3,901.3	3,901.3				

Pacific Rock, Inc. April 1, 2019

6/1/2016	5,026.5	7 242 2
6/20/2016	2,215.7	7,242.2
7/8/2016	3,001.5	7.022.6
7/26/2016	4,032.1	7,033.6
8/9/2016	4,087.2	4,087.2
9/7/2016	2,959.6	6,917.5
9/22/2016	3,957.9	0,917.5
10/20/2016	4,661.1	4661.1
11/10/2016	3,035.1	3,035.1
12/7/2016	4,304.6	4,304.6

Total Fuel Consumed (gal)	71,228
Average Monthly Use (gal)	5936
Average Daily Use (gal)	228

In 2016, Pacific Rock consumed 71,228 gallons of fuel or 228 gallons per day (assuming a 312-day operational year). Fuel use is generally proportional to production.

Electricity

Pacific Rock has also provided the electricity usage information from January 2018 to February 2019. The below table provides a monthly breakdown of use and a total kWh annual use. We have provided overall averages for monthly and daily use.

Energy – Electricity Consumption						
Billing Period	kWh	Average daily use during specified period				
1/4/2018 to 2/4/2018	762	24.58				
2/2/2018 to 3/6/2018	913	28.53				
4/5/2018 to 5/4/2018	875	30.17				
5/4/2018 to 6/5/2018	691	21.59				
6/5/2018 to 7/5/2018	694	23.13				
7/5/2018 to 8/3/2018	688	23.72				
8/3/2018 to 9/4/2018	651	20.34				
9/4/2018 to 10/3/2018	547	18.86				
10/3/2018 to 11/1/2018	583	20.10				
11/1/2018 to 12/4/2018	745	22.58				
12/4/2018 to 1/4/2019	866	27.94				
1/4/2019 to 2/4/2019	494	15.94				

Total Year kWh usage	8509
Average Monthly kWh usage	709

Pacific Rock, Inc. April 1, 2019

Average Daily kWh usage	27
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The Noise and Air Quality Analysis have been submitted under separate cover.

Please call me or Helen Eloyan at (805) 275-1515 if you have any questions or if you need additional information.

Respectfully submitted,

for a Heal

John Hecht

President

Sespe Consulting, Inc.

- Attachments 1. Project Description, dated 04/01/19
 - 2. Reclamation Plan, dated 04/01/19
 - 3. Select Weight Tickets

Attachment 3 of Sespe 4/1/2019 Memorandum – Select Weight Tickets

Attachments 1 and 2 of Sespe 4/1/2019 memorandum are on file at Ventura County and available for review on request.

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STABEN BROS. INC. • License #981942

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ACCEPTED AS SATISFACTORY BY:_

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123200 Tare	1	65440	15.20	711	
159330 Net	3	65130	15.24	11	
79.47 Tons	7	48899	17.08	<i>i</i> *	H.
CARRIER CARRIER		47	Transport	// / =	
DRIVER EQUIPT, NO.	PACIFIC	ROCK, INC., Weighmaster	×7 34	21/1	
TONY 302	GROSS BY	BREV Juder		Mth Day	Year /
TRUCK I.D. No		DE	PUTYDATE		
RAILER I.D. No.	TARE			Mth, ~ Day	Voor
FRAILER J. I.D. No.	BY /	Let I miletal DE	PUTY DATE	/ Jay	Year / 5

WEIGHMASTER	APPTICIOATE
WEIGHMAGIEN	CERTIFICATE

MULTIPLE LOAD CERTIFICATE:

48012

PACIFIC ROCK, INC. P.O. BOX 257 SOMIS, CA 93066	WEIGHED AT: 1000 Pancho Road, Ca		SELLER/ WEIGHED FOR: PACIFIC ROCK, INC.
1(805) 389-0250 WEIGHT IN LBS.	BUYER/ DELIVERED TO: Val 7 RA		
29.2020 Gross	LOADS POUNDS	MARK	COMMODITY
128640 Tare	1 7/220	20,73	12 Ten
164380 Net	3 79900	21.34	ii e
82.19 Tons	4 73220	2053	11 1645
CARRIER FOLITES SUN	PACIFIC ROCK, INC., Weighmaster		
FOLIAL JR 24	GROSS BY Self-Jahren	•	Mith Day Year
TRUCK I.D. No		DEPUTY DATE	
TRAILER I.D. No.	TARE BY BALLAGE C	DEPUTY DATE	Mth. Day Year

WEIGHMA	STER	CERTIFI	CATE

PACIFIC ROCK, INC. P.O. BOX 257 SOMIS, CA 93066	WEIGHE	ED AT: 1000 Pancho Road, Camar		SELLER/ WEIGHED FOR: PACIFIC ROCK, INC.
1(805) 389-0250 WEIGHT IN LBS.	BUYER/ DELIVE	RED TO: Thille & Hile	000	Va LTURA NOS
31/96/2 Gross	LOADS	POUNDS	MARK	COMMODITY
123400 Tare	1	69140	12.00	3/2 4
122360 Net		63200	16.53	"
Tons	1/	73740	18.75	
CARRIER P. WIOW S DOW	PACIFIC	ROCK, INC., Weighmaster	/)	1
DRIVER EQUIPT. NO.	GROSS BY	Test, mo., weighnaster	<i>4</i>	Mth Day Year
TRUCK I.D. No		DEPU	TYDATE	7
TRAILER I.D. No.	TARE			Mth Day Year
TRAILER I.D. No.	BY	the deplement DEPU	TYDATE	31/31/5

WEIGH	MASTER	CERTIF	CATE

PACIFIC ROCK, INC. P.O. BOX 257 SOMIS, CA 93068	WEIGHE	DAT: 1000 Pancho Road, Car	marillo	SELLER/ WEIGHED FOR: PACIFIC ROCK, INC.
1(805) 389-0250 WEIGHT IN LBS.	BUYER/ DELIVER	RED TO: HEWAY ROZ	in	Constant
(Char) Gidss	LOADS	POUNDS	MARK	COMMODITY
I was been suit and been	1	24220	26,24	to " the set !
202 VU CTare	. "	73300	2276	- 1/
25 4430 Net	3	74560	20141	11
/ 3) - Tons	4	76 950	21.61	11
E got to got procure	3	75 640	20,95	* //
CARRIER	6	77/60	21.71	77
2 C/Y	PACIFIC	ROCK, INC., Weighmaster	4	
DRIVER EQUIPT.NO.	GROSS BY	Bel John		Mth Day Year
RUCK I.D. No		DI	EPUTY DATE	
RAILER I.D. No.	TARE	1 /h		
TRAILER I.D. No.	BY	Bull Judic 101	EPUTY DATE	Mth Day Year

WEIGHMASTER CERTIFICAT		

SO SO	C ROCK, INC. P.O. BOX 257 MIS, CA 93068	WEIGHE	DAT: 1000 Pancho Road, Ca		SELLER/ WEIGHED FOR: PACIFIC ROCK, INC.
(1) 10 10 10 10 10 10 10 10 10 10 10 10 10	805) 389-0250 GHT IN LBS.	BUYER/ DELIVER	1 July 12 14 14 15		
: 4/1,	10 820 Gross		POUNDS	MARK	COMMODITY
15	0680 Tare	2	2350	24,40	6" Anny
25	0200 Net		73300	14,78	A ,
1 3	C. 10 Tons	4	70749	19.49	11
1 67	CARRIER	BACIEIG	93355	21.67	**************************************
DRIVER	EQUIPT. NO.	GROSS	ROCK, INC., Weighmaster		Mth Day Year
TRUCK	I.D. No		D	DEPUTY DATE	
TRAILER	I.D. No.	TARE			Mile Day
TRAILER	I.D. No.	BY .	Sald Freshold 10	DEPUTY DATE	Mth Day Year

772 C. HELL TANKS - LAND				
MITTALINE	AOTED	AFRE	-	
WEIGHM	ASIER	CERT	II-IC:A	1 F

48017

PACIFIC ROCK, INC. P.O. BOX 257 SOMIS, CA 93066	WEIGHED AT: 1000 Pancho Road, Cama		SELLER/ WEIGHED FOR: PACIFIC ROCK, INC.
1(805) 389-0250 WEIGHT IN LBS.	BUYER/ DELIVERED TO: PAL / UR	eA.	
プロケラスク Gross	LOADS POUNDS	MARK	COMMODITY
153040 Tare	7 77420	1244	Va rom
144180 Net	3 25620	12.71	
22.09 Tons	4 76920	19.23	11
CARRIER			
E & Ome	PACIFIC ROCK, INC., Weighmaster		
DRIVER EQUIPT. NO.	GROSS BY		Mth. Day Year
TRUCK I.D. No	DEF	PUTY DATE	
TRAILER I.D. No.	TARE		Mth Day Year
TRAILER I.D. No.	BY Self-de DEF	PUTY DATE	Mth Day Year

WEIGHMASTER CERTIFICATE

THIS IS TO CERTIFY that the following described commodity was weighed, measured, or counted by a weighmaster, whose signature is on this certificate, who is a recognized authority of accuracy, as prescribed by Chapter 7 (commencing with Section 12700) of Division 5 of the California Business and Professions Code, administered by the Division of Measurement Standards of the California Department of Food and Agriculture.

PACIFIC ROCK, INC. P.O. BOX 257 SOMIS, CA 93066 1(805) 389-0250	WEIGHED AT: 1000 Pancho	Road, Camarillo	SELLER/ WEIGHED FOR: PACIFIC ROCK, INC.
WEIGHT IN LBS.	BUYER/ DELIVERED TO: 📿 🗓	and of A	•
44742 O Gross	LOADS POUNE	ALCO CONT. CO. CO. CO. CO. CO. CO. CO. CO. CO. CO	COMMODITY
195210 Tare	2 75 11 6	0 200	lo asine
153150 Net	3 73 5 5	C 2052	1,
126,59 Tons	7 2220	0 19.83	to the second second
CARRIER	6 74600	21.03	11
DRIVER EQUIPT, NO.	PACIFIC ROCK, INC., Weig	hmaster 🖔	
ANDY 10	GROSS BY	edeci e	Mth Day Year
TRUCK / I.D. No	C (40)	DEPUTY DATE	
FRAILER I.D. No.	TARE BY.	DEPUTY DATE	Mth Day Year
DRIVER 🗆 ON GROSS & TARE MULTIPLE LOAD CERTIFICAT	: TARE 32 37	40 x Addles	LOADS